### STRUCTURAL ABBREVIATIONS ABBREV. DEFINITION ABBREV. DEFINITION ADDNL additional HORZ horizonta AFF above finished floor inner face ALT alternate interior architectural B, BOT bottom of xxx BAL. BB balance latera bond beam pounds BCX bottom chord extension left end brick ledge long leg horizont BLDG building long leg outstandin BLW below Iongitudinal BRG BRK bearing masonrv MAX brick maximur BTWN mechanical between const., control joint MFR manufacture CLR CMU conc masonry unit MTL metal COL CONC column NOM nominal concrete OC, O/C on center CONN connection outer face, opp. face construction opposite hand CONT CTR continuous OPNG center deformed bar anchor precas DET,DTL detail plate DIM DWGS dimension right end drawings DWL REINF reinforceme each REQD required each end retaining SOG each face slab on grade effective slip critical expansion join schedule EL.ELEV elevation SECT section EOC edge of concrete edae of deck EOM edge of masonry STFNR stiffener edge of slab steel each side SUPPL supplier each way EXIST existing top of xxx EXP expansion top chord extension EXT exterior, extensior THK thick, thickness Foot-Kips transverse FL, FLR typical FOB face of brick unless noted otherwise FOM face of masonry VERT vertical FOS face of stud verify in field full penetration wide, width foot, feet welded wire fabric FTG footing general horizontal each face horizontal inner face

## 1.0 GENERAL NOTES:

- THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE DRAWINGS OF ALL OTHER DISCIPLINES AND THE SPECIFICATIONS. THE CONTRACTOR SHALL VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO SLEEVES, CHASES, HANGERS, INSERTS, ANCHORS, HOLES AND OTHER ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH ALL SAFETY PRECAUTIONS AND REGULATIONS DURING THE WORK. THE ENGINEER WILL NOT ADVISE ON NOR ISSUE DIRECTION AS TO SAFETY PRECAUTIONS AND PROGRAMS.
- THE STRUCTURAL DRAWINGS HEREIN REPRESENT THE FINISHED STRUCTURE.THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY GUYING AND BRACING REQUIRED TO ERECT AND HOLD THE STRUCTURE IN PROPER ALIGNMENT UNTIL ALL STRUCTURAL WORK AND CONNECTIONS HAVE BEEN COMPLETED. THE INVESTIGATION, DESIGN, SAFETY, ADEQUACY AND INSPECTION OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- THE ENGINEER SHALL NOT BE RESPONSIBLE FOR THE METHODS. TECHNIQUES AND SEQUENCES OF PROCEDURES TO PERFORM THE WORK. THE SUPERVISION OF THE WORK IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR
- DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION, WHERE CONDITIONS ARE NOT SPECIFICALLY SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO APPROVAL BY THE ENGINEER
- ALL STRUCTURAL SYSTEMS WHICH ARE TO BE COMPOSED OF COMPONENTS TO BE FIELD ERECTED. SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE AND ERECTION IN ACCORDANCE WITH THE SUPPLIER'S INSTRUCTIONS AND REQUIREMENTS
- LOADING APPLIED TO THE STRUCTURE DURING THE PROCESS OF CONSTRUCTION IS THE CONTRACTORS SOLE RESPONSIBILITY AND SHALL NOT EXCEED THE SAFE LOAD - CARRYING CAPACITY INDICATED IN THE "DESIGN CRITERIA NOTES". DO NOT APPLY ANY CONSTRUCTION LOADS UNTIL STRUCTURAL FRAMING IS PROPERLY CONNECTED TOGETHER AND UNTIL ALL TEMPORARY BRACING
- ALL ASTM AND OTHER REFERENCES ARE PER THE LATEST EDITIONS OF THESE STANDARDS, UNLESS
- SHOP DRAWINGS AND OTHER ITEMS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION, ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE GENERAL CONTRACTOR BEFORE SUBMITTAL, AND SHALL BEAR THE CONTRACTOR'S APPROVAL STAMP ACCEPTING RESPONSIBILITY FOR DIMENSIONS, QUANTITIES AND COORDINATION WITH OTHER TRADES. IF SHOP DRAWINGS AND OTHER SUBMITTALS DO NOT BEAR THE CONTRACTORS APPROVAL STAMP, THEY WILL NOT BE REVIEWED AND WILL BE RETURNED. NO EXCEPTIONS. THE ENGINEER'S REVIEW IS TO BE FOR CONFORMANCE WITH THE DESIGN CONCEPT AND GENERAL COMPLIANCE WITH THE RELEVANT CONTRACT DOCUMENTS.THE ENGINEER'S REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW
- CHECK AND COORDINATE THE SHOP DRAWINGS PRIOR TO SUBMISSION. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, DIMENSIONS, ETC. ALL SUBMITTALS INCLUDING CONCRETE MIX DESIGNS, CMU SPECS, ETC. MUST BE DATED AND NO MORE THAN ONE (1)
- SUBMIT SHOP DRAWINGS IN THE FORM OF THREE PRINTS.IN NO CASE SHALL REPRODUCTION OF THE CONTRACT DRAWINGS BE USED AS SHOP DRAWINGS WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER OF RECORD.CONTRACTOR SHALL PROVIDE IN HIS SCHEDULE FOR SHOP DRAWING REVIEW AND RETURN TIME, A MINIMUM OF FIFTEEN (15) WORKING DAYS IN THE STRUCTURAL ENGINEER'S OFFICE.AS A MINIMUM, SUBMIT THE FOLLOWING ITEMS FOR REVIEW: CONCRETE MIX DESIGN(S)
- REINFORCING STEEL SHOP DRAWINGS INCLUDING FLEVATED SLABS PRE-MANUFACTURED WOOD SYSTEM/TRUSS SHOP DRAWINGS WITH CALCULATIONS. OTHER SUBMITTALS MAY BE REQUIRED PER THE "SCHEDULE OF SPECIAL INSPECTIONS" OR THE SEPARATE NOTES CONTAINED HEREIN.
- UNLESS OTHERWISE INDICATED, ALL ITEMS NOTED TO BE DEMOLISHED SHALL BECOME THE CONTRACTOR'S PROPERTY AND BE REMOVED FROM THE SITE.
- CONTRACTORS SHALL VISIT THE SITE PRIOR TO BID TO ASCERTAIN CONDITIONS WHICH MAY ADVERSELY AFFECT THE WORK OR COST THEREOF
- 3. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE PROTECTION OF PERSONS AND PROPERTY EITHER ON OR ADJACENT TO THE PROJECT AND SHALL PROTECT SAME AGAINST INJURY, DAMAGE OR
- FIREPROOFING OF STRUCTURAL ELEMENTS IS NOT SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO THE SPECIFICATIONS AND ARCHITECTURAL DRAWINGS FOR FIRE RATING REQUIREMENTS,
- MATERIALS AND METHODS. THE CONTRACTOR SHALL INFORM THE STRUCTURAL ENGINEER, CLEARLY AND EXPLICITLY IN WRITING, OF ANY DEVIATION OR SUBSTITUTION OF REQUIREMENTS OF THE CONTRACT DOCUMENTS. CONTRACTOR IS NOT RELIEVED OF ANY REQUIREMENTS OF THE CONTRACT DOCUMENTS BY VIRTUE OF THE STRUCTURAL ENGINEER'S REVIEW OF SHOP DRAWINGS, PRODUCT DATA, ETC. UNLESS THE CONTRACTOR HAS CLEARLY AND EXPLICITLY INFORMED THE STRUCTURAL ENGINEER IN WRITING OF ANY DEVIATIONS OR SUBSTITUTIONS AT TIME OF SUBMISSION. AND THE STRUCTURAL ENGINEER HAS
- ALL THINGS WHICH, IN THE OPINION OF THE CONTRACTOR, APPEAR TO BE DEFICIENCIES, OMISSIONS, CONTRADICTIONS OR AMBIGUITIES IN THE DRAWINGS OR SPECIFICATIONS. SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER, CORRECTIONS OR WRITTEN INTERPRETATIONS SHALL BE ISSUED BEFORE AFFECTED WORK MAY PROCEED.

GIVEN WRITTEN APPROVAL FOR THE SPECIFIC DEVIATIONS OR SUBSTITUTIONS.

- . IF THE CONTRACTOR CANNOT CONSTRUCT ANY PORTION OF THE WORK IDENTIFIED IN THE DRAWINGS IN ACCORDANCE WITH THESE DRAWINGS AND SPECIFICATIONS, THEN THE CONTRACTOR IS RESPONSIBLE FOR CONTACTING THE STRUCTURAL ENGINEER PRIOR TO PROCEEDING WITH THE WORK, WORK THAT DOES NOT COMPLY WITH THE DRAWINGS MAY REQUIRE REMOVAL, TESTING, OR ENGINEERING EVALUATION AT THE CONTRACTOR'S EXPENSE.
- 3. CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS PRIOR TO ORDERING MATERIALS OR PROCEEDING WITH NEW WORK IN AREAS AFFECTED BY EXISTING CONDITIONS. STRUCTURAL ENGINEER SHALL BE INFORMED IN WRITING OF CONFLICTS BETWEEN EXISTING AND PROPOSED NEW
- CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL DIMENSIONS SHOWN ON THE CONTRACT DOCUMENTS. INCONSISTENCIES ON THE STRUCTURAL DRAWINGS OR BETWEEN THE STRUCTURAL DRAWINGS AND ANY OTHER CONTRACT, SHOP, FABRICATION, OR OTHER DRAWINGS OR INFORMATION SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER PRIOR TO PROCEEDING WITH AFFECTED WORK.

### 2.0 DESIGN CRITERIA NOTES:

COLD - FORMED METAL

PARTITION ALLOWANCE

BALCONY..

- THE PRIMARY DESIGN STANDARDS AND/OR CRITERIA INCLUDE BUT NOT LIMITED TO THE FOLLOWING: BLDG CODE (N.C.B.C.2018, AS AMENDED, ASCE 7-10) GENERA CONCRETI STRUCTURAL STEEL AISC 341 STEEL JOISTS / GIRDERS SJI METAL DECK
- DESIGN GRAVITY SUPER IMPOSED DEAD LOADS USED IN THE DESIGN OF THIS STRUCTURE ARE AS FOLLOWS (SELF WEIGHT OF STRUCTURE IS NOT INCLUDED): 10 PSF MIN 20 PSF MAX. FLOORS - TYPICAL 15 PSF

NAS

DESIGN GRAVITY LIVE LOADS USED IN THE DESIGN OF THIS STRUCTURE ARE AS FOLLOWS: SLAB ON GRADE.. ..200 PSF ROOF, TYPICAL... ..20 PSF MIN.

40 PSF

...60 PSF

+/- 0.18

- FLOOR LIVE LOAD REDUCTION PER N.C.B.C. HAS BEEN UTILIZED. ROOF LIVE LOAD REDUCTION PER N.C.B.C HAS BEEN UTILIZED.
- DESIGN LATERAL LOADS USED IN THE DESIGN OF THIS STRUCTURE ARE AS FOLLOWS:
- WIND LOADS PER ASCE7-10 BASIC WIND SPEED (3 SECOND GUST) 115 MPH RISK CATEGORY WIND EXPOSURE
- INTERNAL PRESSURE COEFFICIENT "GCpi" COMPONENTS & CLADDING PRESSURES (PSF)

APARTMENT/CORRIDOR.

	TRIBUTARY AREA (FT² )			
	ZONE	$A \leq 10$	A = 50	A≥100
WALLS	ZONE 4 (-)	-28.0	-25.3	-24.2
WALLS	ZONE 5 (-)	-34.6	-29.2	-26.9
	ZONE 4&5(+)	+25.8	+23.1	+22.0
	ZONE 1 (-)	-32.4	-29.4	-28.0
ROOF	ZONE 2 (-)	-39.0	-32.9	-30.2
	ZONE 3 (-)	-67.5	-53.7	-47.8
	ALL ZONES (+)	+16.0	+16.0	+16.0

\* CORNER & EDGE ZONES SHALL EXTEND 10'-8"FROM BUILDING EDGES. \* "+" INDICATES POSITIVE AND "-" NEGATIVE PRESSURE (SUCTION)

NET ROOF SUCTION

WING MAIN WIND FORCE RESISTING SYSTEM	NORTH/SOUTH	EAST/WEST
	LIGHT FRAME SHEAR WALLS	LIGHT FRAME SHEAR WALLS
WIND BASE SHEAR "Vo"	78 KIPS	55 KIPS
SEISMIC LOADS PER N.C.B.C. 2018:		
SITE CLASS		D (ASSUMED)
SHORT PERIOD DESIGN SPECTRAL RESPONSE "Sds"		0.308g
1-SECOND PERIOD DESIGN SPECTRAL RESPONSE "Sd1"		0.169g
SEISMIC USE GROUP		II
IMPORTANCE FACTOR "le"		1.0
SEISMIC DESIGN CATEGORY		С
WING SEISMIC		
FORCE RESISTING SYSTEM	NORTH/SOUTH	EAST/WEST
	LIGHT FRAME	LIGHT FRAME

LIGHT FRAME SHEAR WALLS

5.	DESIGN SNOW LOADS USED IN THE DESIGN OF THIS STRUCTURE ARE AS FOLLOWS: DRIFTING SNOW LOADS PER ASCE 7-10: NONE WHERE "Pg" IS LESS THAN 10 PSF		
	50 YEAR GROUND SNOW LOAD "Pg"	15 PSF	
	EXPOSURE FACTOR "Ce"	1.0	
	THERMAL FACTOR "Ct"	1.0	
	IMPORTANCE CATEGORY CLASSIFICATION	II	
	IMPORTANCE FACTOR "Is"	1.0	
	Pf	10.5 PSF (ASCE 7-05, EQ. 7-1)	
	NET FLAT ROOF SNOW LOAD "Pf"	10.5 PSF `	

SHEAR WALLS

THIS STRUCTURE HAS BEEN DESIGNED WITH "SAFETY FACTORS" IN ACCORDANCE WITH GENERALLY ACCEPTED PRINCIPLES OF STRUCTURAL ENGINEERING. THE FUNDAMENTAL NATURE OF THE "SAFETY FACTOR" IS TO COMPENSATE FOR UNCERTAINTIES IN THE DESIGN. FABRICATION AND ERECTION OF STRUCTURAL BUILDING COMPONENTS. IT IS INTENDED THAT "SAFETY FACTORS" BE USED SO THAT THE LOAD CARRYING CAPACITY OF THE STRUCTURE DOES NOT FALL BELOW THE DESIGN LOAD AND THAT THE BUILDING WILL PERFORM UNDER DESIGN. LOAD WITHOUT DISTRESS. WHILE THE USE OF "SAFETY FACTORS" IMPLIES SOME EXCESS CAPACITY BEYOND DESIGN LOAD, SUCH EXCESS CAPACITY CANNOT BE ADEQUATELY PREDICTED AND SHALL NOT BE RELIED UPON.

BUILDING SHALL NOT BE USED AS AN EMERGENCY SHELTER.

### 3.0 DEFERRED SUBMITTAL NOTES:

- SHOP DRAWINGS AND OTHER ITEMS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO
- IN NO CASE SHALL REPRODUCTION OF THE CONTRACT DRAWINGS BE USED AS SHOP DRAWINGS WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER OF RECORD. CONTRACTOR SHALL PROVIDE IN HIS SCHEDULE FOR SHOP DRAWING REVIEW AND RETURN TIME, A MINIMUM OF FIFTEEN (15) WORKING DAYS IN THE STRUCTURAL ENGINEER'S OFFICE.
- ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE GENERAL CONTRACTOR BEFORE SUBMITTAL, AND SHALL BEAR THE CONTRACTOR'S APPROVAL STAMP ACCEPTING RESPONSIBILITY FOR DIMENSIONS, QUANTITIES AND COORDINATION WITH OTHER TRADES. IF SHOP DRAWINGS AND OTHER SUBMITTALS DO NOT BEAR THE CONTRACTORS APPROVAL STAMP, THEY WILL NOT BE REVIEWED AND WILL BE RETURNED. NO EXCEPTIONS
- 4. ALL SHOP DRAWINGS AND CALCULATIONS FOR DELEGATED DESIGN REQUIRING AN ENGINEER'S SEAL SHALL BE SEALED PRIOR TO SUBMISSION FOR REVIEW. IF SHOP DRAWINGS AND OTHER SUBMITTALS DO NOT BEAR THE DELEGATED ENGINEER'S SEAL, THEY WILL NOT BE REVIEWED AND WILL BE
- WHERE NOTED SEALED DRAWINGS OR CALCULATIONS ARE REQUIRED TO BE SEALED AND SIGNED BY A LICENSED STRUCTURAL ENGINEER IN THE PROJECT STATE. NOTE THAT PLACEMENT OR LAYOUT PLANS FOR TRUSSES AND JOISTS DO NOT REQUIRE ENGINEERS SEAL
- THE ENGINEER OR RECORD'S (EOR) REVIEW IS TO BE FOR CONFORMANCE WITH THE DESIGN CONCEPT AND GENERAL COMPLIANCE WITH THE RELEVANT CONTRACT DOCUMENTS.THE EOR REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW. CHECK AND COORDINATE THE SHOP DRAWINGS PRIOR TO SUBMISSION. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, DIMENSIONS, ETC. ALL SUBMITTALS INCLUDING CONCRETE MIX DESIGNS, CMU SPECS, ETC. MUST BE DATED AND NO MORE THAN ONE (1)
- AS A MINIMUM, SUBMIT THE FOLLOWING ITEMS FOR REVIEW:. CONCRETE MIX DESIGN(S).
- B. REINFORCING STEEL SHOP DRAWINGS PRE-MANUFACTURED WOOD SYSTEM/TRUSS SHOP DRAWINGS WITH CALCULATIONS. OTHER SUBMITTALS MAY BE REQUIRED PER THE "SCHEDULE OF SPECIAL INSPECTIONS" OR THE SEPARATE NOTES CONTAINED HEREIN.
- 8. ANY SHOP DRAWINGS WITH LANGUAGE LIMITING REVIEWER RESPONSES SUCH AS BUT NOT LIMITED TO THE FOLLOWING WILL NOT BE REVIEWED AND WILL BE RETURNED. NO EXCEPTIONS.
- "RESPONSES SUCH AS "GC TO VERIFY" OR "ARCH TO VERIFY" ARE NOT ACCEPTABLE B" CLOUDS MARKED IN ..... MANNER WILL BE CONSIDERED NOT ADDRESSED"
- 9. SHOP DRAWINGS SHALL NOT BE USED AS RFI'S AND ARE TO BE CONSIDERED COMPLETELY SEPARATE SUBMITTALS.

## 4.0 SITE PREPARATION NOTES:

- WITHIN AN AREA A MINIMUM OF 5 FEET BEYOND THE BUILDING LIMITS, EXCAVATE A MINIMUM OF 3' OF EXISTING SOIL REMOVE ALL ORGANICS PAVEMENT ROOTS. DEBRIS AND OTHERWISE
- THE SURFACE OF THE EXPOSED SUBGRADE SHALL BE INSPECTED BY PROBING OR TESTING TO CHECK FOR POCKETS OF SOFT OR UNSUITABLE MATERIAL. EXCAVATE UNSUITABLE SOIL AS
- DIRECTED BY THE GEOTECHNICAL ENGINEER / TESTING AGENCY. PROOF-ROLL THE SURFACE OF THE EXPOSED SUBGRADE WITH A LOADED TANDEM AXLE DUMP
- TRUCK.REMOVE ALL SOILS WHICH PUMP OR DO NOT COMPACT PROPERLY AS DIRECTED BY THE GEOTECHNICAL ENGINEER/TESTING AGENCY.
- 4 FILL ALL EXCAVATED AREAS WITH APPROVED CONTROLLED FILL PLACE IN 8 INCH LOOSE LIFTS AND COMPACT TO A MINIMUM OF 95% OF THE MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D - 698
- ALL CONTROLLED FILL MATERIAL SHALL BE A SELECT GRANULAR MATERIAL FREE FROM ALL ORGANICS OR OTHERWISE DELETERIOUS MATERIAL WITH NOT MORE THAN 20% BY WEIGHT PASSING A NO. 200 SIEVE ( CLASSIFIED AS SC. SM. SP OR BETTER IN ACCORDANCE WITH THE UNIFIED SOIL CLASSIFICATION SYSTEM ) AND WITH A PLASTICITY INDEX NOT EXCEEDING 6%
- PROVIDE FIELD DENSITY TESTS FOR EACH 3,000 S.F. OF BUILDING AREA FOR EACH LIFT OF

### 5.0 FOUNDATION NOTES: (TYP)

FEET IN ANY SINGLE POUR.

- 1. FOUNDATION DESIGN IS BASED ON GEOTECHNICAL REPORT BY BLE CORP, DATED
- 2 ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACL 301 "SPECIFICATION" FOR STRUCTURAL CONCRETE BUILDINGS ". HOT WEATHER CONCRETING SHALL BE IN ACCORDANCE WITH ACI 305.COLD WEATHER CONCRETING SHALL BE IN ACCORDANCE WITH
- ALL REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60, UNLESS OTHERWISE NOTED.
- SEE "CAST-IN-PLACE CONCRETE NOTES" FOR MINIMUM CONCRETE COVER REQUIREMENTS AND CONCRETE ELEMENT PROPERTIES
- ALL REINFORCING MARKED CONTINUOUS (CONT.) ON THE PLANS AND DETAILS SHALL BE LAPPED LTS AT SPLICES UNLESS OTHERWISE NOTED SEE EMBEDMENT & LAP SPLICE
- NO UNBALANCED BACKFILLING SHALL BE DONE AGAINST FOUNDATION WALLS UNLESS WALLS ARE SECURELY BRACED AGAINST OVERTURNING, EITHER BY TEMPORARY BRACING OR BY PERMANENT CONSTRUCTION.
- PRIOR TO COMMENCING ANY FOUNDATION WORK, THE CONTRACTOR IS SOLELY RESPONSIBLE FOR COORDINATING WORK WITH ANY EXISTING UTILITIES. FOUNDATIONS SHALL BE LOWERED WHERE REQUIRED TO AVOID UTILITIES. STRUCTURAL ENGINEER MUST BE NOTIFIED IF FOOTINGS ARE LOWERED MORE THAN 2 FEET RELATIVE TO THAT WHICH IS SHOWN
- UNLESS OTHERWISE NOTED, THE CENTERLINES OF COLUMN FOUNDATIONS SHALL BE LOCATED ON COLUMN CENTERLINES.
- ALL RETAINING WALLS SHALL HAVE AT LEAST 12" OF FREE DRAINING GRANULAR BACKFILL FULL HEIGHT OF WALL PROVIDE VERTICAL CONTROL JOINTS NOT TO EXCEED 25 FEET O NOR 3 TIMES THE WALL HEIGHT. MAXIMUM LENGTH OF WALL POURS SHALL NOT EXCEED 50
- 10. BOTTOM OF EXTERIOR FOUNDATIONS SHALL BEAR AT A MINIMUM DEPTH OF 1'-6" BELOW FINAL
- GRADE FOR FROST PROTECTION. ALL FOOTINGS HAVE BEEN DESIGNED BASED UPON AN ASSUMED SOIL BEARING PRESSURE OF 2500 PSF. ALL FOOTINGS SHALL BEAR ON UNDISTURBED, FIRM NATURAL SOIL OR COMPACTED FILL. ALL FOUNDATION EXCAVATIONS SHALL BE EVALUATED BY THE GEOTECHNICAL ENGINEER/TESTING AGENCY PRIOR TO POURING FOUNDATION CONCRETE.
- TOP OF FOOTING ELEVATION SHALL BE AS SHOWN ON THE FOUNDATION PLAN. THESE ELEVATIONS ARE A MAXIMUM AND SHALL BE LOWERED AS REQUIRED TO OBTAIN THE REQUIRED DESIGN BEARING PRESSURE. STRUCTURAL ENGINEER MUST BE NOTIFIED IF FOOTINGS ARE LOWERED MORE THAN 2 FEET RELATIVE TO THAT WHICH IS SHOWN.
- WHERE FOOTING EXCAVATIONS MUST REMAIN OPEN OVERNIGHT OR IF RAINFALL BECOMES IMMINENT WHILE BEARING SOILS ARE EXPOSED, A 2" TO 4" THICK MUD MAT OF UNREINFORCED LEAN (fc = 2000psi) CONCRETE SHALL BE PLACED ON THE BEARING SOILS

BEFORE PLACEMENT OF THE FOOTING REINFORCING.

### 6.0 SLAB ON GRADE NOTES:

- PROVIDE CONCRETE SLABS OVER A VAPOR BARRIER PER ARCHITECT DRAWINGS AND 4" OF POROUS FILL. CONCRETE SLABS SHALL HAVE A MAXIMUM SLUMP OF 5 INCHES, USING TYPE 1
- ALL WELDED WIRE FABRIC SHALL BE IN ACCORDANCE WITH ASTM A-185. LAP ADJOINING PIECES AT LEAST ONE FULL MESH
- ALL POROUS FILL MATERIAL SHALL BE A CLEAN GRANULAR MATERIAL WITH 100% PASSING A 1-1/2" SIEVE AND NO MORE THAN 5% PASSING A NO. 4 SIEVE. POROUS FILL SHALL BE

COMPACTED TO 95% MAX. DRY DENSITY PER ASTM D-698.

- SLAB JOINTS SHALL BE FILLED WITH APPROVED MATERIAL. THIS SHOULD TAKE PLACE AS LATE AS POSSIBLE, PREFERABLY 4 TO 6 WEEKS AFTER THE SLAB HAS BEEN CAST. PRIOR TO FILLING REMOVE ALL DEBRIS FROM THE SLAB JOINTS. THEN FILL IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AS FOLLOWS: 6" SLABS - FILL WITH EPOXY RESIN
- OTHER SLABS FILL WITH FIELD MOLDED OR ELASTOMERIC SEALAN
- UNLESS OTHERWISE APPROVED, ALL SLAB REINFORCEMENT SHALL BE SECURED INTO POSITION WITH PLASTIC TIPPED OR STAINLESS STEEL BAR SUPPORTS. BRICK OR OTHER MASONRY ARE NOT PERMITTED FOR USE AS SUPPORTS.
- WALKWAYS AND OTHER EXTERIOR SLABS ARE NOT INDICATED ON THE STRUCTURAL DRAWINGS. SEE THE SITE PLAN AND ARCHITECTURAL DRAWINGS FOR LOCATIONS. DIMENSIONS. ELEVATIONS, JOINTING DETAILS AND FINISH DETAILS. PROVIDE 4" WALKS REINFORCED WITH 6X6 -W1.4XW1.4 WWF UNLESS OTHERWISE NOTED.
- SLABS TO BE PERMANENTLY EXPOSED TO WEATHER SHALL BE AIR ENTRAINED TO 5% (+/- 1%) WITH AN ADMIXTURE THAT CONFORMS TO ASTM C-260.
- SLABS NOT PERMANENTLY EXPOSED TO WEATHER SHALL NOT BE AIR ENTRAINED AND ENTRAPPED AIR SHALL BE LIMITED TO 3%.
- 9. ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 301, "SPECIFICATION FOR STRUCTURAL CONCRETE BUILDINGS". HOT WEATHER CONCRETING SHALL BE IN ACCORDANCE WITH ACI 305 COLD WEATHER CONCRETING SHALL BE IN ACCORDANCE WITH ACI 306.
- 10 IN ORDER TO AVOID CONCRETE SHRINKAGE CRACKING, PLACE CONCRETE SLABS IN AN ALTERNATING LANE ( OR CHECKERBOARD ) PATTERN, THE MAXIMUM LENGTH OF SLAB CAST IN ANY ONE CONTINUOUS POUR IS RECOMMENDED TO BE LESS THAN 100 FEET. THE MAXIMUM SPACING OF CONTROL JOINTS SHALL BE 3 TIMES THE SLAB THICKNESS IN FEET. (EXAMPLE 4" SLAB X 3 = 12'-0" CJ SPACING TYPICAL)
- . THE USE OF POLYPROPYLENE FIBERS (IN LIEU OF WELDED WIRE FABRIC) IS PROHIBITED WITHOUT THE WRITTEN AUTHORIZATION OF THE ENGINEER
- 2. SEE THE ARCHITECTURAL DRAWINGS FOR EXACT LOCATION OF DEPRESSED SLAB AREAS AND DRAINS. SLOPE SLAB TO DRAINS WHERE SHOWN
- 13. THE FINISH TOLERANCE OF ALL SLABS SHALL BE IN ACCORDANCE WITH ACI 301, TYPE A.

# 7.0 POST-INSTALLED ANCHORS:

POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE ENGINEER-OF-RECORD (E.O.R.) PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REBAR. HOLES SHALL BE DRILLED AND CLEANED IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII), SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW. SHALL BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER-OF-RECORD ALONG WITH CALCULATIONS THAT ARE PREPARED & SEALED BY A REGISTERED PROFESSIONAL ENGINEER. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERTINENT EQUIVALENT PERFORMANCE VALUES (MINIMUM) OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARD(S). PROVIDE SPECIAL INSPECTIONS AS REQUIRED BY THE ANCHOR'S EVALUATION

REPORT (ICC-ES ESR OR IAPMO-UES CONTACT MANUFACTURER'S REPRESENTATIVE FOR THE INITIAL

- TRAINING AND INSTALLATION OF ANCHORS AND FOR PRODUCT RELATED QUESTIONS AND AVAILABILITY. CALL SIMPSON STRONG-TIE AT (800) 999-5099. FOR ANCHORING INTO CRACKED AND UN-CRACKED CONCRETE
- MECHANICAL ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.2 AND / OR ICC-ES AC193 FOR CRACKED AND UN-CRACKED CONCRETE. PRE-APPROVED PRODUCTS INCLUDE:
- SIMPSON STRONG-TIE "STRONG-BOLT 2" (ICC-ES ESR-2713)
- SIMPSON STRONG-TIE "TITEN HD", "TITEN HD-RD" & "TITEN HD-CS" (ICC-ES ESR-2713) SIMPSON STRONG-TIF "STAINLESS STEEL TITEN HD" (IAPMO-UES ER-493) SIMPSON STRONG-TIE "TITEN TURBO" (IAPMO-UES ER-712)
- ADHESIVE ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI355.4 AND ICC-ES AC308 FOR CRACKED AND UN-CRACKED CONCRETE. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING A MINIMUM AGE OF 21 DAYS. HOLES SHALL BE DRY AT THE TIME OF INSTALLATION, ACI355-4 TEMPERATURE CATEGORY 'B', ASSUMED IN DESIGN PRIOR TO INSTALLATION OF ADHESIVE ANCHORS IN HORIZONTAL OR LIPWARDLY INCLINED ORIENTATIONS RESISTING SUSTAINED TENSION LOADS, INSTALLERS ARE REQUIRED TO BE CERTIFIED IN ACCORDANCE WITH THE ACI/CRSI ADHESIVE ANCHOR
- INSTALLER CERTIFICATION PROGRAM AND MUST BE CONTINUOUSLY INSPECTED. PRE-APPROVED PRODUCTS INCLUDE: THREADED ROD & REBAR AS ANCHOR ELEMENTS - SIMPSON STRONG-TIE "SET-3G"
- (ICC-ES ESR-4057) THREADED ROD & REBAR AS ANCHOR ELEMENTS - SIMPSON STRONG-TIE "SET-XP"
- (ICC-ES ESR-2508) THREADED ROD & REBAR AS ANCHOR ELEMENTS - SIMPSON STRONG-TIE "AT-XP"
- POST INSTALLED REINFORCING BARS USING THE ACI318 DEVELOPMENT LENGTH PROVISION - SIMPSON STRONG-TIE "SET-3G" (ICC-ES ESR-4057) POST INSTALLED REINFORCING BARS USING THE ACI318 DEVELOPMENT LENGTH
- PROVISION SIMPSON STRONG-TIE "SET-XP" (ICC-ES ESR-2508) SIMPSON STRONG-TIE CLEAN DXS DUST EXTRACTION SYSTEM IN APPROVED FOR USE WITH THE PRODUCTS LISTED ABOVE TO DRILL AND CLEAN HOLES.
- 3. FOR ANCHORING INTO GROUT-FILLED CONCRETE MASONRY UNITS. MECHANICAL ANCHORS - SHALL HAVE BEEN TESTED IN ACCORDANCE WITH
- ICC-ES AC01 (EXPANSION ANCHORS) OR ICC-ES AC106 (SCREW ANCHORS) PRE-APPROVED PRODUCTS INCLUDE:
- SIMPSON STRONG-TIE "STRONG-BOLT 2" (IAPMO-UES ER-240) SIMPSON STRONG-TIF "WEDGE-ALL" (ICC-ES ESR-1396)
- SIMPSON STRONG-TIE "TITEN HD" & "STAINLESS STEEL TITEN HD"
- SIMPSON STRONG-TIE "TITEN TURBO" (IAPMO-UES ER-716)
- B ADHESIVE ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH (ICC-ES AC58) PRE-APPROVED PRODUCTS INCLUDE:
  - SIMPSON STRONG-TIE "SET-XP" (IAPMO-UES ER-265) SIMPSON STRONG-TIF "AT-XP" (IAPMO-UES FR281)
- SIMPSON STRONG-TIE "ET-HP" (IAPMO-UES ER241)
- 4. FOR ANCHORING INTO HOLLOW CONCRETE MASONRY UNITS MECHANICAL ANCHORS - SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC106 FOR PERFORMANCE IN HOLLOW CONCRETE MASONRY. PRE-APPROVED
- PRODUCTS INCLUDE: SIMPSON STRONG-TIE "STAINLESS STEEL TITEN HD" (ICC-ES ESR-1056)
- SIMPSON STRONG-TIE "TITEN TURBO" (IAPMO-UES ER-716) B. ADHESIVE ANCHORS - SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC58
- FOR PERFORMANCE IN HOLLOW CONCRETE MASONRY USING MANUFACTURER'S RECOMMENDED SCREEN TUBES. PRE-APPROVED PRODUCTS INCLUDE: SIMPSON STRONG-TIE "SET-XP" (IAPMO-UES ER-265)

PRE-APPROVED PRODUCTS INCLUDE:

- SIMPSON STRONG-TIE "AT-XP" (IAPMO-UES ER-281) 5. FOR ANCHORING INTO UN-REINFORCED MASONRY A. ADHESIVE ANCHORS - SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC60 FOR PERFORMANCE IN UN-REINFORCED MASONRY CONFIGURATIONS A, B, AND C.
- SIMPSON STRONG-TIE "ET-HP" (ICC-ES ER-3638) 5. FOR ANCHORING LOW VELOCITY AND THREADED STUDS INTO CONCRETE, MASONRY AND STEEL
- POWDER-ACTUATED FASTENERS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC70. PRE-APPROVED PRODUCTS INCLUDE:
- SIMPSON STRONG-TIE "POWDER-ACTUATED FASTENERS" (ICC-ES ESR-2138)
- B. GAS-ACTUATED FASTENERS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ICC-ES AC70. PRE-APPROVED PRODUCTS INCLUDE: SIMPSON STRONG-TIE "GAS ACTUATED FASTENERS" (ICC-ES ESR-2811)

### 8.0 CAST-IN-PLACE CONCRETE NOTES:

FOOTINGS

SLABS-ON-GRADE

CONCRETE MIXES SHALL BE DESIGNED PER ACL 301 CHAPTER 3. USING PORTLAND CEMENT CONFORMING TO ASTM C-150 OR C-595.AGGREGATE CONFORMING TO ASTM C-33. AND ADMIXTURES CONFORMING TO ASTM C-494, C-1017, C-618, C-989 AND C-260, CONCRETE SHALL BE READY-MIXED IN ACCORDANCE WITH ASTM C-94

3000 PSI

3000 PSI

CONCRETE SHALL CONFORM TO THE FOLLOWING COMPRESSIVE STRENGTH, SLUMP AND UNIT WEIGHT RATIO REQUIREMENTS: FLEMENT MIN fc (28 DAYS) UNIT WEIGHT CONCRETE NOT NOTED 3000 PSI 2" TO 4" 145 PCF

\*AT CONTRACTOR'S OPTION. AN APPROVED ADMIXTURE MAY BE USED TO PRODUCE FLOWABLE CONCRETE. MAXIMUM SLUMP SHALL NOT EXCEED 10 INCHES. THE CONTRACTOR SHALL SUBMIT TEST RESULTS OF THE PROPOSED CONCRETE MIXES ALONG WITH THE MANUFACTURER'S

- TECHNICAL DATA FOR APPROVAL PRIOR TO POURING CONCRETE ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 301, "SPECIFICATION FOR
- STRUCTURAL CONCRETE BUILDINGS". HOT WEATHER CONCRETING SHALL BE IN ACCORDANCE WITH ACI 305. COLD WEATHER CONCRETING SHALL BE IN ACCORDANCE WITH ACI 306.

2" TO 4"

2" TO 4"

145 PCF

145 PCF

- 4. WATER REDUCING ADMIXTURE SHALL BE USED IN ALL CONCRETE.
- AIR ENTRAINING ADMIXTURE IN ACCORDANCE WITH ACI 301 TABLE 3.4.1 SHALL BE USED IN ALL CONCRETE EXPOSED TO FREEZING AND THAWING DURING CONSTRUCTION AND/OR SERVICE
- WATER/CEMENT RATIO SHALL NOT EXCEED 0.50 FOR ANY CONCRETE SUBJECTED TO
- ALL PUMPED CONCRETE SHALL HAVE A WATER/CEMENT RATIO LESS THAN 0.50 AND SHALL CONTAIN A HIGH RANGE WATER REDUCING ADMIXTURE (SUPERPLASTICIZER
- IN NO CASE SHALL A WATER/CEMENT RATIO EXCEED THE FOLLOWING: f'c 3000 PSI 0.60 MAX. w/c RATIO
- ALL REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60 U.N.O. EXCEPT THAT REINFORCING WHICH IS REQUIRED TO BE WELDED SHALL CONFORM TO ASTM A706. ALL WELDING OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH AWS D1.4. EPOXY COATED
- 10. ALL WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A-185.

REINFORCING SHALL CONFORM TO ASTM A-775.

- ALL REINFORCING STEEL SHALL BE SET AND TIED IN PLACE PRIOR TO POURING OF CONCRETE DO NOT FIELD BEND BARS PARTIALLY EMBEDDED IN HARDENED CONCRETE UNLESS SPECIFICALLY INDICATED OR APPROVED BY THE ENGINEER.
- REINFORCING STEEL INCLUDING HOOKS AND BENDS SHALL BE DETAILED IN ACCORDANCE WITH ACI 315. ALL REINFORCING STEEL INDICATED AS BEING CONTINUOUS (CONT) SHALL BE LAPPED "LTS" PER EMBEDMENT AND LAP SPLICE SCHEDULE UNLESS OTHERWISE NOTED.
- UNLESS OTHERWISE NOTED, THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT CONCRETE EXPOSED TO EARTH OR WEATHER:
- #6 THROUGH #18 BARS #5 BAR W31 OR D31 WIRE & SMALLER - 1 1/2" CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND: SLABS. WALLS & JOISTS #14 AND #18 BARS #11 BAR AND SMALLER BEAMS AND COLUMNS

CONCRETE CAST AGAINST EARTH

SHALL PASS CONTINUOUSLY THROUGH THE JOINT

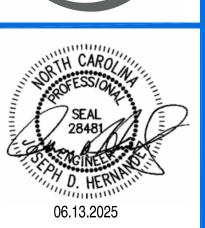
STRUCTURAL ENGINEER.

- PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS - 1 1/2" SHELLS FOLDED PLATE MEMBERS #6 BAR AND LARGER #5 BAR, W31 OR D31 WIRE AND SMALLER
- BAR SUPPORTS AND HOLDING BARS SHALL BE PROVIDED FOR ALL REINFORCING STEEL TO INSURE MINIMUM CONCRETE COVER AND PLACEMENT. BAR SUPPORTS SHALL BE PLASTIC TIPPED OR
- 15. ALL EDGES OF PERMANENTLY EXPOSED CONCRETE SURFACES SHALL BE CHAMFERED 3/4" UNLESS 16. THE CONTRACTOR SHALL PROVIDE THE ENGINEER WITH DOCUMENTATION THAT ALL MATERIALS
- CONFORM TO THE QUALITY STANDARDS SPECIFIED IN THE GENERAL BUILDING CODE. IN ORDER TO AVOID CONCRETE SHRINKAGE CRACKING. PLACE CONCRETE SLABS IN AN ALTERNATING LANE PATTERN. THE MAXIMUM LENGTH OF SLAB CAST IN ANY ONE CONTINUOUS
- POUR SHALL BE LIMITED TO 80 FEET 18. FORMWORK SHALL REMAIN IN PLACE UNTIL CONCRETE HAS OBTAINED AT LEAST 90% OF ITS 28
- DAY COMPRESSIVE STRENGTH. THE CONTRACTOR SHALL PROVIDE ALL SHORING AND RE-SHORING. CONSTRUCTION JOINTS, REQUIRED TO FACILITATE CONSTRUCTION, ARE SUBJECT TO THE APPROVAL OF THE STRUCTURAL ENGINEER AND MAY REQUIRE ADDITIONAL REINFORCING. SUCH JOINTS SHALL BE CLEARLY DETAILED ON THE SHOP DRAWINGS AND ALL REINFORCING
- 20. REFER TO ARCHITECTURAL DRAWINGS FOR DOOR AND WINDOW OPENINGS. DRIPS. REGLETS. WASHES, MASONRY ANCHORS, BRICK LEDGE ELEVATIONS, SLAB DEPRESSIONS AND
- 21. REFER TO ARCHITECTURAL DRAWINGS FOR CONCRETE FINISHES. WHERE FINISH IS NOT SPECIFIED, CONFORM TO REQUIREMENTS OF ACI 301.
- 22. REFER TO PLUMBING, MECHANICAL AND ELECTRICAL DRAWINGS FOR UNDERFLOOR, PERIMETER AND OTHER DRAINS AND FOR SLEEVES, OUTLET BOXES, CONDUIT, ANCHORS, ETC. THE VARIOUS TRADES ARE RESPONSIBLE FOR THEIR ITEMS.
- 23. FILL SLABS, NOT SHOWN ON THE STRUCTURAL DRAWINGS, SHALL BE REINFORCED WITH A MINIMUM OF 6X6-W1.4XW1.4 WWM UNLESS NOTED OTHERWISE ON OTHER DRAWINGS OR IN THE 24. REINFORCING BARS SHALL BE WELDED ONLY WHERE SHOWN ON THE STRUCTURAL DRAWINGS AND
- (AWS D1.4). NO OTHER REINFORCING MAY BE WELDED WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER. TACK WELDING OF ANY REINFORCING IS STRICTLY PROHIBITED. 25. ALL REINFORCING TERMINATING AT THE TOPS OF THE COLUMNS AND PILASTERS SHALL BE HOOKED,
- 26. CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER A MINIMUM OF 48 HOURS PRIOR TO ALL CONCRETE POURS IN ORDER TO PERMIT REINFORCING STEEL REVIEW IF REQUIRED BY THE

WELDS SHALL BE IN ACCORDANCE WITH THE "STRUCTURAL WELDING CODE - REINFORCING STEEL"

TRUCTURA STRUCTURA SINGINEERS





SIGNATURE:

The orchards at Naples Road, LI 341 N main Street Hendersonville, NC 28792 Luis Graef: Presiden



PROJECT:

 $\leq$ Naple

REVISIONS

DWG DECRIPTION

ISSUE DATE: 09/27/24

PROJECT #: 22105

DRAWN BY:

CHECKED BY:

**GENERAL NOTES** 

### 9.0 PLYWOOD/GYPBOARD SHEATHING TO WOOD NOTES:

- ALL PLYWOOD CONSTRUCTION SHALL BE IN ACCORDANCE WITH AMERICAN PLYWOOD ASSOCIATION (APA) SPECIFICATIONS.
- ALL ROOF PANEL SHEATHING SHALL BE 7/16" (NOM.) TYPE CDX. EXP. 1 APA RATED 24/16 SHEATHING. SUITABLE EDGE SUPPORT SHALL BE PROVIDED BY USE OF PANEL CLIPS OR BLOCKING BETWEEN FRAMING. UNLESS OTHERWISE NOTED CONNECT ROOF SHEATHING WITH 6d COMMON NAILS AT 6" O/C AT SUPPORTED PANEL EDGES AND 12" O/C AT INTERMEDIATE SUPPORTS.
- 3. ALL FLOOR SHEATHING SHALL BE 19/32" (NOM.) APA RATED STURD-1-FLOOR, @ 16" O.C. EXP. 1, WITH TONGUE AND GROOVE EDGE. UNLESS OTHERWISE NOTED CONNECT FLOOR SHEATHING WITH 10d COMMON NAILS SPACED 6" O/C AT SUPPORTED EDGES AND 12" O/C AT INTERMEDIATE SUPPORTS. FIELD-GLUE USING ADHESIVES MEETING APA SPECIFICATIONS AFG-01, APPLIED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
- 4. ALL WALL PANEL SHEATHING, INCLUDING DESIGNATED SHEAR WALLS, SHALL BE 7/16" (NOM.) TYPE CDX, EXP. 1 APA RATED 24/16 SHEATHING. UNLESS OTHERWISE INDICATED, CONNECT WALL SHEATHING WITH 10d COMMON NAILS SPACED 6" O/C AT SUPPORTED PANEL EDGES AND 12" O/C AT INTERMEDIATE SUPPORTS. SEE SHEAR WALL SCHEDULE FOR FASTENING REQUIREMENTS.
- 5. INSTALL ALL PLYWOOD SHEATHING WITH THE LONG DIMENSION OF THE PANEL ACROSS SUPPORTS AND WITH PANEL CONTINUOUS OVER TWO OR MORE SPANS. STAGGER PANEL END JOINTS. ALLOW 1/8" SPACING AT PANEL ENDS AND EDGES UNLESS OTHERWISE RECOMMENDED BY THE SHEATHING MANUFACTURER.
- 6. ALL NAILING SHALL BE CAREFULLY DRIVEN AND NOT OVERDRIVEN. THE USE OF PNUEMATIC NAIL GUNS MAY BE USED PROVIDED (1) NAIL IS INSTALLED FOR EVERY OVERDRIVEN NAIL (THOSE SUNK > 1/8" INTO SHEATHING). THE USE OF STAPLES IS PROHIBITED.
- 7. ALL EXTERIOR WALLS SHALL BE SHEATHED ON BOTH FACES WITH GYP-BOARD SHEATHING (SEE ARCH. DWGS. FOR THICKNESS) AND CONNECTED WITH 5d COOLER NAILS SPACED 7" O/C AT SUPPORTED PANEL EDGES AND INTERMEDIATE SUPPORTS.
- 8. PROVIDE 2x BLOCKING AT UNSUPPORTED PANEL EDGES AS FOLLOWS: ROOFS AND FLOORS ONLY WHERE INDICATED ON PLAN WALLS PER THE SHEAR WALL SCHEDULE ON SHEET S1.2.

### 10.0 WOOD FRAMING NOTES:

- ALL WOOD FRAMING MATERIAL SHALL BE SURFACED DRY AND USED AT 19% MAXIMUM MOISTURE
  CONTENT. ALLOWABLE STRESS REQUIREMENTS OF ALL MATERIAL SHALL BE IN ACCORDANCE WITH
  THE U RATING AS NOTED BELOW
- ALL STUD AND WALL FRAMING SHALL BE EITHER OF THE FOLLOWING:
   A. NO. 2 GRADE SOUTHERN YELLOW PINE (SYP)
- B. NO. 2 GRADE SOUTHERN YELLOW PINE (SY
- 3. ALL JOIST, RAFTER & MISC. FRAMING SHALL BE NO. 2 GRADE, SOUTHERN PINE. PROVIDE FULL-DEPTH (OR METAL) BRIDGING AT MIDSPAN AND AT A MAXIMUM SPACING OF 8'-0" O/C IN BETWEEN.
- 4. ALL FRAMING EXPOSED TO THE WEATHER OR IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE-TREATED IN ACCORDANCE WITH THE AMERICAN WOOD PRESERVERS ASSOCIATION SPECIFICATIONS. WHERE POSSIBLE, ALL CUTS AND HOLES SHOULD BE COMPLETED BEFORE TREATMENT. CUTS AND HOLES DUE TO ON-SITE FABRICATION SHALL BE BRUSHED WTIH 2 COATS OF COPPER NAPHTHENATE SOLUTION CONTAINING A MINIMUM OF 2% METALLIC COPPER IN SOLUTION (PER AWPA STD. M4).
- 5. THE CONTRACTOR SHALL CAREFULLY SELECT LUMBER TO BE USED IN LOADBEARING APPLICATIONS.
  THE LENGTH OF SPLIT ON THE WIDE FACE OF 2" NOMINAL LOADBEARING FRAMING SHALL BE LIMITED
  TO LESS THAN 1/2 OF THE WIDE FACE DIMENSION. THE LENGTH OF SPLIT ON THE WIDE FACE OF 3"
  (NOMINAL) AND THICKER LUMBER SHALL BE LIMITED TO 1/2 OF THE NARROW FACE DIMENSION.
- ALL NAILING NOT OTHERWISE INDICATED SHALL BE IN ACCORDANCE WITH THE "NAILING SCHEDULE"
   ON SHEET S1.1. NAILING SHALL NOT BE OVERDRIVEN.
- 7. PROVIDE DOUBLE JOISTS UNDER ALL PARTITIONS WHICH RUN PARALLEL WITH JOISTS AND UNDER ALL CONCENTRATED LOADS FROM FRAMING ABOVE.
- PROVIDE HEADER BEAMS OF THE SAME SIZE AS JOISTS OR RAFTERS TO FRAME AROUND OPENINGS
   IN THE PLYWOOD DECK UNLESS OTHERWISE INDICATED.
- 9. STRUCTURAL STEEL PLATE CONNECTORS SHALL CONFORM TO ASTM A-36 SPECIFICATIONS AND BE 1/4" THICK UNLESS OTHERWISE INDICATED. BOLTS CONNECTING WOOD MEMBERS SHALL BE PER ASTM A-307 AND BE 3/4" DIAMETER UNLESS OTHERWISE INDICATED. PROVIDE WASHERS FOR ALL

BOLT HEADS AND NUTS IN CONTACT WITH WOOD SURFACES.

- 10. BOLT HOLES SHALL BE CAREFULLY CENTERED AND DRILLED NOT MORE THAN 1/16" LARGER THAN THE BOLT DIAMETER. BOLTED CONNECTIONS SHALL BE SNUGGED TIGHT BUT NOT TO THE EXTENT OF CRUSHING WOOD UNDER WASHERS.
- 11. PREFABRICATED "MICRO-LAM" LUMBER HEADERS AND BEAMS SHALL BE AS MANUFACTURED BY "TRUSS JOIST MacMILLAN CORP.", BOISE, IDAHO OR APPROVED EQUAL. MICRO-LAM MATERIAL SHALL BE 2.0E, SOUTHERN PINE. DO NOT CUT OR NOTCH MICRO-LAM MATERIAL WITHOUT THE MANUFACTURER'S APPROVAL.
- 12. PREFABRICATED METAL JOIST HANGERS, HURRICANE CLIPS, HOLD-DOWN ANCHORS AND OTHER ACCESSORIES SHALL BE AS MANUFACTURED BY "SIMPSON STRONG-TIE COMPANY", (TEL.800-999-5099), OR APPROVED EQUAL. INSTALL ALL ACCESSORIES PER THE MANUFACTURER'S REQUIREMENTS. ALL STEEL SHALL HAVE A MINIMUM THICKNESS OF 0.04 INCHES (PER ASTM A446, GRADE A) AND BE GALVANIZED (COATING G60).
- 13. HOLES AND NOTCHES DRILLED OR CUT INTO WOOD FRAMING SHALL NOT EXCEED THE REQUIREMENTS OF N.C.B.C. 2018.
- 14. ALL PLATES, ANCHORS, NAILS, BOLTS, NUTS, WASHERS, AND OTHER MISCELLANEOUS HARDWARE SHALL BE HOT DIP GALVANIZED.

### 11.0 PRE-ENGINEERED WOOD TRUSS NOTES:

WOOD TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER TO SUPPORT THE FOLLOWING LOADS:
 MINIMUM GRAVITY LOADING:
 ROOF TRUSSES
 TOP CHORD LIVE LOAD: 20 PSF 40 PSF
 DEAD LOAD: 8 PSF 15 PSF

BOTTOM LIVE LOAD: \_\_\_\_\_
DEAD LOAD: 10 PSF

APPLICABLE PROVISIONS OF THE LATEST EDITION OF THE NATIONAL DESIGN SPECIFICATION
OF THE NATIONAL FOREST PRODUCTS ASSOCIATION, THE DESIGN SPECIFICATION FOR METAL PLATE
CONNECTED WOOD TRUSSES OF THE TRUSS PLATE INSTITUTE AND N.C.B.C 2303.4

3. WOOD MATERIALS SHALL BE SOUTHER PINE, DOUGLAS FIR OR LARCH AND SHALL BE KILN DRIED AND

WOOD TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH THE

- USED AT 19% MAXIMUM MOISTURE CONTENT. PROVIDE GRADE NO. 2 OR AS REQUIRED TO SATISFY STRESS REQUIREMENTS.
- 4. CONNECTOR PLATES SHALL BE NOT LESS THAN 0.036 INCHES (20 GAUGE) IN COATED THICKNESS, SHALL MEET OR EXCEED ASTM GRADE A OR HIGHER AND SHALL BE HOT DIPPED GALVANIZED ACCORDING TO ASTM A-525 (COATING G60). MINIMUM STEEL YIELD STRESS SHALL BE 33,000 PSI.
- 5. TRUSSES SHALL BE FABRICATED IN A PROPERLY EQUIPPED MANUFACTURING FACILITY OF A PERMANENT NATURE. TRUSSES SHALL BE MANUFACTURED BY EXPERIENCED WORKMEN, USING PRECISION CUTTING, JIGGING AND PRESSING EQUIPMENT UNDER THE REQUIREMENTS IN QUALITY CONTROL STANDARD QST-88 OF THE TRUSS PLATE INSTITUTE.
- 6. SECONDARY BENDING STRESSES IN TRUSS TOP AND BOTTOM CHORDS DUE TO DEAD, LIVE AND WIND LOADS SHALL BE CONSIDERED IN THE DESIGN. LOAD DURATION FACTORS SHALL BE PER THE "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION".
- 7. WOOD TRUSSES SHALL BE ERECTED IN ACCORDANCE WITH THE TRUSS MANUFACTURER'S REQUIREMENTS. THIS WORK SHALL BE DONE BY A QUALIFIED AND EXPERIENCED CONTRACTOR. TRUSS ERECTION BY AN INEXPERIENCED OR NON-QUALIFIED CONTRACTOR CAN RESULT IN CONSTRUCTION COLLAPSE AND/OR SERIOUS INJURY AND DAMAGE.
- 8. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY AND PERMANENT BRACING AS REQUIRED FOR SAFE ERECTION AND PERFORMANCE OF THE TRUSSES. THE GUIDELINES SET FORTH BY THE TRUSS PLATE INSTITUTE PUBLICATION "HIB-91, COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" SHALL BE A MINIMUM REQUIREMENT.
- 9. TRUSS MEMBERS AND COMPONENTS SHALL NOT BE CUT, NOTCHED, DRILLED NOR OTHERWISE ALTERED IN ANY WAY WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.
- 10. SUBMIT COMPLETE SHOP DRAWINGS FOR ALL WOOD TRUSSES SHOWING MEMBER SIZES, SPECIES, GRADE, MOISTURE CONTENT, SPAN, CAMBER, DIMENSIONS, CHORD PITCH, BRACING REQUIREMENTS AND LOADINGS. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER AND SHALL BEAR THE SEAL OF A PROFESSIONAL ENGINEER REGISTERED IN NORTH CAROLINA.

### 12.0 LAMINATED VENEER LUMBER (LVL) NOTES:

- SUBMIT MANUFACTURER'S DESCRIPTIVE LITERATURE INDICATING MATERIAL COMPOSITION,
   THICKNESS, DIMENSIONS, LOADING AND FABRICATION DETAILS.
- 2. SUBMIT MANUFACTURER'S LITERATURE INDICATING INSTALLATION DETAILS. INCLUDE LOCATIONS AND DETAILS OF BEARING, BLOCKING, BRIDGING AND CUTTING FOR WORK BY OTHERS.
- AND DETAILS OF BEARING, BLOCKING, BRIDGING AND CUTTING FOR WORK BY OTHERS

  3. LVL BASIS OF DESIGN IS PER 2.0E GP LAM HAVING THE FOLLOWING PROPERTIES.

A. QUALIFIED TO ASTM D 5456 BY APA-THE ENGINEERED WOOD ASSOCIATION.
B. MODULUS OF ELASTICITY E = 2.0 x 10 PSI
C. SHEAR MODULUS OF ELASTICITY G = 0.125 x 10 PSI
D. FLEXURAL STRESS Fb = 2,900 PSI
E. HORIZONTAL SHEAR Fv = 285 PSI

COMPRESSION PERP. TO GRAIN

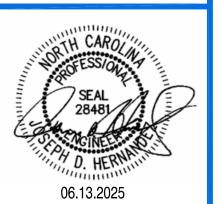
4. DELIVER MATERIALS TO THE JOB SITE IN MANUFACTURER'S ORIGINAL PACKAGING, CONTAINERS AND BUNDLES WITH MANUFACTURER'S IDENTIFICATION INTACT AND LEGIBLE.

Fc = 845 PSI

- STORE AND HANDLE MATERIALS TO PROTECT AGAINST CONTACT WITH DAMP AND WET SURFACES, EXPOSURE TO WEATHER, BREAKAGE AND DAMAGE. PROVIDE AIR CIRCULATION UNDER COVERING AND AROUND STACKS OF MATERIALS.
- EXCEPT FOR CUTTING TO LENGTH, GP LAM LVL BEAMS AND HEADERS SHALL NOT BE CUT, DRILLED OR NOTCHED, EXCEPT AS NOTED IN MANUFACTURER'S LITERATURE.
- 7. PROVIDE GP LAM LVL BEAMS AND HEADERS WHERE INDICATED ON DRAWINGS USING HANGERS AND ACCESSORIES SPECIFIED.
- INSTALL GP LAM LVL BEAMS AND HEADERS IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.

STRUCTURAL ENGINEERS, PL





SIGNATURE:

CLIENT:

The orchards at Naples Road, LLC 341 N main Street Hendersonville, NC 28792 Luis Graef: President



PERT
PROJECT:

Orchards at Naples Road
Apartment Complex
Jendersonville, North Carolina

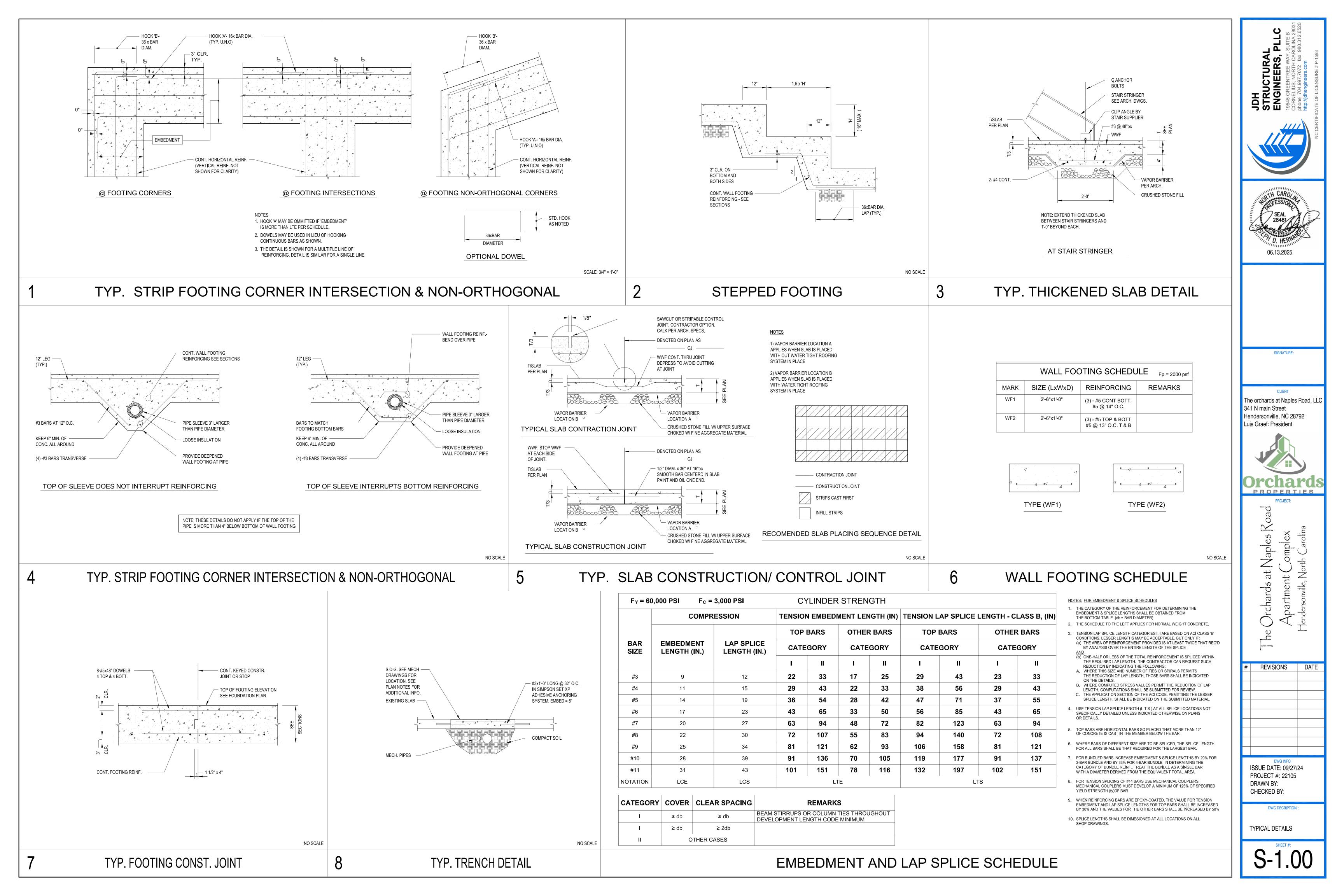
#	REVISIONS	DATE

ISSUE DATE: 09/27/24
PROJECT #: 22105
DRAWN BY:
CHECKED BY:

GENERAL NOTES

S-0.01

DWG DECRIPTION:



WALL STUD SCHEDULE			
MARK	LEVEL	STUDS	SPACING
W1	B - ROOF NOTES: (4) (5)	2 x 6 SPF NO. 2	24" O.C.
	B - 1	(3)2 x 4 SPF NO. 2	24" O.C.
W2	1 - 3	(2)2 x 4 SPF NO. 2	24" O.C.
	3 - ROOF	2 x 4 SPF NO. 2	24" O.C.
W3	1 - ROOF	2 x 4 SPF NO. 2	24" O.C.

- 2x6 STUDS WITH SAME SPECIES AND GRADE CAN BE SUBSTITUTED FOR 2x4 STUDS WHERE INDICATED ON ARCH DWGS. TO ACCOMMODATE M.E.P.
- SPF DENOTES SPRUCE-PINE-FIR. BRACE AT 1/3 POINTS.
- BRACE AT MID-POINT. NO ADDITIONAL BRACING REQUIRED WHERE WALL TYPE IS WITHIN A SHEAR

NAILED 16d COMMON NAILED 16d COMMON MAX. SPAN 2 ROWS AT 24" oc STAGGER AT 12" 2 ROWS AT 12" oc 3 ROWS AT 12" oc 2 ROWS AT 12" oc 2 ROWS AT 24" oc STAGGER AT 12" 2 ROWS AT 12" oc 22'-6" 3 ROWS AT 12" oc 2 ROWS AT 12" oc 2 ROWS AT 24" oc STAGGER AT 12" N/A

N/A

27'

2 ROWS AT 12" oc

FASTENING REQUIREMENTS FOR MULTIPLE MEMBERS

- TOP AND BOTTOM ROWS OF CONNECTORS SHALL BE 2" FROM EDGE BOLT HOLES ARE TO BE THE SAME DIAMETER AS THE BOLT. EVERY BOLT MUST
- BOLT HOLES ARE EXTEND THROUGH THE FULL THICKNESS OF THE MEMBER. USE WASHERS UNDER HEAD AND NUT. CARRIAGE BOLTS MAY BE USED, BUT THE OUTERMOST OF THE HEAD MAY NOT BE DRAWN IN BEYOND FLUSH WITH THE
- FOR THREE-PIECE MEMBER, SPECIFIED NAILING IS FROM EACH SIDE. FOUR-PLY MEMBERS, REGARDLESS OF DEPTH, MUST BE BOLTED.
- OUTSIDE FACE OF THE LVL MEMBER.

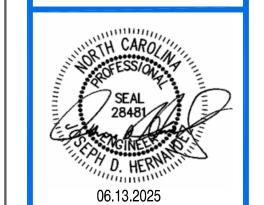
TYP. WOOD HEADER SCHEDULE 2x4 STUD WALLS				
WALL STUDS	ROUGH OPENING WIDTH "W"	G SIZE JAMB STUDS		
2x4	"W" ≦ 3'-0"	(2) 2x8 w/ 1/2" PLYWOOD PL	SINGLE	SINGLE
2x4	3'-0" < "W" < 6'-0"	(2) 2x8 w/ 1/2" PLYWOOD PL	SINGLE	DOUBLE
2x4	6'-0" < "W" < 9'-0"	(2) 2x10 w/ 1/2" PLYWOOD PL	DOUBLE	DOUBLE
2x4	9'-0" < "W" < 12'-0"	(2) 1 3/4" X 11" LVL	DOUBLE	TRIPLE
2x4	12'-0" > "W"	SEE SECTIONS	_	

TYP. WOOD HEADER SCHEDULE 2x6 STUD WALLS				
WALL STUDS	ROUGH OPENING	SIZE	JAMB :	STUDS
WALL GIODG	WIDTH "W"	OIZE	JACKS	KINGS
2x6	"W" ≦ 3'-0"	(3) 2x8 w/ 1/2" PLYWOOD PL	SINGLE	SINGLE
2x6	3'-0" < "W" < 6'-0"	(3) 2x8 w/ 1/2" PLYWOOD PL	SINGLE	DOUBLE
2x6	6'-0" < "W" < 9'-0"	(3) 2x10 w/ 1/2" PLYWOOD PL	DOUBLE	DOUBLE
2x6	9'-0" < "W" < 12'-0"	(3) 1 3/4" X 11" LVL	DOUBLE	TRIPLE
2x6	12'-0" > "W"	SEE SECTIONS	_	

SEE "LOOSE LINTEL" SCHEDULE FOR BRICK SHELF ANGLES

JOIST SCHEDULE		
MARK	MEMBER	SPACING
J1	2 x 10 SPF NO.2	16" OC

- WHERE DOUBLE JOISTS ARE REQUIRED, PROVIDE "TYPICAL POST" AT EACH END. SEE POST SCHEDULE ON \$1.00 SERIES SHEETS.
- 2x8 BLOCKING OR 2x12 BLOCKING @ 6'-0" O.C.



SIGNATURE:

The orchards at Naples Road, LL0

PROPERTIES

341 N main Street

Luis Graef: President

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Naples |

at

rchards

REVISIONS

DATE

Hendersonville, NC 28792

WALL STUD SCHEDULE

FASTENING REQUIREMENTS

MULTIPLE MEMBERS

WOOD HEADER SCHEDULE

JOIST SCHEDULE

### WOOD BEAM SCHEDULE POST MARK B1 (2)2x12 SPF NO.2 SEE PLAN B2 (2)1-3/4x11-7/8 LVL (2.0E) SEE PLAN В3 SEE PLAN (3)1-3/4x14 LVL (2.0E) (2)2x10 SPF NO.2 SEE PLAN

SEE WALL OPENING SCHEDULE. TYPICAL FOR OPENING FRAMING NOT SPECIFICALLY NOTED ON THE PLANS.

(3)1-3/4x11-7/8 LVL (2.0E)

SEE TYPICAL POST SCHEDULE & ELEVATION ON \$1.00 SERIES

SEE PLAN

3. IF NO POST IS SHOWN ON PLAN, USE (2) WALL STUDS UNDER

WOOD HEADER SCHEDULE			
ARK	SIZE	JACKS	KINGS
H1	(3)1-3/4x9-1/4 LVL (2.0E)	TRIPLE	DOUBLE

1. SEE WALL OPENING SCHEDULE. TYPICAL FOR OPENING

(3)2x12 SPF NO.2

# NOT USED

CONNECTION, LOCATION	NAIL	NUMBER OR SPACING
BAND JOIST TO SILL OR TOP PLATE, TOE NAIL	8d	6" o.c.
JOIST TO BAND JOIST, FACE NAIL	16d COMMON	3
JOIST TO SILL OR GIRDER, TOE NAIL	8d COMMON	3
BRIDGING TO JOIST, TOE NAIL EACH END	8d COMMON	2
LEDGER STRIP	16d COMMON	3 AT EACH JOIST
1x6 OR LESS SUBFLOOR TO EACH JOIST, FACE NAIL	8d COMMON	2
OVER 1x6 SUBFLOOR TO EACH JOIST, FACE NAIL	8d COMMON	3
2-INCH SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL	16d COMMON	2
SOLE PLATE TO JOIST OR BLOCKING, FACE NAIL	16d COMMON	16" O.C.
TOP OR SOLE PLATE TO STUD, END NAIL	16d COMMON	2
STUD TO SOLE PLATE, TOE NAIL	8d COMMON	4
DOUBLED STUDS, FACE NAIL	10d COMMON	24" O.C.
DOUBLED TOP PLATES, FACE NAIL	10d COMMON	16" O.C.
TOP PLATES, LAP AND INTERSECTIONS, FACE NAIL	16d COMMON	2
CONTINUOUS HEADER, TWO PIECES	16d COMMON	16" O.C. ALONG EDGE
CEILING JOISTS TO PLATE, TOENAIL	8d COMMON	3
CONTINUOUS HEADER TO STUD, TOENAIL	8d COMMON	4
CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL	16d COMMON	3
CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL	16d COMMON	3
RAFTER TO PLATE, TOE NAIL	8d COMMON	3
1-INCH BRACE TO EACH STUD AND PLATE, FACE NAIL	8d COMMON	2
WIDER THAN 1x8 SHEATHING TO EACH BEARING, FACE NAIL	8d COMMON	3
BUILT-UP CORNER STUDS	16d COMMON	24" O.C.
BUILT-UP GIRDER OR BEAMS, THREE MEMBERS	20d COMMON	32" O.C. TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES
2" PLANKS, AT EA. BEARING	16d COMMON	2
COLLAR TIE TO RAFTER, FACE NAIL	10d COMMON	3
JACK RAFTER TO HIP, TOE NAIL OR FACE NAIL	10d COMMON 16d COMMON	3 2
ROOF RAFTER TO 2-by RIDGE BEAM, TOE NAIL OR FACE NAIL	16d COMMON 16d COMMON	2 2
JOIST TO BAND JOIST, FACE NAIL	16d COMMON	3

WOOD POST SCHEDULE			
POST CONNECTION			NECTION
MARK	SIZE	BASE	CAP
P1	6x6 SPF NO. 2	PBS66	CC64/ECC64
P2	5.25x5.25 PSL 2.0E	PBS66	CC64/ECC64
P3	(3)2x6 SPF NO. 2		
P4	HSS3-1/2x3-1/2x3/16		
P5	4x4 SPF NO. 2	PBS44	CC44/ECC44
P6	3.5x3.5 PSL 2.0E	PBS44	CC44/ECC44
P7	3.5x5.25 PSL 2.0E	PBS46	CC46
P8	(3)2x4 SPF NO. 2		

ALL POSTS SHALL BE SPRUCE PINE FIR- NO. 2 OR BETTER USED TO BUILD UP STUDS SHALL BE 10d COMMON WIRE NAILS W/ MINIMUM DIAMETER= 0.148 IN. AND MINIMUM LENTGH= 3 IN.

WOOD BEAM SCHEDULE

WOOD HEADER SCHEDULE

NARROW ROOF SHEATHING PANEL REQUIREMENTS: IF WOOD SHEATHING PANEL IS: PROVIDE 2 CLIPS LOWER EDGE - BLOCK LOWER EDGE BLOCK LOWER EDGE

DOUBLE DOUBLE

LESS THAN 12": BLOCK UPPER EDGE - BLOCK LOWER EDGE NAIL SPACING -BOUNDARY NAILING – PER PLAN NOTES EDGES PER PLAN NOTES SUPPORTED EDGES (TYP.) NAIL @ 12" O.C.
ALONG INTERMEDIATE
FRAMING MEMBERS

(TYP) NAIL @ 12" O.C. ALONG-- FRAMING MINIMUM NAILING REQUIREMENTS PANEL ENDS - 6" oc INTERMEDIATE SUPPORTS - 12" oc EDGE DISTANCE = 3/8" CONTINUOUS PANEL-BOUNDARY NAILING -— CONTINUOUS PANEL JOINTS

STAGGER FASTENERS

TO MINIMIZE WOOD SPLITTING

PARALLEL TO WOOD GRAIN

DECKING LAYOUT @ BLOCKED DIAPHRAGMS

BOUNDARY NAILING SHALL INCLUDE NAILING OVER LENGTH OF SHEAR WALLS BOUNDARY NAILING SHALL INCLUDE NAILING ALONG ALL CONTINUOUS PANEL JOINTS BOUNDARY NAILING SHALL INCLUDE NAILING ENTIRE PERIMETER OF DECKING SPACE PANELS 1/8" MINIMUM FOR PANEL EXPANSION (ENDS AND EDGES)

DECKING LAYOUT @ UNBLOCKED DIAPHRAGMS

SUPPORTED EDGE NAILING SHALL INCLUDE NAILING OVER LENGTH OF SHEAR WALLS SUPPORTED EDGE NAILING SHALL INCLUDE NAILING ENTIRE PERIMETER OF DECKING SPACE PANELS 1/8" MINIMUM FOR PANEL EXPANSION (ENDS AND EDGES)

DWG DECRIPTION: TYPICAL DETAILS

ISSUE DATE: 09/27/24

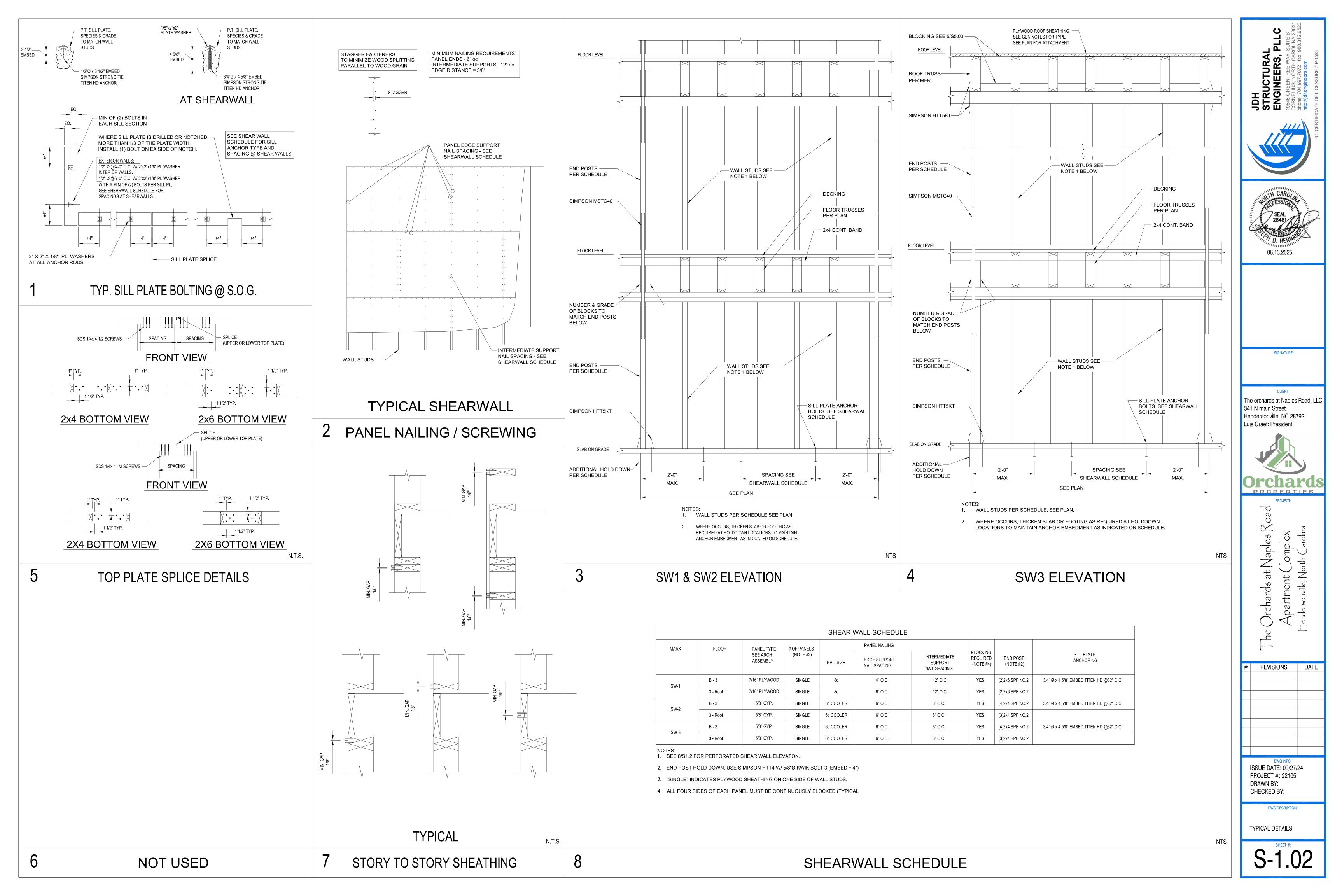
PROJECT #: 22105

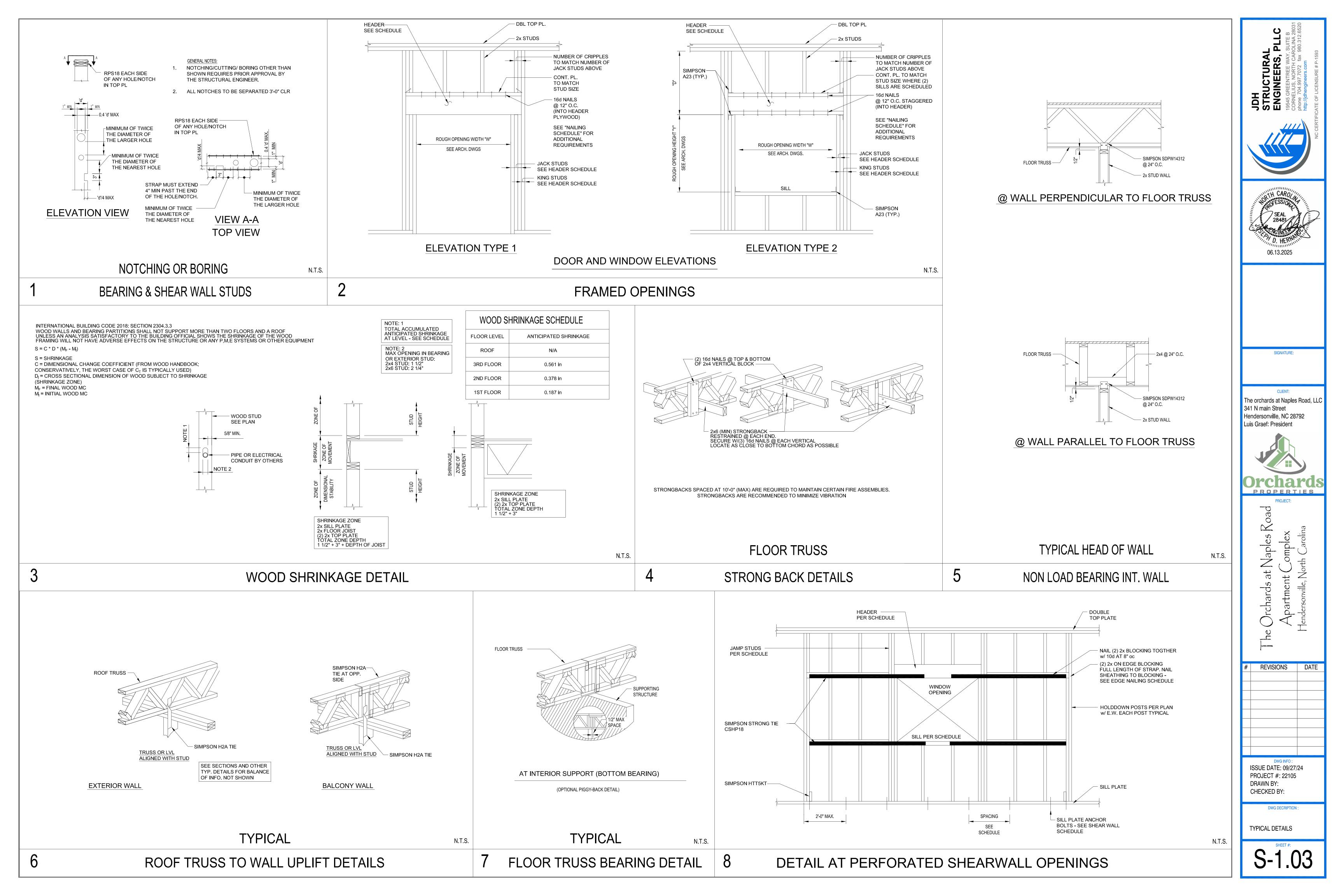
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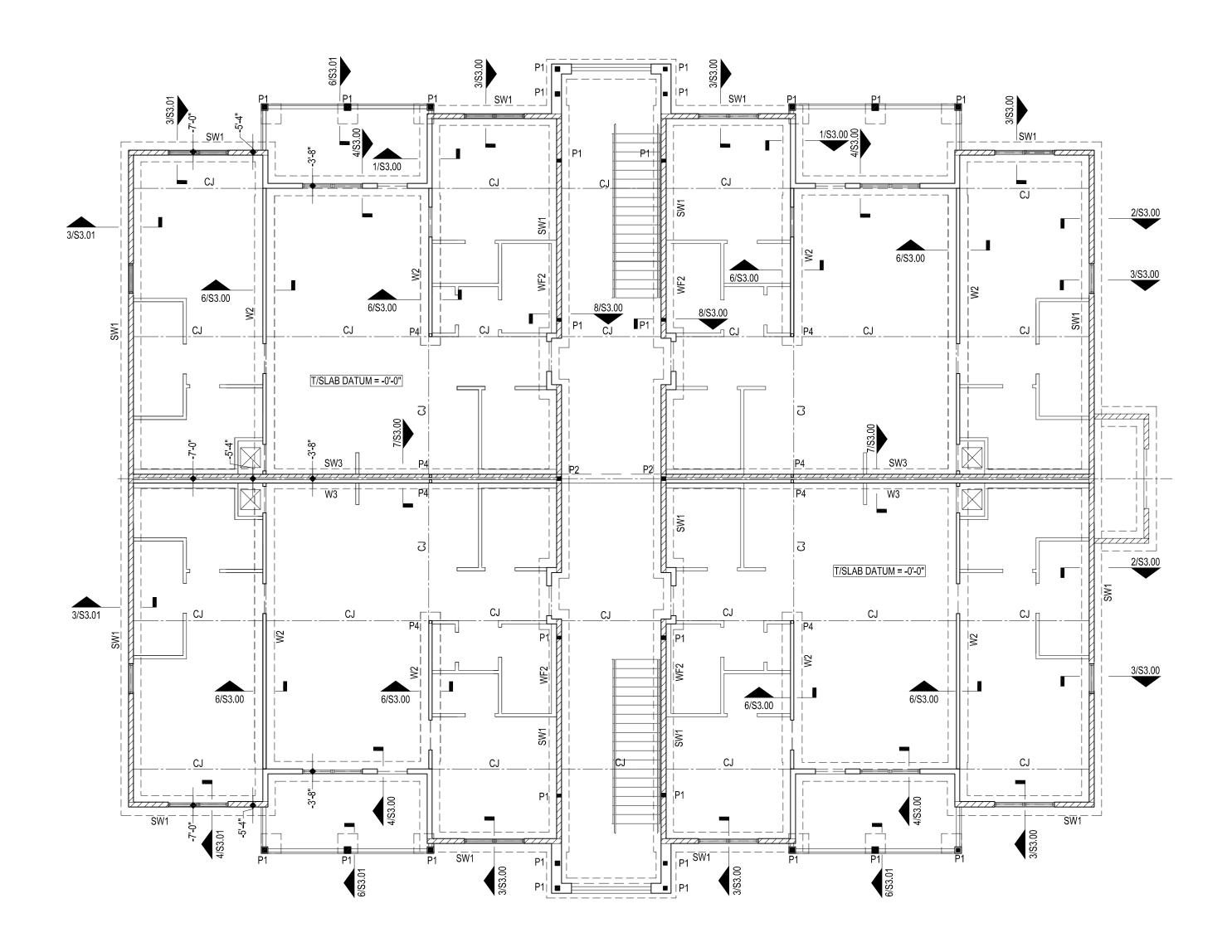
CHECKED BY:

POST SCHEDULE NAIL FASTENER SCHEDULE

TYPICAL DECKING LAYOUT DETAILS







# FIRST FLOOR FOUNDATION & FRAMING PLAN SCALE: 1/8" = 1'-0"

# LEGEND:

1. — INDICATES STEP IN FOOTING

2. INDICATES STEP IN FLOOR SLAB OR ROOF STRUCTURE

3. SW\* INDICATES SHEAR WALL LOCATION AND LABEL

4. \_\_\_\_\_ INDICATES LOAD BEARING WALL AND LABEL

5. INDICATES NON-STRUCTURAL WALL

6. 

INDICATES LOAD BEARING WALL BELOW

7. SIMDICATES WALL OPENING

8. — — INDICATES WALL OPENING BELOW

9. INDICATES DIRECTION OF FLOOR OR ROOF FRAMING

10. "Jx" INDICATES FLOOR JOIST LABEL. SEE JOIST SCHEDULE ON \$1.00 SERIES SHEETS

11. "Bx" INDICATES WOOD BEAM LABEL. SEE WOOD BEAM SCHEDULE ON \$1.00 SERIES SHEETS

12. "RJx" INDICATES ROOF JOIST LABEL. SEE JOIST SCHEDULE ON \$1.00 SERIES SHEETS

13. INDICATES WOOD POST LABEL, SEE S1.05 FOR SCHEDULE AND DETAIL.

14. MIDICATES CMU WALL

# FOUNDATION PLAN NOTES:

1. T/SL DATUM ELEVATION IS 0'-0". ELEVATIONS FOR FOOTINGS, SLABS, STEEL, WALLS, FLOORS, ELEVATOR PITS, ETC. ARE REFERENCED + OR - FROM DATUM ELEVATION (I.E. T/SL  $\pm$ 2'-6", T/W  $\pm$ 5'-3", T/STL  $\pm$ 6 1/4", ETC.).

2. T/FTG ELEVATIONS SHOWN ON PLAN ARE FOR STRIP AND SPREAD FOOTINGS. T/FTG ELEVATION AROUND PERIMETER SHALL BE -1'-4" U.N.O. WITH FOOTING STEPS SHOWN IN RELATIVE LOCATIONS. SEE \$1.00 SERIES SHEETS "TYPICAL DETAILS "FOR FOOTING STEP AND SPACING REQUIREMENTS.

3. TYPICAL SLAB ON GRADE (S.O.G.) IS 4" NORMAL WEIGHT CONCRETE REINFORCED WITH 6x6-W1.4xW1.4 WWF (FLAT SHEETS) ON 6" CRUSHED STONE BASE. SEE ARCHITECTURAL DRAWINGS FOR VAPOR BARRIER REQUIREMENTS. SEE S2.00 FOR SLAB CONTROL JOINT LAYOUT.

4. SUPPORT WWF AT 1" FROM TOP OF S.O.G. WITH SAND PLATES (CHAIRS WITH PLATE BASES) OR OTHER ACCEPTABLE DEVICES. BRICKS ARE NOT PERMITTED.

5. NO UNDERCUTTING AND BACKFILLING IS PERMITTED UNDER ANY FOOTING DUE TO HIGH ALLOWABLE BEARING PRESSURES USED IN FOOTING DESIGN. LEAN CONCRETE (f'c= 2000psi) OR FOOTING CONCRETE SHALL BE USED TO "BACKFILL" ANY OVEREXCAVATION.

6. CONTRACTOR SHALL SHORE ALL WALLS RECEIVING BACKFILL ON ONLY ONE SIDE OR RECEIVING UNEQUAL LEVELS OF BACKFILL ON OPPOSITE SIDES, UNLESS NOTED OTHERWISE IN THE DETAILS. ANY WALLS FOR WHICH SHORING IS INDICATED AS REQUIRED IN THE PLANS OR DETAILS SHALL BE SHORED REGARDLESS OF BACKFILL CONDITIONS.

7. MASONRY SHOWN ON STRUCTURAL DRAWINGS DEFINES ONLY THE EXTENT AND REQUIREMENTS OF MASONRY UTILIZED FOR STRUCTURAL PURPOSES (I.E. BEARING WALLS, SHEAR WALLS, RETAINING WALLS, FOUNDATION WALLS, COLUMNS, ETC.).

8. W1 TYP U.N.O., WF1 TYP U.N.O., SEE 8/S-1.02 FOR SHEARWALLS W/ OPENINGS.

9. ALL STUDS TO ALIGN W/ TRUSSES

10. DIMENSIONS SHOWN ON PLAN ARE TO CENTERLINE OF COLUMN OR CENTERLINE OF WALL U.N.O.

11. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND DRAWINGS OF OTHER DISCIPLINES FOR LOCATIONS AND DIMENSIONS OF OPENINGS, DEPRESSIONS, AND NON- STRUCTURAL MASONRY.

12. SEE S1.00 SERIES SHEETS FOR "GENERAL NOTES" AND FOR "TYPICAL DETAILS". TYPICAL DETAILS ARE GENERALLY NOT CUT ON PLANS BUT RATHER ARE INTENDED TO DEFINE TYPICAL CONSTRUCTION CONDITIONS. WHERE TYPICAL DETAILS ARE CUT IN PLAN, THE INTENT IS TO ILLUSTRATE THE TYPE OF CONDITION AT WHICH THAT DETAIL IS INTENDED TO APPLY RATHER THAN EVERY OCCURRENCE OF THAT DETAIL.

13. SEE S1.00 SERIES SHEETS FOR FOOTING SCHEDULE.

14. SEE S1.00 SERIES SHEETS FOR COLUMN SCHEDULE AND BASE PLATE DETAILS.

15. ALL STRUCTURAL WALLS SHOWN ARE TYPE " W1 " U.N.O.

16. FOOTINGS ARE NOTED ON PLAN WITH THE FOLLOWING DESIGNATIONS:

Fxx = FOOTING MARK PER SCHEDULE

T/FTG. = TOP OF FOOTING ELEVATION FROM DATUM

Cx = COLUMN MARK PER SCHEDULE

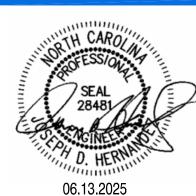
xxK = COLUMN LOAD (kips) (ASD)

CPx = CONCRETE PIER MARK PER SCHEDULE

T/P = TOP OF PIER ELEVATION FROM DATUM

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19545 GREENTRE WAY, SUITE B
CORNELIUS, NORTH CAROLINA 28031

NC CERTIFICAL



IGNATURE:

CLIENT:
The orchards at Naples Road, LLC
341 N main Street
Hendersonville, NC 28792
Luis Graef: President



PROJECT:

he Orchards at Naples Road
Apartment Complex
Hendersonville, North Carolina

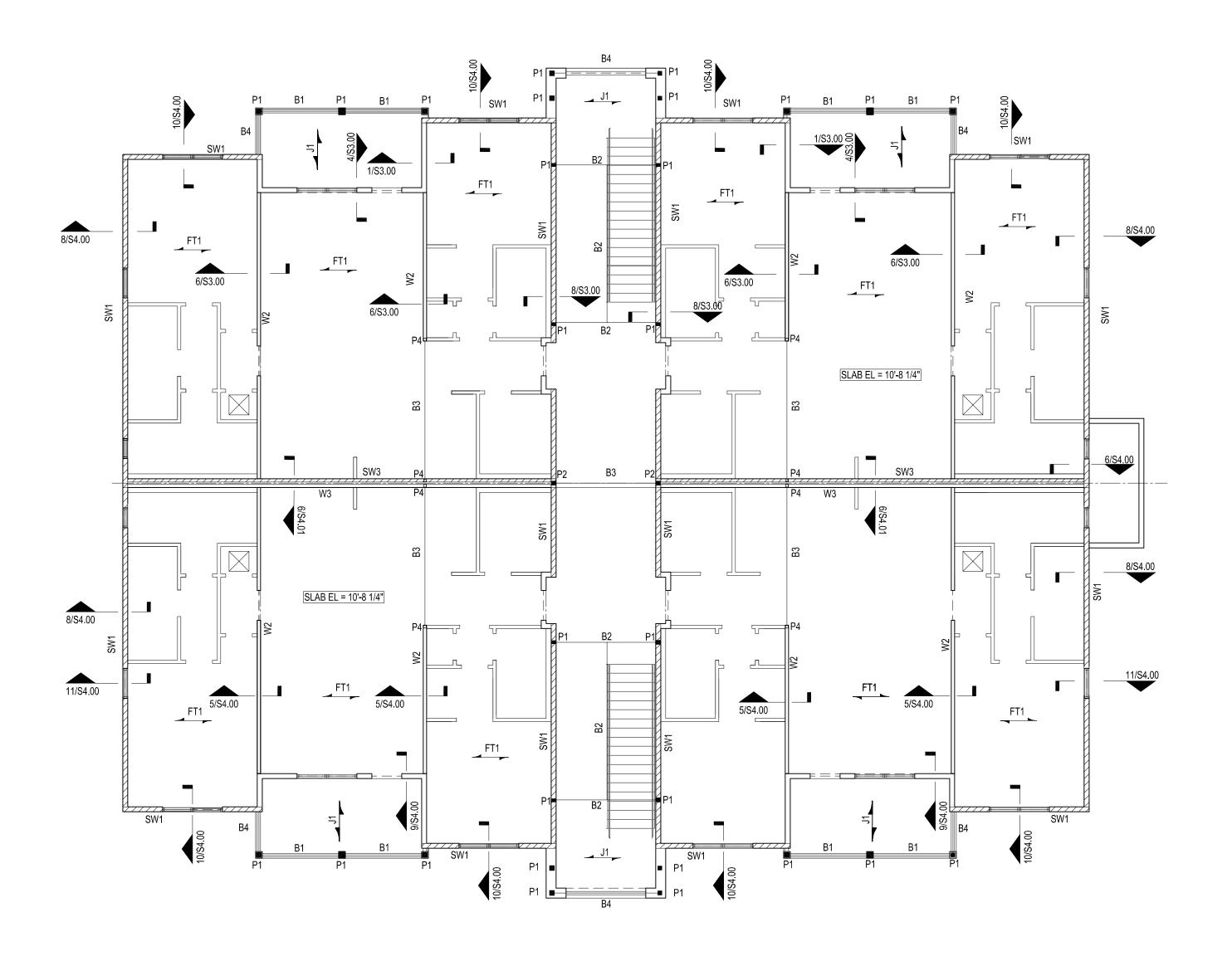
#	REVISIONS	DATE
	DWG INFO :	

ISSUE DATE: 09/27/24
PROJECT #: 22105
DRAWN BY:
CHECKED BY:

DWG DECRIPTION:

BUILDING 2 FIRST FLOOR FOUNDATION/FRAMING PLAN

S-2.01



# SECOND FLOOR FRAMING PLAN

SCALE: 1/8" = 1'-0"

# LEGEND:

1. → INDICATES STEP IN FOOTING

2. INDICATES STEP IN FLOOR SLAB OR ROOF STRUCTURE

3. SW\* INDICATES SHEAR WALL LOCATION AND LABEL

4. INDICATES LOAD BEARING WALL AND LABEL

5. INDICATES NON-STRUCTURAL WALL

6. \_ \_ \_ INDICATES LOAD BEARING WALL BELOW

7. — INDICATES WALL OPENING

8. — — INDICATES WALL OPENING BELOW

9. INDICATES DIRECTION OF FLOOR OR ROOF FRAMING

10. " Jx " INDICATES FLOOR JOIST LABEL. SEE JOIST SCHEDULE ON \$1.00 SERIES SHEETS

11. "Bx" INDICATES WOOD BEAM LABEL. SEE WOOD BEAM SCHEDULE ON S1.00 SERIES SHEETS

12. "RJx" INDICATES ROOF JOIST LABEL. SEE JOIST SCHEDULE ON \$1.00 SERIES SHEETS

ON \$1.00 SERIES SHEETS

13. SINDICATES WOOD POST LABEL, SEE \$1.05 FOR

SCHEDULE AND DETAIL.

FLOOR FRAMING PLAN NOTES:

1. SEE PLAN FOR FINISHED FLOOR ELEVATIONS FROM DATUM ELEVATION, U.N.O. ON PLAN AS (+X'-X") OR (-X'-X") AS REFERENCED FROM NOMINAL DATUM.

2. WOOD TRUSS FABRICATOR SHALL REFERENCE ARCHITECTURAL DRAWINGS FOR ADDITIONAL DIMENSIONAL INFORMATION.

E. WOOD INCOMPLETE ENERGY AND INCOMPLETE BY WINDOWN DITTORN & DIMENSION & IN COMPLETE

3. WHERE AVAILABLE, DIMENSIONS AND LOCATIONS FOR OPENINGS ARE SHOWN ON THE LOWEST LEVEL ON WHICH THE OPENING FIRST OCCURS AND ON SUBSEQUENT LEVELS WHERE DIMENSIONS OR LOCATIONS VARY.

4. SHEATH REMAINING LENGTH OF WALL (BEYOND MIN. SHEAR WALL LENGTH AS PER SCHEDULE) WITH EQUIVALENT NON-SHEAR WALL SHEATHING THICKNESS AND GWB PER ARCH. DWGS. ATTACH NON-SHEAR WALL SHEATHING WITH 10d NAILS @ 16" O.C.

5. TYPICAL FLOOR DECK OVER WOOD SUPPORT SHALL BE 3/4" TONGUE AND GROOVED EXPOSURE 1, OSB OR PLYWOOD SHEATHING. ATTACH PER GENERAL NOTES ON \$1.00 SERIES SHEETS, WOOD FRAMING NOTES U.N.O. ON DRAWINGS.

6. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND DRAWINGS OF OTHER DISCIPLINES FOR LOCATIONS AND DIMENSIONS OF OPENINGS, DEPRESSIONS, AND NON-STRUCTURAL MASONRY.

7. SEE S1.00 SERIES SHTS. FOR "GENERAL NOTES" AND FOR "TYPICAL DETAILS". TYPICAL DETAILS ARE GENERALLY NOT CUT ON PLANS BUT RATHER ARE INTENDED TO DEFINE TYPICAL CONSTRUCTION CONDITIONS. WHERE TYPICAL DETAILS ARE CUT IN PLAN, THE INTENT IS TO ILLUSTRATE THE TYPE OF CONDITION AT WHICH THAT DETAIL IS INTENDED TO APPLY RATHER THAN EVERY OCCURRENCE OF THAT DETAIL.

8. SEE S1.00 SERIES SHTS. FOR SHEAR WALL INFORMATION.

9. SEE S1.00 SERIES SHEETS FOR WOOD JOIST AND ALL BEAM SCHEDULES.

10. SEE S1.00 SERIES SHEETS FOR BEARING WALL SCHEDULES.

11. DIMENSIONS SHOWN ON PLAN ARE TO CENTERLINE OF COLUMN OR CENTERLINE OF WALL U.N.O.

12. ALL STRUCTURAL WALLS SHOWN ARE TYPE "W1" U.N.O.

13. SEE 3/S1.03 FOR TYPICAL DECKING LAYOUT DETAILS, USE UNBLOCKED DIAPHRAGM U.N.O. USE BLOCKED DIAPHRAGM FOR SHADED AREA.

USE 10d NAILS WITH 2" BOUNDARY SPACING IN BOTH DIRECTIONS.

14. INDICATES WOOD POST LABEL, SEE S1.05 FOR SCHEDULE AND DETAIL.

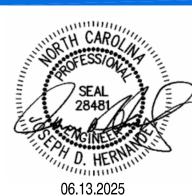
15. SWX INDICATES SHEARWALL LOCATION AND LABEL

16. ALL STUDS TO ALIGN W/ TRUSSES

17. FT1 INDICATES 18" DEEP FLOOR TRUSS @ 24" O.C. BY OTHERS

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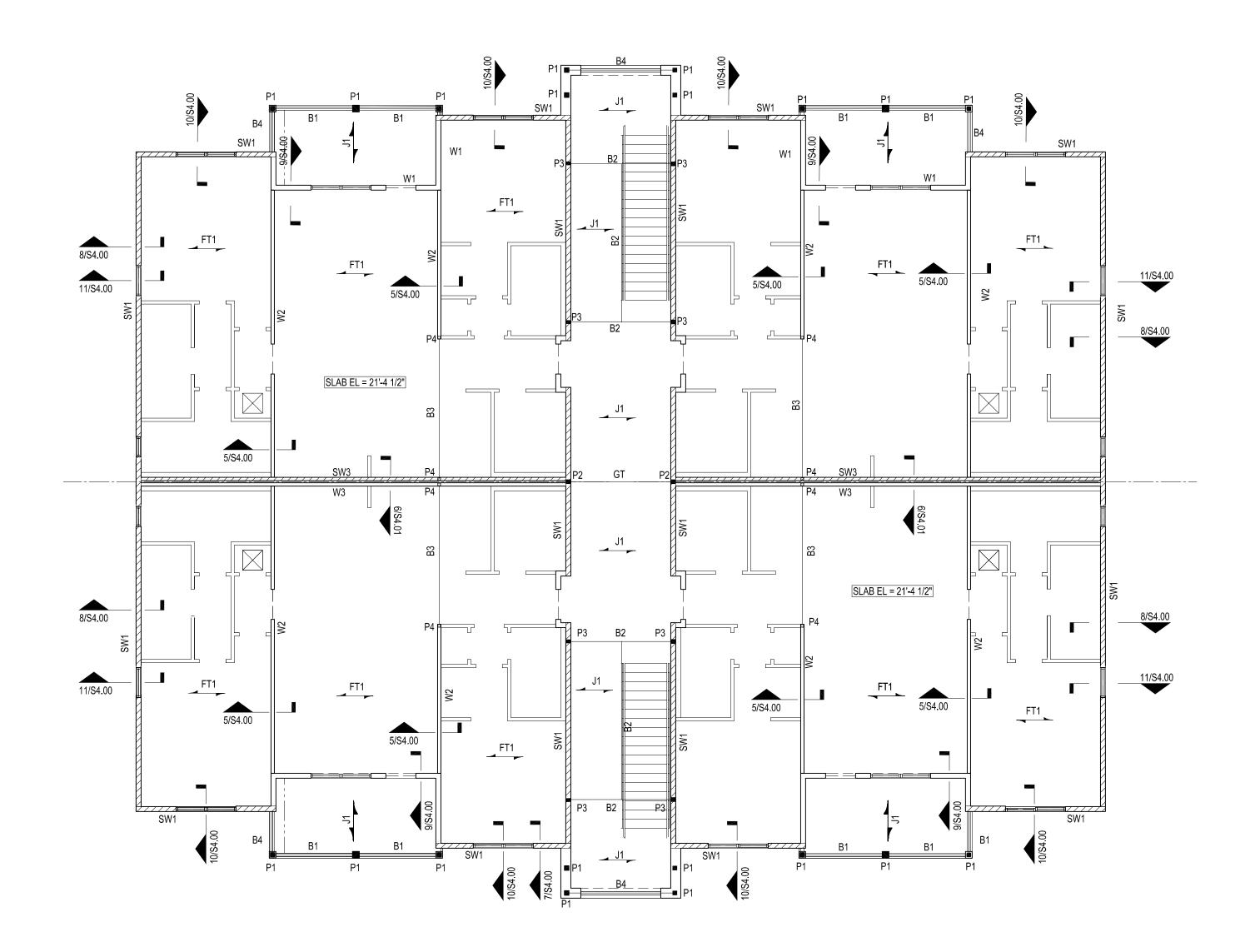
#	REVISIONS	DATE
DWG INFO :		

ISSUE DATE: 09/27/24
PROJECT #: 22105
DRAWN BY:
CHECKED BY:

DWG DECRIPTION:

BUILDING 2 SECOND FLOOR FRAMING PLAN

S-2.02



# THIRD FLOOR FRAMING PLAN

SCALE: 1/8" = 1'-0"

# LEGEND:

1. — INDICATES STEP IN FOOTING

2. INDICATES STEP IN FLOOR SLAB OR ROOF STRUCTURE

3. SW\*
INDICATES SHEAR WALL LOCATION AND LABEL

W\*
INDICATES LOAD BEARING WALL AND LABEL

5. INDICATES NON-STRUCTURAL WALL

6. 

INDICATES LOAD BEARING WALL BELOW

7. 🗀 🖂 INDICATES WALL OPENING

7. INDICATES WALL OPENING

10. "Jx" INDICATES FLOOR JOIST LABEL. SEE JOIST SCHEDULE ON \$1.00 SERIES SHEETS

11. "Bx" INDICATES WOOD BEAM LABEL. SEE WOOD BEAM SCHEDULE ON \$1.00 SERIES SHEETS

12. "RJx" INDICATES ROOF JOIST LABEL. SEE JOIST SCHEDULE ON \$1.00 SERIES SHEETS

13. SINDICATES WOOD POST LABEL, SEE S1.05 FOR SCHEDULE AND DETAIL.

# FLOOR FRAMING PLAN NOTES:

1. SEE PLAN FOR FINISHED FLOOR ELEVATIONS FROM DATUM ELEVATION, U.N.O. ON PLAN AS (+X'-X") OR (-X'-X") AS REFERENCED FROM NOMINAL DATUM.

2. WOOD TRUSS FABRICATOR SHALL REFERENCE ARCHITECTURAL DRAWINGS FOR ADDITIONAL DIMENSIONAL INFORMATION.

3. TYPICAL ROOF DECK OVER WOOD SUPPORT SHALL BE 3/4" TONGUE AND GROOVED EXPOSURE 1, OSB OR PLYWOOD

SHEATHING. ATTACH PER GENERAL NOTES ON S1.00 SERIES SHEETS, WOOD FRAMING NOTES U.N.O. ON DRAWINGS.

4. WHERE AVAILABLE, DIMENSIONS AND LOCATIONS FOR OPENINGS ARE SHOWN ON THE LOWEST LEVEL ON WHICH THE OPENING FIRST OCCURS AND ON SUBSEQUENT LEVELS WHERE DIMENSIONS OR LOCATIONS VARY.

5. SHEATH REMAINING LENGTH OF WALL (BEYOND MIN. SHEAR WALL LENGTH AS PER SCHEDULE) WITH EQUIVALENT NON-SHEAR WALL SHEATHING THICKNESS AND GWB PER ARCH. DWGS. ATTACH NON-SHEAR WALL SHEATHING WITH 10d NAILS @ 16" O.C.

6. TYPICAL FLOOR DECK OVER WOOD SUPPORT SHALL BE 3/4" TONGUE AND GROOVED EXPOSURE 1, OSB OR PLYWOOD SHEATHING. ATTACH PER GENERAL NOTES ON \$1.00 SERIES SHEETS, WOOD FRAMING NOTES U.N.O. ON DRAWINGS.

7. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND DRAWINGS OF OTHER DISCIPLINES FOR LOCATIONS AND DIMENSIONS OF OPENINGS, DEPRESSIONS, AND NON-STRUCTURAL MASONRY.

8. SEE S1.00 SERIES SHTS. FOR "GENERAL NOTES" AND FOR "TYPICAL DETAILS". TYPICAL DETAILS ARE

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9. SEE S1.00 SERIES SHTS. FOR SHEAR WALL INFORMATION.

10. SEE S1.00 SERIES SHEETS FOR WOOD JOIST AND ALL BEAM SCHEDULES.

11. SEE S1.00 SERIES SHEETS FOR BEARING WALL SCHEDULES.

12. DIMENSIONS SHOWN ON PLAN ARE TO CENTERLINE OF COLUMN OR CENTERLINE OF WALL U.N.O.

13. ALL STRUCTURAL WALLS SHOWN ARE TYPE "W1" U.N.O.

14. SEE 3/S1.03 FOR TYPICAL DECKING LAYOUT DETAILS, USE UNBLOCKED DIAPHRAGM U.N.O. USE BLOCKED DIAPHRAGM FOR SHADED AREA.

USE 10d NAILS WITH 2" BOUNDARY SPACING IN BOTH DIRECTIONS.

15. INDICATES WOOD POST LABEL, SEE S1.05 FOR SCHEDULE AND DETAIL.

16. SWX INDICATES SHEARWALL LOCATION AND LABEL

17. ALL STUDS TO ALIGN W/ TRUSSES

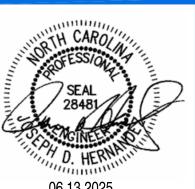
18. FT1 INDICATES 18" DEEP FLOOR TRUSS @ 24" O.C. BY OTHERS

19. W1 TYP U.N.O., WF1 TYP U.N.O., SEE 8/S-1.02 FOR SHEARWALLS W/ OPENINGS.

20. GT INDICATES GIRDER TRUSS

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CORNELIUS, NORTH CAROLINA 28031

NC CERTIFICAT



GNATURE:

CLIENT:
The orchards at Naples Road, LLC
341 N main Street
Hendersonville, NC 28792
Luis Graef: President



PER

Orchards at Naples Road
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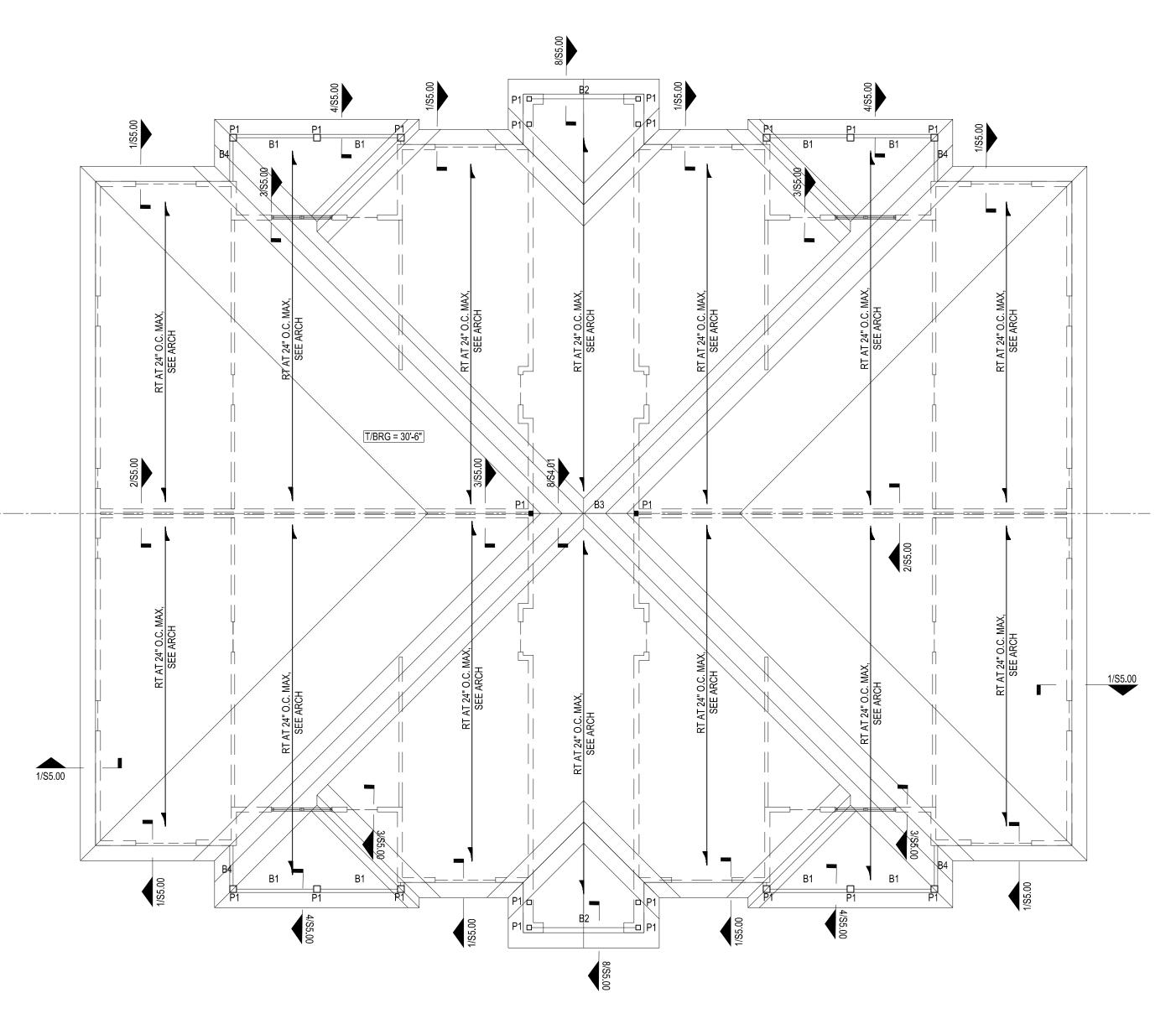
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ISSUE DATE: 09/27/24
PROJECT #: 22105
DRAWN BY:
CHECKED BY:

DWG DECRIPTION:

BUILDING 2 THIRD FLOOR FRAMING PLAN

S-2.03



# ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

# LEGEND:

1. → INDICATES STEP IN FOOTING

2. INDICATES STEP IN FLOOR SLAB OR ROOF STRUCTURE

4. \_\_\_\_\_ INDICATES LOAD BEARING WALL AND LABEL

5. INDICATES NON-STRUCTURAL WALL

6. \_ \_ \_ INDICATES LOAD BEARING WALL BELOW

7. 

INDICATES WALL OPENING

8. — — INDICATES WALL OPENING BELOW

9. INDICATES DIRECTION OF FLOOR OR ROOF FRAMING

10. "Jx" INDICATES FLOOR JOIST LABEL. SEE JOIST SCHEDULE ON S1.00 SERIES SHEETS

11. "Bx" INDICATES WOOD BEAM LABEL. SEE WOOD BEAM SCHEDULE ON \$1.00 SERIES SHEETS

12. "RJx" INDICATES ROOF JOIST LABEL. SEE JOIST SCHEDULE ON S1.00 SERIES SHEETS

13. SINDICATES WOOD POST LABEL, SEE S1.05 FOR SCHEDULE AND DETAIL.

# ROOF FRAMING PLAN NOTES:

1. SEE PLAN FOR TRUSS BEARING ELEVATIONS FROM DATUM ELEVATION, U.N.O. ON PLAN AS (+X'-X") OR (-X'-X") AS REFERENCED FROM NOMINAL DATUM.

2. WOOD TRUSS FABRICATOR SHALL REFERENCE ARCHITECTURAL DRAWINGS FOR ADDITIONAL DIMENSIONAL INFORMATION.

3. TYPICAL ROOF DECK OVER WOOD SUPPORT SHALL BE 3/4" TONGUE AND GROOVED EXPOSURE 1, OSB OR PLYWOOD

4. WHERE AVAILABLE, DIMENSIONS AND LOCATIONS FOR OPENINGS ARE SHOWN ON THE LOWEST LEVEL ON WHICH

SHEATHING. ATTACH PER GENERAL NOTES ON \$1.00 SERIES SHEETS, WOOD FRAMING NOTES U.N.O. ON DRAWINGS.

THE OPENING FIRST OCCURS AND ON SUBSEQUENT LEVELS WHERE DIMENSIONS OR LOCATIONS VARY.

5. SHEATH REMAINING LENGTH OF WALL (BEYOND MIN. SHEAR WALL LENGTH AS PER SCHEDULE) WITH EQUIVALENT NON-SHEAR WALL SHEATHING THICKNESS AND GWB PER ARCH. DWGS. ATTACH NON-SHEAR WALL SHEATHING WITH 10d NAILS @ 16" O.C.

6. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND DRAWINGS OF OTHER DISCIPLINES FOR

LOCATIONS AND DIMENSIONS OF OPENINGS, DEPRESSIONS, AND NON-STRUCTURAL MASONRY.

7. SEE S1.00 SERIES SHTS. FOR "GENERAL NOTES" AND FOR "TYPICAL DETAILS". TYPICAL DETAILS ARE GENERALLY NOT CUT ON PLANS BUT RATHER ARE INTENDED TO DEFINE TYPICAL CONSTRUCTION CONDITIONS. WHERE TYPICAL DETAILS ARE CUT IN PLAN, THE INTENT IS TO ILLUSTRATE THE TYPE OF CONDITION AT WHICH THAT DETAIL IS INTENDED TO APPLY RATHER THAN EVERY OCCURRENCE OF THAT DETAIL.

8. SEE S1.00 SERIES SHTS. FOR SHEAR WALL INFORMATION.

9. SEE S1.00 SERIES SHEETS FOR WOOD JOIST AND ALL BEAM SCHEDULES.

10. SEE S1.00 SERIES SHEETS FOR BEARING WALL SCHEDULES.

11. DIMENSIONS SHOWN ON PLAN ARE TO CENTERLINE OF WALL U.N.O.

12. W1 TYP U.N.O., WF1 TYP U.N.O., SEE 8/S-1.02 FOR SHEARWALLS W/ OPENINGS.

13. SEE 3/S1.03 FOR TYPICAL DECKING LAYOUT DETAILS, USE UNBLOCKED DIAPHRAGM U.N.O.

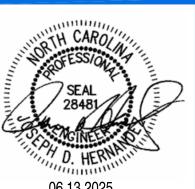
14. INDICATES WOOD POST LABEL, SEE S1.05 FOR SCHEDULE AND DETAIL.

15. MAU-1 AND MAU-2 DENOTES ROOF TOP UNITS, SEE MECH. DRAWINGS FOR LOCATION AND EQUIPMENT WEIGHT. JOIST SUPPLIER TO DESIGN TO SUIT. SEE S1.00 FOR ROOF LOADS.

16. SEE 1/S1.05 FOR WALL OPENINGS

17. ALL STUDS TO ALIGN W/ TRUSSES

18. FT1 INDICATES 18" DEEP FLOOR TRUSS @ 24" O.C. BY OTHERS



The orchards at Naples Road, LLC 341 N main Street Hendersonville, NC 28792 Luis Graef: President



Naples

#	REVISIONS	DATE
DWG INFO :		

ISSUE DATE: 09/27/24 PROJECT #: 22105 DRAWN BY: CHECKED BY:

DWG DECRIPTION:

**BUILDING 2 ROOF** FRAMING PLAN

