STRUCTURAL NOTES

GENERAL NOTES:

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

ALL DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK. DO NOT SCALE DRAWINGS.

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING. SHEETING. TEMPORARY BRACING. GUYS OR TIEDOWNS. ELECTRONIC VERSIONS OF THE STRUCTURAL DRAWINGS ARE THE SOLE. COPYRIGHTED PROPERTY OF SNELL ENGINEERING AND ARE NOT

TO BE USED OR TRANSFERRED WITHOUT THE EXPRESS, WRITTEN PERMISSION OF SNELL ENGINEERING.

THE STRUCTURAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE - BUILDING 7TH EDITION (2020). THE FOLLOWING SUPERIMPOSED LOADINGS HAVE BEEN UTILIZED:

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20 PSF.
      LIVE LOAD
      LIVE LOAD (CONCENTRATED)

    300 LBS.

      DEAD LOAD

    25 PSF.

      DEAD LOAD (AVAILABLE TO RESIST UPLIFT)
                                                        10 PSF.
                                                        SEE PLAN FOR WEIGHTS, INCLUDE
      MECH. EQUIPMENT
                                                         10 PSF ADDITIONAL DL OVER THE
                                                         UNIT FOOTPRINT
STAIRS:
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100 PSF. LIVE LOAD LIVE LOAD (CONCENTRATED) 300 LBS. OVER 2"X2" AREA **DWELLING LIVING AREAS**

DEAD LOAD 40 PSF(INCLUDES 10 PSF PARTITION WALL LOAD.) BALCONY DEAD LOAD 60 PSF BALCONY LIVE LOAD 60 PSF

FIRST FLOOR LIVE LOAD ABOVE 1ST FLOOR LIVE LOAD (LL REDUCTION USED) -40 PSF.

LIVE LOAD

FLOORS AND OTHER SIMILAR SURFACES SHALL BE DESIGNED TO SUPPORT THE UNIFORMLY DISTRIBUTED LIVE LOADS OR CONCENTRATED LIVE LOADS SHOWN ABOVE, WHICHEVER PRODUCES THE GREATER LOAD EFFECT. DISTRIBUTED LOADS AND CONCENTRATED LOADS ARE NON-CONCURRENT UNLESS NOTED OTHERWISE.

40 PSF.

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ASCE 7-16
ULTIMATE WIND SPEED
                                         - 152 MPH
ALLOWABLE WIND SPEED
                                         - 117 MPH
EXPOSURE C
ENCLOSED STRUCTURE
INTERNAL PRESSURE COEFFICIENT
                                                +/- 0.18
RISK FACTOR II
SEE WIND SCHEDULE FOR PRESSURES
FEMA X ZONE
FLOOD DESIGN CLASS
BFE ELEVATION
                                                 11.00 NAVD
FRFFBOARD
PROPOSED LOWEST FLOOR ELEVATION
BOT. OF LOWEST HORIZ. STRUCT. MEMBER
                                                 12.00 NAVD
REQ'D MIN. DRY FLOOD-PROOF ELEVATION
                                                 12.00 NAVD
STRUCTURAL SYSTEMS OF BUILDINGS AND STRUCTURES ARE DESIGNED, CONNECTED AND ANCHORED TO RESIST
FLOTATION, COLLAPSE OR PERMANENT LATERAL MOVEMENT DUE TO STRUCTURAL LOADS AND STRESSES FROM
FLOODING EQUAL TO THE DESIGN FLOOD ELEVATION IN CONFORMANCE WITH ASCE 24-14 AND FBC-R 322.1.2/FBC 1612.1.
RISK CATEGORY
SEISMIC IMPORTANCE FACTOR le
SITE CLASS D
SEISMIC DESIGN CATEGORY A
Ss 0.048 g
Sds 0.051 g
                    Sd1 0.038 g
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IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO CONFIRM ALL FINAL WEIGHTS OF EQUIPMENT (MECHANICAL, CHILLED WATER, KITCHEN, SPA/POOL, ETC.), CLADDING, FINISHES, ETC. PRIOR TO ERECTING STRUCTURE SUPPORTING THESE ITEMS. ALL WEIGHTS ACCOUNTED FOR IN THE STRUCTURAL DESIGN ARE LISTED ABOVE OR ON THE PLANS.

SHOP DRAWING REVIEW:

SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC.

ANY COMPONENT NOTED AS "DELEGATED" SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF FLORIDA AND NOT BY THE SER. SIGNED AND SEALED DRAWINGS SHALL BE PROVIDED TO THE ARCHITECT AND SEOR FOR REVIEW AS A SHOP DRAWING; CALCULATIONS WILL BE PROVIDED IF REQUESTED.

ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW NOTATION WILL BE RETURNED UNCHECKED. EVERY EFFORT WILL BE MADE TO RETURN THE SHOP DRAWINGS WITHIN TEN BUSINESS DAYS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO SUBMIT THE SHOP DRAWINGS ALLOWING FOR AN ADEQUATE REVIEW PERIOD.

ONE SET OF PRINTS WILL BE RETAINED BY THE ENGINEER AND ONE BY THE ARCHITECT, THE CONTRACTOR SHALL RECEIVE THE REMAINING PRINTS FOR SUBMITTAL TO THE BUILDING DEPARTMENT AND AS REQUIRED FOR DISTRIBUTION.

FOR INFORMATION (RFI) OR SIMILAR DOCUMENTATION BY THE ENGINEER. EVERY EFFORT WILL BE MADE TO RETURN THE RFIS WITHIN TWO BUSINESS DAYS. SUBMITTALS SHALL CLEARLY IDENTIFY THE SPECIFIC PROJECT, APPLICABLE CODES AND DESIGN CRITERIA, AND DETAILING OF ALL COMPONENTS NECESSARY TO ENSURE PROPER INSTALLATION OF THE COMPONENTS AND SYSTEM. SHOP DRAWINGS SHOULD BE SUBMITTED FOR ALL COMPONENTS OF THE STRUCTURAL FRAMING SYSTEM, AS REQUIRED BY THE ARCHITECT,

IN ALL INSTANCES THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN A REQUEST

AND AS NOTED ELSEWHERE IN THESE NOTES, INCLUDING, BUT NOT LIMITED TO: CONCRETE MIX DESIGNS MASONRY BLOCK MASONRY BLOCK ACCESSORIES MASONRY REINFORCING

CONCRETE REINFORCEMENT STRUCTURAL STEEL (INCLUDING ANCHOR BOLTS) PRF-FNGINFFRFD WOOD TRUSSES POST-TENSIONED CABLES POST-TENSIONED SLAB ON GRADE DESIGN

PRE-ENGINEERED STEEL STAIRS ANY PROPOSED MANUFACTURER CHANGE FROM THE BASIS OF DESIGN

IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE ALL TRADES AND CONSULTANTS, CROSS REFERENCE THEIR DRAWINGS WITH THE OVERALL DESIGN, AND PROVIDE TO EACH A COMPLETE SET OF DRAWINGS AND SUBMITTALS TO ENSURE COMPATIBILITY OF CONSTRUCTION PER DESIGN INTENT.

FOUNDATIONS:

ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2,000 PSF ON COMPACTED FILL OR COMPACTED NATIVE SOIL. BEFORE CONSTRUCTION COMMENCES, SOIL BEARING CAPACITY SHALL BE VERIFIED BY A SUBSURFACE INVESTIGATION, AS WELL AS FIELD AND LABORATORY TESTS PERFORMED BY A CERTIFIED TESTING LABORATORY, WHOSE REPORT SHALL INCLUDE ANALYSIS AND RECOMMENDATIONS FOR SITE PREPARATION IN ORDER TO BEAR THE FOUNDATION LOADS. ABOVE REPORT SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW BEFORE FOUNDATION CONSTRUCTION BEGINS.

SOIL COMPACTION:

FOUNDATIONS ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE AS SPECIFIED ABOVE. SOIL COMPACTION SHALL MEET THE MORE STRINGENT OF THE CRITERIA LISTED BELOW OR AS SPECIFIED IN THE GEOTECHNICAL REPORT. REMOVE TOP-SOIL TO A MINIMUM DEPTH OF ONE FOOT OVER THE ENTIRE BUILDING AREA AND FIVE FEET BEYOND BUILDING LINES. COMPACT SUB-GRADE USING A VIBRATORY COMPACTER SUCH AS "VIBRATOW II" OR EQUIVALENT WITH A MINIMUM OF FOUR PASSES. PLACE AND COMPACT FILL MATERIAL TO FINISHED GRADE LEVEL IN LIFTS NOT EXCEEDING 12" THICK. SUB-GRADE AND EACH LIFT SHALL BE COMPACTED TO MINIMUM 95% MODIFIED PROCTOR DENSITY DETERMINED IN ACCORDANCE WITH ASTM. D-1557. VERIFICATION THAT THE COMPACTION REQUIREMENTS HAVE BEEN MET SHALL BE MADE BY AN INDEPENDENT GEOTECHNICAL CONSULTANT EMPLOYED BY THE OWNER AND APPROVED BY THE ENGINEER. LOCATIONS FAILING TO MEET THE REQUIREMENTS SHALL BE RECOMPACTED AND RETESTED AT THE CONTRACTORS EXPENSE AND AS DIRECTED BY THE INDEPENDENT GEOTECHNICAL CONSULTANT.

PENETRATIONS:

NO PENETRATIONS SHALL BE MADE IN ANY STRUCTURAL MEMBERS WITHOUT PREVIOUS APPROVAL OF THE EOR, EXCEPT THOSE PENETRATIONS SHOWN ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHOULD SUBMIT DRAWINGS SHOWING: ANY LOCATION WHERE OPENINGS, PENETRATIONS, OR ANY PLACE MORE THAN 3 PIPES OR CONDUITS ARE LOCATED IN A SLAB SYSTEM; ANY LOCATION IN A BEAM OR COLUMN WHERE MORE THAN 4% OF THE MEMBER SECTION IS REMOVED IN ANY ORIENTATION.

PLUMBING SLEEVES:

MINIMUM SLEEVE SPACING SHALL BE THREE DIAMETERS CENTER TO CENTER OF THE LARGER SLEEVE OR 6" CLEAR BETWEEN SLEEVES, WHICHEVER IS GREATER. PRIOR TO CONSTRUCTION SLEEVE LOCATIONS AND SIZES SHALL BE APPROVED BY THE ENGINEER. CONDUITS, PIPES AND SLEEVES SHALL BE PLACED AND SPACED IN ACCORDANCE WITH ACI 318 6.3.

REINFORCING STEEL: SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. SECURE APPROVAL OF SHOP DRAWINGS PRIOR TO

COMMENCING FABRICATION. REINFORCING BARS SHOWN IN SECTIONS DEPICT TYPICAL CONFIGURATION AND ARE NOT SPECIFIC TO THE CONCRETE MEMBER CUT ON PLAN; SEE PLAN AND SCHEDULES FOR ALL BAR SIZE AND QUANTITIES. TAKE-OFFS AND QUANTITIES SHALL BE OBTAINED FROM THE SCHEDULES AND PLANS, NOT FROM SECTIONS. WHERE HOOKS, LAP LENGTHS, ETC. ARE SHOWN IN SECTIONS, ALL LENGTHS AND DETAILS SHALL MEET ACI REQUIREMENTS FOR REINFORCING DETAILS. HOOKS SHALL BE ORIENTED AS REQUIRED TO FIT WITHIN THE

WELDED WIRE FABRIC:

CONCRETE MEMBER.

TO CONFORM TO ASTM A-185, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. MINIMUM LAP SHALL BE ONE SPACE PLUS TWO INCHES.

ALL CONCRETE SHALL MEET ACI 318 'BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE' AND ACI 301 'SPECIFICATIONS FOR STRUCTURAL CONCRETE', INCLUDING, BUT NOT LIMITED TO, MIX DESIGN, STRENGTH, COVER, PLACEMENT, CURING, TESTING, FORMS, FLATNESS, ETC. SEE THE ARCHITECTURAL SPECIFICATIONS AND PLANS FOR ANY ADDITIONAL REQUIREMENTS.

ALL CONCRETE SHALL BE AN APPROVED MIX DESIGN PROPORTIONED TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW WITH A PLASTIC AND WORKABLE MIX:

3000 PSI FOR FOUNDATIONS AND SLABS ON GRADE. 4000 PSI FOR ALL OTHER STRUCTURAL CONCRETE.

SUBMIT PROPOSED MIX DESIGN FOR EACH PORTION OF WORK PRIOR TO USE. ALL MIX DESIGNS SHALL BE ACCOMPANIED BY A MINIMUM OF 15 FIELD STRENGTH TEST RECORDS, AS NOTED IN ACI 301 4.2.3.2(A); NO OTHER DOCUMENTATION OF AVERAGE COMPRESSIVE STRENGTH WILL BE ACCEPTED WITHOUT PRIOR WRITTEN APPROVAL BY THE SER. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. MIX SHALL MEET THE REQUIREMENTS OF ASTM C33 FOR COARSE AGGREGATE. FOR ALL FLATWORK, AT LEAST 75% OF LARGE AGGREGATE SHALL CONSIST OF #57 STONE. CONCRETE SHALL COMPLY WITH ALL THE REQUIREMENTS OF ASTM STANDARD C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED

ALL CONCRETE MIX DESIGNS SHALL INCLUDE A WRITTEN DESCRIPTION INDICATING WHERE EACH PARTICULAR MIX IS TO BE PLACED

CONCRETE SHALL BE PLACED AND CURED ACCORDING TO ALL STANDARDS AND SPECIFICATIONS, INCLUDING ALL ACI REQUIREMENTS

THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1-1/2) HOURS. IF FOR ANY REASON THERE IS A LONGER DELAY THAN THAT STATED ABOVE, THE CONCRETE SHALL BE DISCARDED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LAB TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE. ALL SLABS SHALL BE CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1-D AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE. ALL SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY. CALCIUM CHLORIDES SHALL NOT BE UTILIZED; OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER.

THE GENERAL CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD A MINIMUM OF 48 HOURS PRIOR PLACEMENT OF ANY STRUCTURAL CONCRETE.

UNLESS NOTED OTHERWISE ON PLANS, THE FOLLOWING CONCRETE CLEAR COVER SHALL BE PROVIDED FOR ALL NON-PRESTRESSED CONCRETE REINFORCEMENT PER ACI 318:

CONCRETE CAST AGAINST EARTH:	ALL BARS	-	3"
CONCRETE EXPOSED TO EARTH (FORMED FACE):	ALL BARS	-	2"
CONCRETE EXPOSED TO WEATHER:	#6 BARS AND GREATER	-	2"
CONCRETE EXPOSED TO WEATHER:	#5 BARS AND SMALLER		2 1 1/

A) ASTM C143 - "STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE." MAXIMUM SLUMP SHALL BE 4-6 INCHES, PRIOR TO ADDING A SUPER PLASTICIZER. B) ASTM C39 - "STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS." A SEPARATE TEST SHALL BE CONDUCTED FOR EACH CLASS, FOR EVERY 50 CUBIC YARDS (OR FRACTION THEREOF), PLACED PER DAY. REQUIRED CYLINDER(S) QUANTITIES AND TEST AGE AS FOLLOWS: 1 AT 7 DAYS 2 AT 28 DAYS

ONE ADDITIONAL RESERVE CYLINDER TO BE TESTED UNDER THE DIRECTION OF THE ENGINEER, IF REQUIRED. IF 28 DAY STRENGTH IS ACHIEVED. THE ADDITIONAL CYLINDER(S) MAY BE DISCARDED.

POUR STRUCTURAL CONCRETE WITHIN THE FOLLOWING TOLERANCES: VARIATION FROM PLUMB: VARIATION FROM LEVEL IN TOPS OF PILASTERS: 1/8" IN 10'-0"

> VARIATION FOOTINGS: PLAN DIMENSIONS: +2", - 1/2" THICKNESS:

CHEMICAL ANCHORS:

SHALL BE AN EQUAL TWO-PART EPOXY POLYMER INJECTION SYSTEM, SUCH AS SIMPSON SET-XP "STRUCTURAL ANCHORING ADHESIVE", HILTI HIT-HY 200 OR ENGINEER APPROVED SUBSTITUTION, INSTALLED IN ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS. INSTALLERS SHALL BE TRAINED BY THE MANUFACTURER'S REPRESENTATIVE. BRUSH AND BLOW OUT ALL HOLES.

ENGINEERED STEEL STAIR SYSTEM AND ALL CONNECTIONS OF SAME TO THIS STRUCTURE SHALL BE DESIGNED BY AN ENGINEER REGISTERED IN THE STATE OF FLORIDA. SUBMIT SHOP DRAWINGS BEARING THE EMBOSSED SEAL AND THE SIGNATURE OF THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. THE CONFIGURATION OF THE STEEL STAIR SYSTEM SHALL BE AS SHOWN ON THE ARCHITECTURAL DRAWINGS. STEEL STAIR SYSTEM AND ALL CONNECTIONS SHALL BE DESIGNED FOR ALL APPLICABLE LOADS AS INDICATED ON THE PLANS AND IN THE BUILDING CODE. THE LOADS SHALL BE CLEARLY INDICATED ON ALL SHOP DRAWINGS. SHOP DRAWINGS SHALL SHOW AND SPECIFY ALL CONNECTIONS UTILIZED WITHIN THE STEEL STAIR SYSTEM AS WELL AS CONNECTIONS TO AND LOADS IMPOSED UPON THE STRUCTURAL SYSTEM SHOWN ON THESE PLANS.

MINIMUM STAIR DESIGN LOADS: UNIFORM LIVE LOAD = 100 PSF

CONCENTRATED LIVE LOAD = 300 LBS SHOP DRAWINGS SHALL SHOW AND SPECIFY ALL CONNECTIONS (INCLUDING THE HANDRAIL) UTILIZED WITHIN THE STEEL STAIR SYSTEM AS WELL AS CONNECTIONS TO AND LOADS IMPOSED UPON THE STRUCTURAL SYSTEM SHOWN ON THESE PLANS. FAILURE TO PROVIDE ALL CONNECTIONS OF THE STEEL STAIR SYSTEM TO THE STRUCTURE SHALL BE CAUSE TO REJECT THE ENTIRE STAIR SUBMITTAL AND INSTALLATION

STRUCTURAL WOOD COMPONENTS (BEAMS, JOISTS, RAFTERS, ETC.) SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE FIBER STRESSES FOR NO. 2 SOUTHERN PINE CONFORMING TO NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION WITH 2015 NDS SUPPLEMENT. AS FOLLOWS:

T ELIVIEITI, AOT	JLLOVVO.	
SHEAR	FV	= 175 PSI.
BENDING 2X6	FB	= 1,000 PSI.
RENDING 2X8	FR	= 925 PSI

BENDING 2X10 FB = 800 PSI.

BENDING 2X12 FB = 750 PSI. WOOD IN CONTACT WITH CONCRETE OR MASONRY, AND AT OTHER LOCATIONS AS SHOWN ON STRUCTURAL DRAWINGS, SHALL BE PROTECTED OR PRESSURE TREATED IN ACCORDANCE WITH AITC-109. MEMBER SIZES SHOWN ARE NOMINAL UNLESS NOTED

ALL NAILS SHOWN ON PLANS ASSUME COMMON WIRE NAILS UNLESS SPECIFICALLY NOTED ON DRAWINGS. BOLTS FOR WOOD CONSTRUCTION AT INTERIOR LOCATIONS SHALL CONFORM TO ASTM A307. THREADED RODS AT INTERIOR LOCATIONS SHALL CONFORM TO ASTM A36. EXTERIOR BOLTS AND THREADED RODS PROTECTED FROM MOISTURE AND WEATHER SHALL BE HOT-DIP GALVANIZED. EXPOSED EXTERIOR BOLTS AND THREADED RODS SHALL BE AISI 316 STAINLESS STEEL. ALL NAILS LARGER THAN 10D AND SCREWS LARGER THAN 7 GAUGE SHALL BE PREDRILLED AS NEEDED TO PREVENT SPLITTING OF THE WOOD. BOLT HOLES IN WOOD MEMBERS SHALL BE A MINIMUM OF 1/32" LARGER THAN THE BOLT DIAMETER, BUT NO GREATER THAN 1/16" LARGER. A METAL PLATE OR WASHER NO SMALLER THAN A STANDARD CUT WASHER SHALL BE LOCATED BETWEEN THE BOLT HEAD AND THE WOOD AND BETWEEN THE NUT AND THE WOOD OF ALL BOLTS.

FOR WOOD WALLS, PROVIDE A SINGLE CONTINUOUS BOTTOM PLATE AND DOUBLE CONTINUOUS TOP PLATE. THE TOP PLATE PLIES SHALL HAVE STAGGERED LAPS OF 24" WITH (8) 10D NAILS IN EACH SPLICE. AT WALL CORNERS AND INTERSECTIONS, TOP PLATES SHALL BE STAGGERED AND OVERLAPPED WITH (4) 10D NAILS IN THE JOINT.

ENGINEERED WOOD TRUSS SYSTEMS SHALL BE DESIGNED BY SUPPLIER'S SPECIALTY ENGINEER TO CONFIGURATION AND LOAD-CARRYING CAPACITY SHOWN ON DRAWINGS AND SPECIFICATIONS. ALL INDIVIDUAL TRUSS MEMBERS, TRUSS PLATE CONNECTIONS, TRUSS-TO-TRUSS CONNECTIONS, COMMON TRUSSES, AND GIRDER TRUSSES SHALL BE DESIGNED FOR COMPONENT AND CLADDING WIND LOADING. EXCEPT THOSE TRUSSES EXCEEDING 700 SQUARE FEET IN TRIBUTARY AREA. ALTERNATE TRUSS LAYOUTS ARE ACCEPTABLE ONLY AS A CHANGE ORDER WHICH WILL INCLUDE ENGINEERING CHARGES FOR REDESIGN OF THE STRUCTURE BY THE ENGINEER OF RECORD. SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS SHALL SHOW AND SPECIFY ALL CONNECTOR TYPES UTILIZED WITHIN TRUSSES, AS WELL AS CONNECTORS UTILIZED IN ALL OTHER CONNECTIONS AND ATTACHMENTS BETWEEN TRUSSES OR COMPONENTS SUPPLIED AS PART OF THE ENGINEERED TRUSS SYSTEM. AN ERECTION DRAWING SHALL BE INCLUDED, IDENTIFYING ALL TRUSS SYSTEM COMPONENTS, AS WELL AS ALL PERMANENT BRACING REQUIRED FOR TRUSS DESIGN.

TRUSSES AT ALL BEARING LOCATIONS SHALL BE STABILIZED. THE GENERAL CONTRACTOR SHALL FOLLOW ALL REQUIREMENTS BY THE DELEGATED TRUSS ENGINEERING PACKAGE. AT A MINIMUM: FOR TRUSS HEEL DEPTHS LESS THAN 8", ONLY BLOCKING BY THE TRUSS ENGINEERING IS REQUIRED; FOR TRUSS HEEL DEPTHS BETWEEN 8" AND 12", USE FULL HEIGHT SAWN-LUMBER BLOCKING; FOR TRUSS HEEL DEPTHS GREATER THAN 12", USE TRUSS BLOCKING OR A SHEATHED KNEEWALL. SEE THE STRUCTURAL DRAWINGS FOR ANY ADDITIONAL BLOCK REQUIREMENTS FOR THE LATERAL-FORCE RESISTING SYSTEM.

"FLOOR TRUSSES" AND "4X2 TRUSSES" REFER TO TRUSSES WITH THE TOP AND BOTTOM CHORDS ORIENTED WITH THE STRONG AXIS HORIZONTAL (I.E. ON THE FLAT). "ROOF TRUSSES" AND "2X4 TRUSSES" REFER TO TRUSSES WITH THE TOP AND BOTTOM CHORDS ORIENTED WITH THE STRONG AXIS VERTICAL.

ENGINEERED SHOP DRAWINGS SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A REGISTERED PROFESSIONAL ENGINEER, LICENSED IN THE STATE WHERE THE STRUCTURE WILL BE ERECTED, AS THE DELEGATED (SPECIALTY) ENGINEER. PLUMBING, ELECTRICAL. AND MECHANICAL DRAWINGS SHALL BE COORDINATED WITH THE TRUSS LAYOUT TO ENSURE THAT THERE ARE NO CONFLICTS WITH DUCTS, RECESSED FIXTURES, PLUMBING PIPES, TRAPS, HOODS, CEILING STEPS/SLOPES, ETC. TRUSS LAYOUT SHALL BE MODIFIED AND/OR TRUSS CHASES SHALL BE ADDED TO AVOID CONFLICTS. TRUSS SPACING SHALL NOT EXCEED MAXIMUM NOTED IN

THE FOLLOWING LOAD DURATION FACTORS SHALL BE USED:

DEAD LOAD	0.90
DEAD LOAD + FLOOR LIVE LOAD	1.00
DEAD LOAD + ROOF LIVE LOAD	1.2
DEAD LOAD + WIND LOAD	1.60

THE SUPERIMPOSED DEAD LOAD. AS SPECIFIED IN THE DESIGN LOADS SECTION ABOVE, INCLUDES THE OVERALL WEIGHT OF THE FIRE SPRINKLER SYSTEM PIPES. THE GENERAL CONTRACTOR SHALL PROVIDE THE TRUSS MANUFACTURER WITH THE LOCATIONS OF THE PIPE SUPPORTS AND THE LOADS FROM ALL SPRINKLER LINES GREATER THAN 2" DIAMETER. THE GENERAL CONTRACTOR SHALL PROVIDE THE TRUSS MANUFACTURER AND THE SEOR WITH THE FINAL WEIGHTS FOR ALL MECHANICAL EQUIPMENT, INCLUDE SECONDARY FRAMING AND CURBS, PRIOR TO FABRICATION OF TRUSSES AND STRUCTURAL MEMBERS SUPPORTING SAID TRUSSES.

ALL TRUSSES SHALL BE DESIGNED FOR A MAXIMUM DEFLECTION OF L/360 FOR LIVE LOAD AND L/240, NOT TO EXCEED 1", FOR TOTAL LOAD; THE MAXIMUM DEFLECTION DUE TO TOTAL LOAD OF 1" IS INCLUSIVE OF ALL DEAD LOAD, SELF-WEIGHT, SUPERIMPOSED DEAD LOAD, AND LIVE LOAD, INCLUDING CREEP.

PLYWOOD ROOF, FLOOR, AND WALL SHEATHING ARE DESIGNED AS DIAPHRAGMS/SHEAR WALLS AND SHALL COMPLY WITH APPLICABLE PROVISIONS OF CHAPTER 23 OF THE BUILDING CODE AND SHALL BE FASTENED IN ACCORDANCE WITH THE MINIMUM REQUIREMENTS OF BUILDING CODE TABLES, UNLESS SHOWN OTHERWISE. PLYWOOD SHALL BE INSTALLED WITH THE STRENGTH AXIS OF EACH PANEL PERPENDICULAR TO THE SUPPORTS IN ALL CASES. PLYWOOD ROOF PANELS SHALL BE INSTALLED AS SHOWN IN CASES 1 THROUGH 4 IN TABLE 2306.2.1 (CONT.). BLOCKING SHALL BE PROVIDED BETWEEN ALL WOOD ROOF FRAMING MEMBERS AT ALL RIDGES AND VALLEYS FOR FULL PLYWOOD EDGE SUPPORT. AT ROOF VENT LOCATIONS, PROVIDE 2X4 BLOCKING, ON THE FLAT, ON ALTERNATING SIDES OF THE VENT BETWEEN ROOF FRAMING MEMBERS.

ALL WOOD SHEAR WALLS SHALL HAVE ALL PLYWOOD EDGES FULLY BLOCKED WITH THE SAME STUD SIZE AS THE WALLS, WITH THE BLOCKING INSTALLED SO THAT THE PLYWOOD IS NAILED INTO THE NARROW STUD FACE. FASTEN THE SHEATHING WITH THE SAME NAIL SIZE AND SPACING TO EACH PLY OF DOUBLE TOP AND BOTTOM WALL PLATES. AS APPLICABLE. FASTEN THE SHEATHING WITH THE SAME NAIL SIZE AND SPACING TO MULTI-PLY COLUMNS AT ENDS OF WALLS; WHERE SOLID COLUMNS ARE USED AT ENDS OF WALLS, FASTEN THE SHEATHING WITH THE SAME NAIL SIZE AND SPACING IN VERTICAL ROWS WITH 1-1/2" ROW SPACING FOR FULL HEIGHT OF COLUMN. WHERE SHEAR WALL PANEL EDGE NAILING IS 3" OR LESS, THE BLOCKING AT PANEL EDGES SHALL BE 3" NOMINAL OR GREATER AND THE NAILING SHALL BE STAGGERED. MULTIPLE PLY STUDS MAY BE USED AS THE PANEL EDGE BLOCKING IN LIEU OF 3" NOMINAL BLOCKING; FASTEN THE PLIES WITH NAILS HAVING THE SAME LENGTH AS THE TOTAL BLOCKING THICKNESS WITH SPACING TO MATCH THE PLYWOOD PANEL EDGE NAILING, STAGGERED. PANELS SHALL NOT BE LESS THAN 4FTX8FT, EXCEPT AT BOUNDARIES AND CHANGES

MANUFACTURED WOOD STRUCTURAL COMPONENTS:

MEMBERS DESIGNATED 'LVL' (E.G., 1-3/4 X 14 LVL) SHALL BE LAMINATED VENEER LUMBER AS MANUFACTURED BY TRUS JOIST CORPORATION (MICRO=LAM), ALPINE ENGINEERED PRODUCTS (ASI TIMBERMAX-LVL), OR ENGINEER APPROVED SUBSTITUTION. FB = 2600 PSI, E = 2,000,000 PSI, FV = 285 PSI.

MEMBERS DESIGNATED 'PSL' (E.G., 3½"X111/4" PSL) SHALL BE PARALLAM PARALLEL STRAND LUMBER AS MANUFACTURED BY TRUS JOIST CORPORATION (MICRO=LAM), OR ENGINEER APPROVED SUBSTITUTION. FB = 2900 PSI, E = 2,000,000 PSI, FV = 290 PSI.

WOOD FRAMING CONNECTORS:

ALL CONNECTORS SHALL BE GALVANIZED. CONNECTOR MODEL NUMBERS SHOWN ARE STRONG-TIE CONNECTORS AS MANUFACTURED BY SIMPSON STRONG-TIE CO., 5956 W. LAS POSITAS BLVD., P.O. BOX 10789, PLEASANTON, CA 94588, 800-999-5099, WWW.STRONGTIE.COM. SUBSTITUTIONS ARE ACCEPTABLE WITH THE APPROVAL OF THE STRUCTURAL ENGINEER. UNLESS SHOWN OTHERWISE, INSTALL THE *LARGEST* FASTENER SIZE AND *MAXIMUM* NUMBER OF FASTENERS SHOWN IN LATEST SIMPSON CATALOG. WHERE SDS SCREWS ARE SPECIFIED IN THE SIMPSON CATALOG, SDS SCREWS MUST BE USED UNLESS EXPRESSLY SHOWN IN THE DRAWINGS OTHERWISE. ALL ROOF AND UPPER-LEVEL UPLIFT CONNECTORS SHALL BE LOCATED ON THE SAME SIDE OF THE WALL AS

ALL COLUMN BASE AND HOLD-DOWN CONNECTORS (HDU, HTT, LTT, ETC.) FOR STRUCTURAL COMPOSITE LUMBER (PSL, LVL, LSL, ETC.) SHALL BE INSTALLED IN THE WIDE FACE OF THE COLUMN, AND NOT THE NARROW FACE. THE NARROW FACE IS THE SURFACE THAT

SHOWS THE VERTICAL THIN EDGES OF THE STRUCTURAL COMPOSITE LUMBER LAYERS.

FOR ALL CONNECTORS REQUIRING A THREADED ROD ATTACHMENT TO CONCRETE OR GROUT-FILLED MASONRY, PROVIDE A THREADED ROD DIAMETER AS SPECIFIED IN THE SIMPSON CATALOG WITH A DOUBLE-NUT AND OVERSIZED WASHER EMBEDDED A MINIMUM OF 9" (UNLESS NOTED OTHERWISE ON THE PLANS) INTO THE MEMBER PRIOR TO THE POUR.





This item has been digitally signed and sealed by Cordell S. Van Nostrand on 01/05/22 using a Digital Signature. Printed copies of this document are not

considered signed and sealed and the

signature must be verified on any

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ABBREVIATIONS

POUND(S)

ARCHITEC:

BOTTOM

BOTTOM OF

BASE PLATE

CANTILEVER

CONTINUOUS

DIAMETER

EACH END EACH FACE

EXPANSION JOINT

ELEVATION

EMBEDMENT

EDGE OF SLAB

FLOOR DRAIN

28 DAY COMPRESSIVE STRENGTH

FINISHED FLOOR ELEVATION

FIRE RETARDANT TREATED

GENERAL CONTRACTOR

HOLLOW STRUCTURAL STEE

NORTH AMERICAN VERTICAL DATUM

NATIONAL GEODETIC VERTICAL DATUM

PRESSURE TREATED/ POST TENSIONED

STRUCTURAL ENGINEER OF RECORD

CONSTRUCTION JOINT

LONG LEG HORIZONTAL

LONG LEG VERTICAL

NOT IN CONTRACT

POUNDS PRE LINEAR FOOT

POUNDS PER SQUARE FOOT

POUNDS PER SQUARE INCH

ON CENTER

PROJECTION

ROOF DRAIN

REFERENCE

REQUIRED

REINFORCING

RIPPED TO FIT

STAINLESS STEEL

SHORT WAY/ SHEAR WALL

STANDARD

TEMPERATURE

THREADED

TRANSVERSE

WORKING POINT

WELDED WIRE FABRI

SHEET LIST

RS-001

RS-011

RS-101

RS-101A

RS-101B

RS-102

RS-102A

RS-102B

RS-102C

RS-103

RS-103A

RS-103B

RS-104

RS-104A

RS-104B

RS-105

RS-105A

RS-105B

RS-107

RS-300 RS-400

RS-410

RS-411

THROUGH

TYPICAL

VERTICAL

STRUCTURAL NOTES

FOUNDATION WALL LOADING PLAN

GROUND FLOOR OVERALL PLAN

GROUND FLOOR PLAN - AREA A

GROUND FLOOR PLAN - AREA B

SECOND FLOOR OVERALL PLAN

SECOND FLOOR PLAN - AREA A

SECOND FLOOR PLAN - AREA B

MECHINCAL COORDINATION PLAN

THIRD FLOOR OVERALL PLAN

THIRD FLOOR PLAN - AREA A

THIRD FLOOR PLAN - AREA B

FOURTH FLOOR OVERALL PLAN

FOURTH FLOOR PLAN - AREA A

FOURTH FLOOR PLAN - AREA B

ROOF FRAMING OVERALL PLAN

ROOF FRAMING PLAN - AREA A

ROOF FRAMING PLAN - AREA B

UNIT PLANS

SCHEDULES TYPICAL DETAILS

BUILDING SECTIONS SECTIONS

SECTIONS

FOUNDATION COLUMN LOADING PLAN

V OR VERT

TOP OF

OH OR OPP HAND OPPOSITE HAND

EACH WAY

EXISTING

FOOTING

GALVANIZED

HORIZONTAL

LONG WAY

MAXIMUM

MINIMUM

CONTROL JOINT

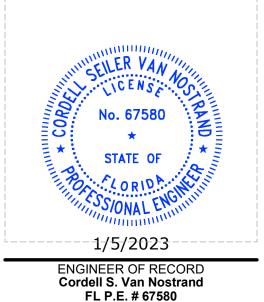
B OR BOT

B/ OR B.O.

H OR HORIZ

DESCRIPTION





PERMIT SET

SUBMISSION

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SR-82

7780 LIGHTARD KNOTT LN FORT MYERS, FL 33905 22063

STRUCTURAL NOTES

UNIT L

3.3 5.1 1.9 3.6 5.4 2.0 4.0 3.0 4.5 1.7 4.0 5.0 5.0

(MIRRORED)

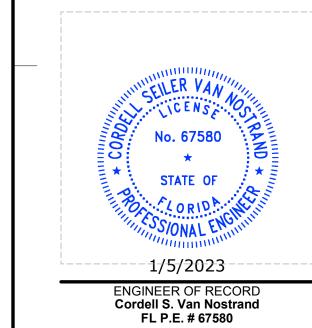
2.8 (MIRRORED)

1.3 / 1.9 5.3 3.5 1.1 1.8 4.0 2.0 5.6 3.8 4.0 1.8 3.0 4.0 5.0 1.7 4.6 3.1 5.0 5.0 2.0 2.0

(MIRRORED)

1.3 / 3.5 5.3 1.9 1.1 1.8 4.0 3.8 5.6 2.0 4.0 1.8 5.0 3.1 4.6 1.7 5.0 2.0 2.0 ENGINEERING CONSULTANTS
1517 State Street, Suite 202, Sarasota, FL 34236
P. 941.954.0681 F. 941.256.3773
snellengineering.com
...SUPPORTING ARCHITECTURE





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KNOWLEDGE AND ABILLTY, THE PLANS AND
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MINIMUM BUILDING CODES.

COLUMN LOADING
LEGEND

DEAD LOAD (KIPS)
LIVE LOAD (KIPS)
WIND UPLIFT (KIPS)

	HEARWALL EACTIONS
GROUND FLOOR	±35.0KIPS
2ND FLOOR	±20.5KIPS
3RD FLOOR	±9.9KIPS
4TH FLOOR	±3.2KIPS

END OF SHEARWALL COLUMNS		
GROUND FLOOR	5-1/4" X 7" PSL & 5-1/4" X 5-1/4" PS	
2ND FLOOR	5-1/4" X 7" PSL	
3RD FLOOR	(4) 2X6	
4TH FLOOR	(2) 2X6 OR (3) 2X4	

FOUNDATION COLUMN LOADING PLAN NOTES:

0.5

2.8

0.9 0.9 1.4 1.4

0.9 0.9 1.4 1.4

- 1. LOADS SHOWN ON THESE SHEETS ARE LISTED IN SERVICE LOADS. ENGINEERED SYSTEMS SHALL USE LOAD COMBINATIONS AS REQUIRED FOR THE DELEGATED
- GROUND FLOOR SHALL BE PRE-ENGINEERED POST-TENSIONED CONCRETE SLAB-ON-GRADE, U.N.O. PROVIDE DRAWINGS SIGNED AND SEALED BY A FLORIDA REGISTERED PROFESSIONAL ENGINEER. SEE ARCHITECTURAL DRAWINGS FOR ALL SLOPES, DROPS AND DRAIN LOCATIONS IN FLOOR SLAB. MAINTAIN MINIMUM SLAB DEPTH AS SHOWN ON THE POST-TENSIONED SLAB ON GRADE SHOP DRAWINGS. SEE STRUCTURAL NOTES.
- 3. OVERALL WALL HOLD-DOWNS, INCLUDING WIND UPLIFT AND SHEARWALL OVERTURNING, SHALL BE RESISTED BY THREADED RODS EXTENDING FROM THE CONCRETE SLAB ON GRADE FOUNDATION SYSTEM TO THE TOP OF THE WALLS AT THE ROOF. THREADED ROD SYSTEM SHALL BE DESIGNED BY A FLORIDA REGISTERED PROFESSIONAL ENGINEER. THE THREADED ROD SYSTEM SHALL BE SELF-TENSIONING TO ACCOMMODATE WOOD SHRINKAGE. SPACING AND SIZE OF THREADED ROD HOLD-DOWNS SHALL BE DESIGNED TO RESIST THE UPLIFTS SHOWN ON THESE DRAWINGS AND ASSUMING THE UPLIFTS OCCUR AT THE ROOF LEVEL AT THESE LOCATIONS. THE THREADED RODS SHALL BE SPACED AS REQUIRED, NOT TO EXCEED 4'-0" O.C.
- 4. WHERE ONLY DEAD AND LIVE LOADS ARE APPLICABLE, WIND UPLIFT IS OMITTED.
- SEE PLAN FOR LOCATIONS OF 2x4 WOOD SHEAR WALLS. SHEAR WALLS EXTEND FROM THE GROUND FLOOR TO THE ROOF ELEVATION. SHEAR WALLS SHALL BE SHEATHED ON ONE SIDE W/ 15/32" APA RATED EXPOSURE 1 PLYWOOD SHEATHING. FASTEN PLYWOOD SHEATHING W/ 10D NAILS AT SUPPORTED PANEL EDGE LOCATIONS @ 4" O.C., AND 12" O.C. IN THE FIELD. ALL 2x6 EXTERIOR AND CORRIDOR WALLS ARE SHEAR WALL UNLESS NOTED OTHERWISE. PROVIDE (3) 2x6 COLUMNS AT ALL CORNERS AND ENDS OF 2x6 SHEAR WALLS WITH LTT19 HOLD DOWN ANCHOR AT BASE OF COLUMN. THE COMPRESSION AND TENSION REACTIONS AT THE ENDS OF SHEARWALLS SHALL BE DESIGNED BY THE POST-TENSIONED CONCRETE SLAB ON GRADE ENGINEER AND THE THREADED ROD TIE-DOWN ENGINEER. THESE COUPLE FORCES ARE DUE TO SHEARWALLS RESISTING WIND, AND ARE REVERSIBLE. SPACE THE THREADED RODS TO ACCOMPLISH AN EFFECTIVE TENSION RESISTANCE LOCATION EQUAL TO THE CENTROID OF THE COLUMN, SHOWN ON THE SHEARWALL SCHEDULE. SEE THE SCHEDULE ON THIS SHEET FOR COUPLE REACTIONS AT EACH

FLOOR ELEVATION, AND THE WOOD COLUMN DETAILS AT ALL FLOOR ELEVATIONS.

6. TYPICAL JAMB COLUMN UPLIFT VALUES AT OPENINGS WITHIN EXTERIOR 2X6 WOOD WALLS SHALL BE 2,200 LBS, UNLESS NOTED OTHERWISE. TYPICAL JAMB COLUMN UPLIFT VALUES AT DOOR OPENINGS WITHIN INTERIOR CORRIDOR 2X6 WOOD WALLS SHALL BE 2,250 LBS, UNLESS NOTED OTHERWISE.



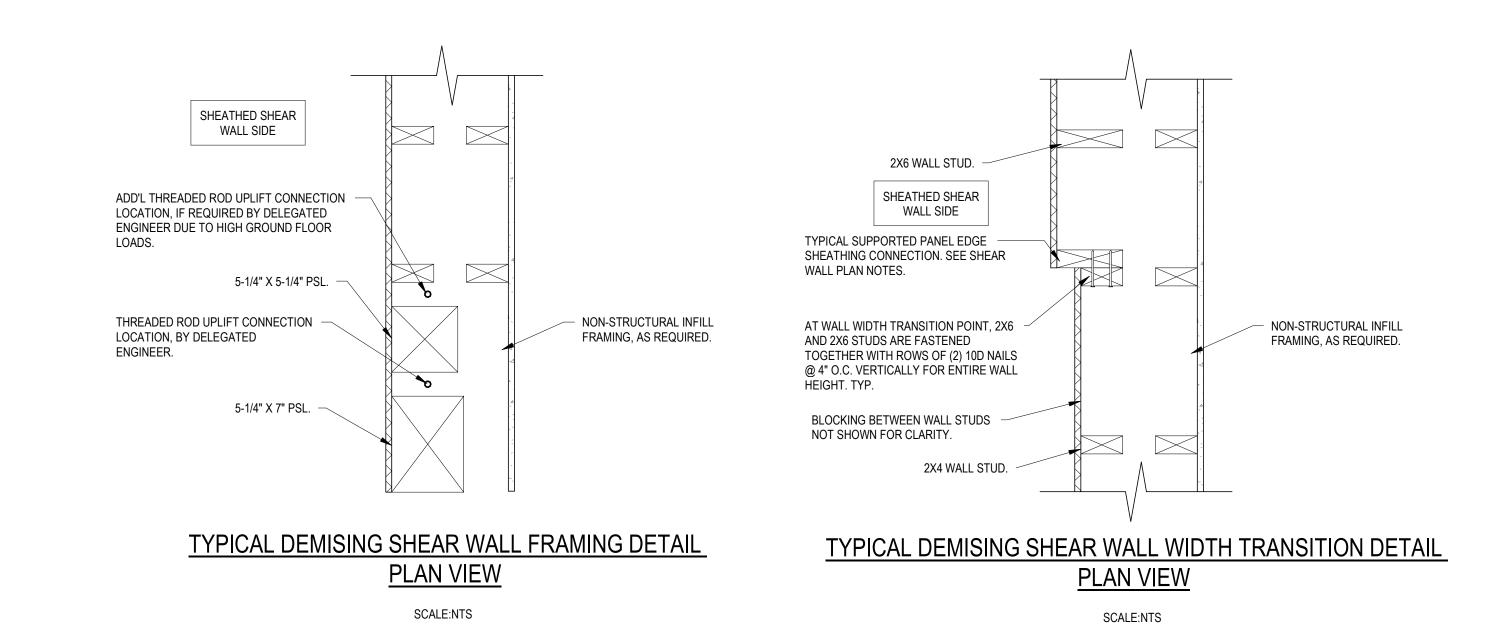
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7780 LIGHTARD KNOTT LN FORT MYERS, FL 33905
PROJECT NO:
22063

FOUNDATION COLUMN LOADING PLAN

RS-010



(MIRRORED)

0.9 0.9

0.9 0.9 1.4 1.4

3/32" = 1'-0"

FOUNDATION COLUMN LOADING PLAN

1.1 1.1 0.9 0.9 1.4 1.4



ENCINEERING CONSOLTANT

Baker Barrios

ORLANDO

189 S. ORANGE E 3/15823773

suellengineering.com

189 S. ORANGE AVE., SUITE 1700

ORLANDO, FLORIDA 32801

407 926 3000

INFO@BAKERBARRIOS.COM

BAKERBARRIOS.COM

BAKERBARRIOS.COM

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No. 67580

No. 67580

*
STATE OF

1/5/2023

ENGINEER OF RECORD
Cordell S. Van Nostrand
FL P.E. # 67580

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MILHAUS_®

MILHAUS

SR-82

7780 LIGHTARD KNOTT LN FORT MYERS, FL 33905
PROJECT NO:

22063

FOUNDATION WALL LOADING

PLAN
SHEET NUMBER:

RS-011

FOUNDATION WALL LOADING PLAN
3/32" = 1'-0"

ND

DEAD LOAD (LBS/FT)
LIVE LOAD (LBS/FT)
WIND UPLIFT (LBS/FT)

FOUNDATION WALL LOADING PLAN NOTES:

LOADS SHOWN ON THESE SHEETS ARE LISTED IN SERVICE LOADS. ENGINEERED SYSTEMS SHALL USE LOAD COMBINATIONS AS REQUIRED FOR THE DELEGATED DESIGN.

- 2. GROUND FLOOR SHALL BE PRE-ENGINEERED POST-TENSIONED CONCRETE SLAB-ON-GRADE, U.N.O. PROVIDE DRAWINGS SIGNED AND SEALED BY A FLORIDA REGISTERED PROFESSIONAL ENGINEER. SEE ARCHITECTURAL DRAWINGS FOR ALL SLOPES, DROPS AND DRAIN LOCATIONS IN FLOOR SLAB. MAINTAIN MINIMUM SLAB DEPTH AS SHOWN ON THE POST-TENSIONED SLAB ON GRADE SHOP DRAWINGS. SEE STRUCTURAL NOTES.
- 3. OVERALL WALL HOLD-DOWNS, INCLUDING WIND UPLIFT AND SHEARWALL OVERTURNING, SHALL BE RESISTED BY THREADED RODS EXTENDING FROM THE CONCRETE SLAB ON GRADE FOUNDATION SYSTEM TO THE TOP OF THE WALLS AT THE ROOF. THREADED ROD SYSTEM SHALL BE DESIGNED BY A FLORIDA REGISTERED PROFESSIONAL ENGINEER. THE THREADED ROD SYSTEM SHALL BE SELF-TENSIONING TO ACCOMMODATE WOOD SHRINKAGE. SPACING AND SIZE OF THREADED ROD HOLD-DOWNS SHALL BE DESIGNED TO RESIST THE UPLIFTS SHOWN ON THESE DRAWINGS AND ASSUMING THE UPLIFTS OCCUR AT THE ROOF LEVEL AT THESE LOCATIONS. THE THREADED RODS SHALL BE SPACED AS REQUIRED, NOT TO EXCEED 4'-0" O.C.
- 4. WHERE ONLY DEAD AND LIVE LOADS ARE APPLICABLE, WIND UPLIFT IS OMITTED.
- 5. ALL EXTERIOR 2X6 LOAD BEARING WALLS SHALL BE DESIGNED FOR AN UPLIFT VALUE OF 600 PLF.

Baker Barrios

ORLANDO

ORLANDO, FLORIDA 32801 407 926 3000 INFO@BAKERBARRIOS.COM **BAKERBARRIOS.COM**

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189 S. ORANGE AVE., SUITE 1700

1/5/2023 ENGINEER OF RECORD Cordell S. Van Nostrand FL P.E. # 67580

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GROUND FLOOR PLAN NOTES:

- 1. VERIFY ALL DIMENSIONS, ELEVATIONS, AND SLAB FINISHES WITH THE ARCHITECTURAL DRAWINGS BEFORE COMMENCING CONSTRUCTION. FOR ADDITIONAL DIMENSIONAL INFORMATION SEE ARCHITECTURAL DRAWINGS.
- 2. ELEVATIONS SHOWN ARE RELATIVE TO THE INTERIOR GROUND FLOOR SLAB SURFACE SET AT 0'-0" (REF.). SEE CIVIL FOR TOP OF INTERIOR GROUND FLOOR SLAB ELEVATION.
- SEE S-200 FOR ALL SCHEDULES.
- 4. GROUND FLOOR SHALL BE PRE-ENGINEERED POST-TENSIONED CONCRETE SLAB-ON-GRADE, U.N.O. SEE WALL AND COLUMNS LOADS PROVIDED ON THE PREVIOUS PLAN SHEETS. PROVIDE DRAWINGS SIGNED AND SEALED BY A FLORIDA REGISTERED PROFESSIONAL ENGINEER. SEE ARCHITECTURAL DRAWINGS FOR ALL SLOPES, DROPS AND DRAIN LOCATIONS IN FLOOR SLAB. MAINTAIN MINIMUM SLAB DEPTH AS SHOWN ON THE POST-TENSIONED SLAB ON GRADE SHOP DRAWINGS. SHOP DRAWINGS SHALL CONSIDER ALL SLAB PENETRATIONS. SEE STRUCTURAL NOTES.
- 5. ALL WOOD IN CONTACT WITH MASONRY, CONCRETE OR USED IN EXTERIOR APPLICATIONS SHALL BE PRESSURE TREATED WITH PRESERVATIVE. (P.T.) EXTERIOR CONNECTORS SHALL BE HOT DIPPED GALVANIZED.
- 6. ALL CORRIDOR WALLS TO BE 2x6, UNLESS NOTED OTHERWISE.
- 7. SEE UNIT PLANS FOR UNIT WALL AND FLOOR FRAMING.

WALL LEGEND

INDICATES A 4" OR 6" LOAD BEARING WOOD FRAMED STUD WALL. SEE PLAN NOTES

AND UNIT PLANS FOR MORE INFORMATION. INDICATES AN 8" LOAD BEARING MASONRY WALL. PROVIDE 1#5 BAR IN GROUTED CELLS AT CORNERS, ENDS, INTERSECTIONS OF WALLS, UNDER POINT LOADS AND AT 24" O.C. MAX. ALL CELLS BELOW GRADE, OR IN CONTACT WITH SOIL, SHALL BE GROUTED SOLID.

GROUND FLOOR OVERALL PLAN

22063

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MILHAUS

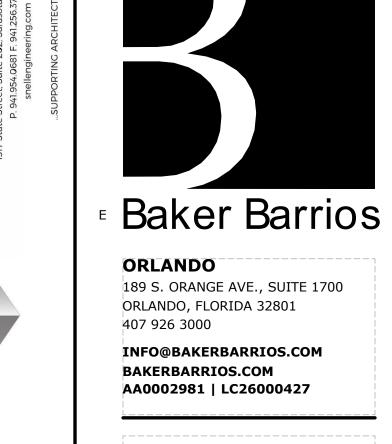
SR-82

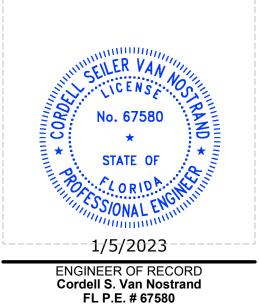
RS-101

COLUMN LEGEND INDICATES COLUMN MARK. COLUMNS ARE ABOVE, BELOW, OR THROUGH AS SHOWN.

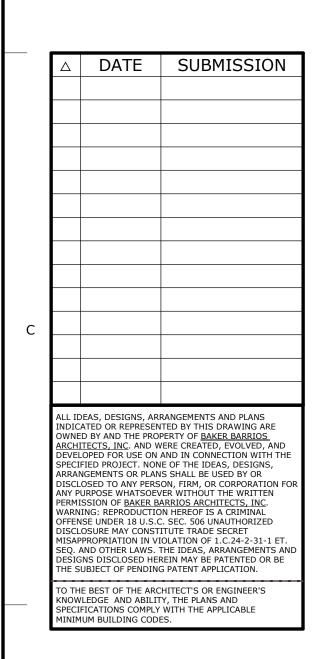
GROUND FLOOR OVERALL PLAN
3/32" = 1'-0"







PERMIT SET



GROUND FLOOR PLAN NOTES:

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WALL LEGEND

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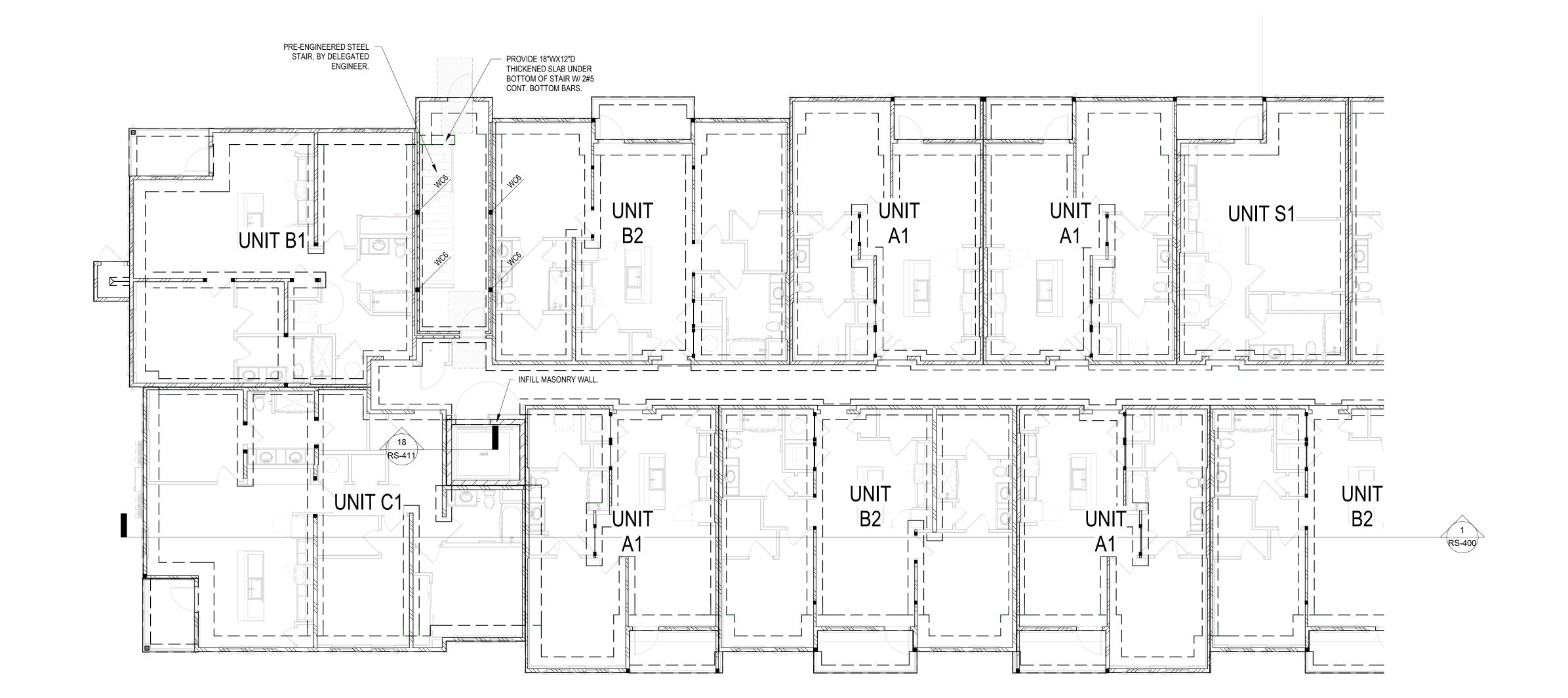
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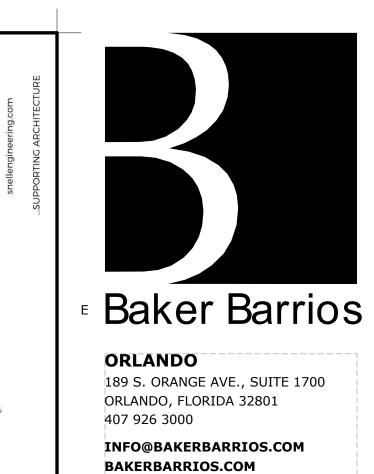
GROUND FLOOR PLAN - AREA A

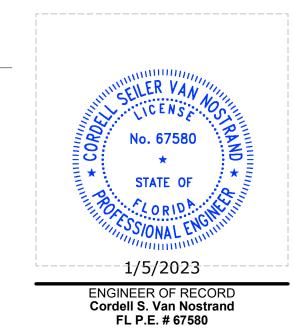
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GROUND FLOOR FRAMING PLAN - AREA A

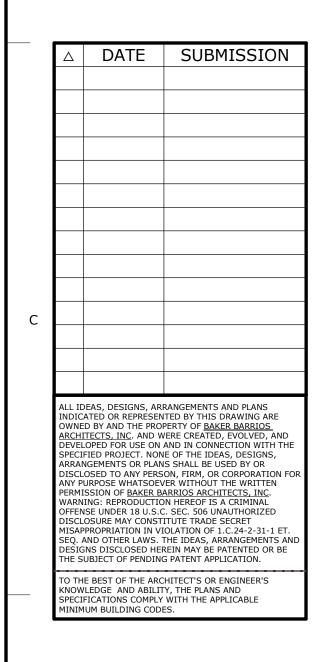






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GROUND FLOOR PLAN NOTES:

PRE-ENGINEERED STEEL -STAIR, BY DELEGATED

UNIT S1

ENGINEER.

GROUND FLOOR FRAMING PLAN - AREA B 1/8" = 1'-0"

PROVIDE 18"WX12"D
THICKENED SLAB UNDER
BOTTOM OF STAIR W/ 2#5

CONT. BOTTOM BARS.

- VERIFY ALL DIMENSIONS, ELEVATIONS, AND SLAB FINISHES WITH THE ARCHITECTURAL DRAWINGS BEFORE COMMENCING CONSTRUCTION. FOR ADDITIONAL DIMENSIONAL INFORMATION SEE ARCHITECTURAL DRAWINGS.
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- 3. SEE S-200 FOR ALL SCHEDULES.
- GROUND FLOOR SHALL BE PRE-ENGINEERED POST-TENSIONED CONCRETE SLAB-ON-GRADE, U.N.O. SEE WALL AND COLUMNS LOADS PROVIDED ON THE PREVIOUS PLAN SHEETS. PROVIDE DRAWINGS SIGNED AND SEALED BY A FLORIDA REGISTERED PROFESSIONAL ENGINEER. SEE ARCHITECTURAL DRAWINGS FOR ALL SLOPES, DROPS AND DRAIN LOCATIONS IN FLOOR SLAB. MAINTAIN MINIMUM SLAB DEPTH AS SHOWN ON THE POST-TENSIONED SLAB ON GRADE SHOP DRAWINGS. SHOP DRAWINGS SHALL CONSIDER ALL SLAB PENETRATIONS. SEE STRUCTURAL NOTES.
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COLUMN LEGEND INDICATES COLUMN MARK. COLUMNS ARE ABOVE, BELOW, OR THROUGH AS SHOWN.



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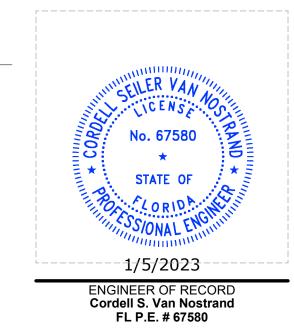
GROUND FLOOR

PLAN - AREA B

RS-101B



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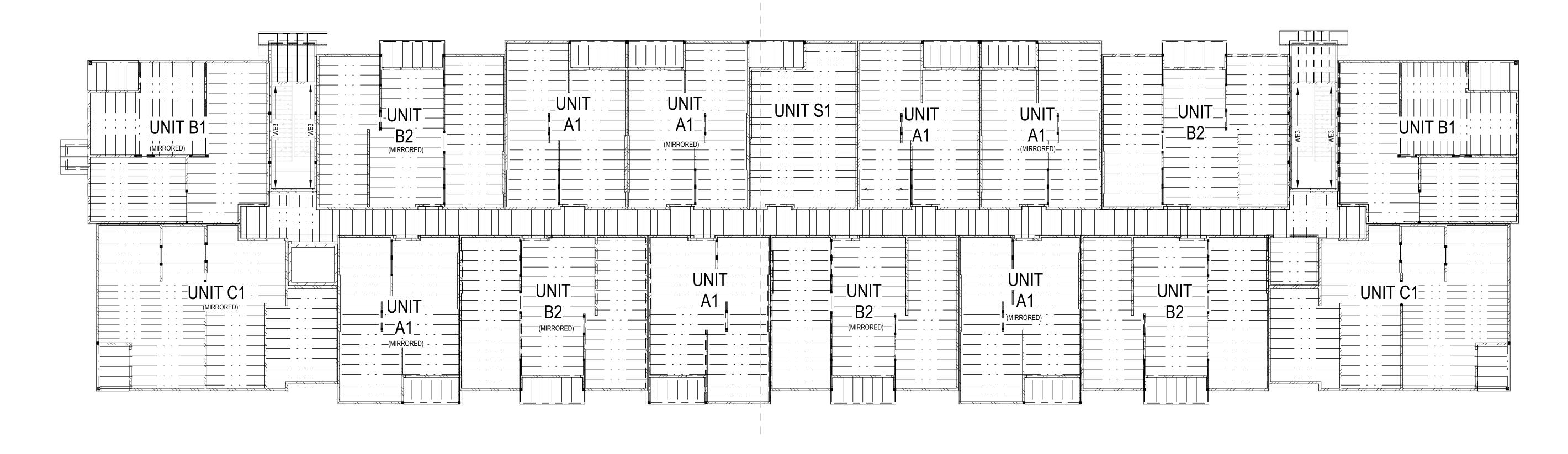
COLUMN LEGEND INDICATES COLUMN MARK. COLUMNS ARE ABOVE, BELOW, OR THROUGH AS SHOWN. SR-82

7780 LIGHTARD KNOTT LN FORT MYERS, FL 33905 22063

MILHAUS

SECOND FLOOR **OVERALL PLAN**

RS-102



SECOND FLOOR OVERALL PLAN
3/32" = 1'-0"



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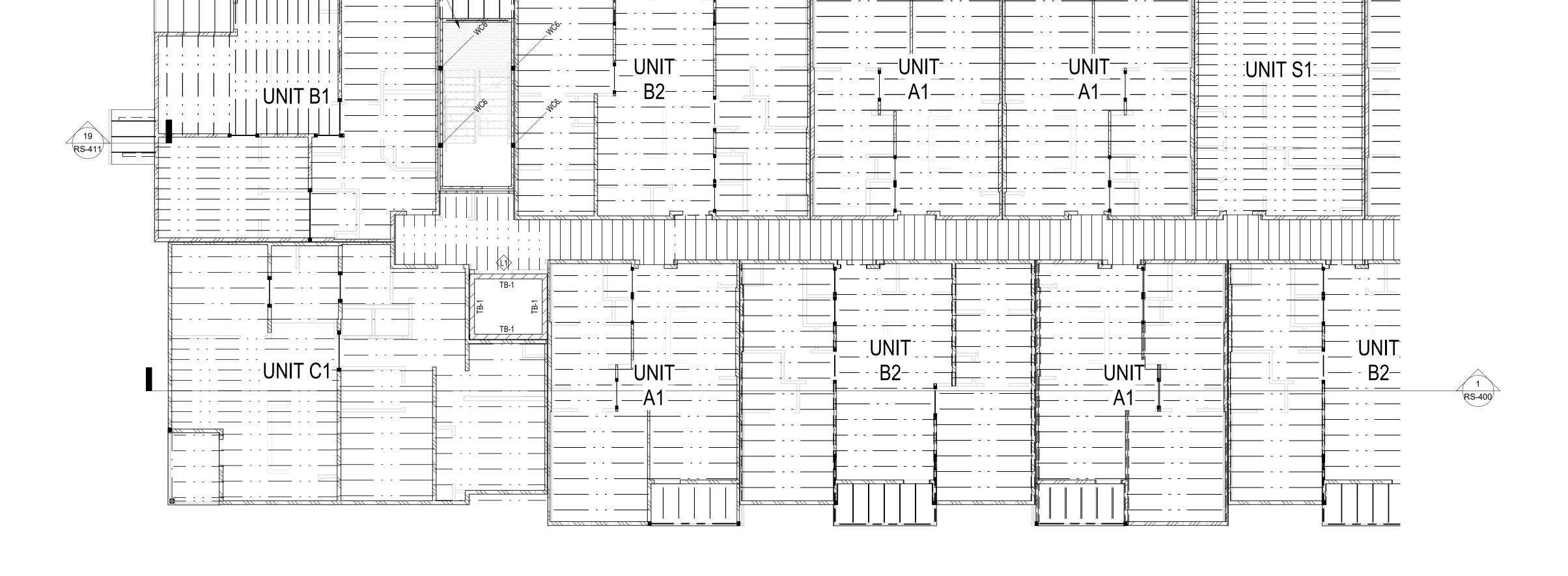
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SECOND FLOOR PLAN - AREA A

RS-102A



SECOND FLOOR FRAMING PLAN - AREA A

PRE-ENGINEERED STEEL -STAIR, BY DELEGATED ENGINEER.

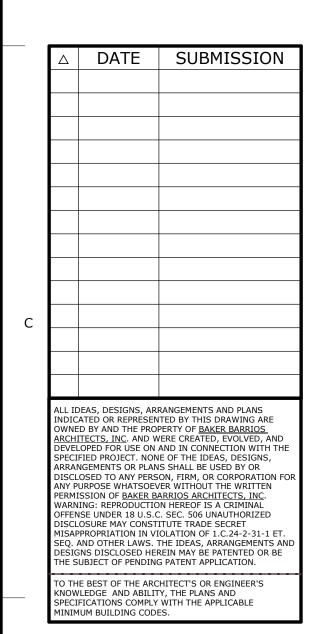


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1/5/2023 ENGINEER OF RECORD Cordell S. Van Nostrand FL P.E. # 67580

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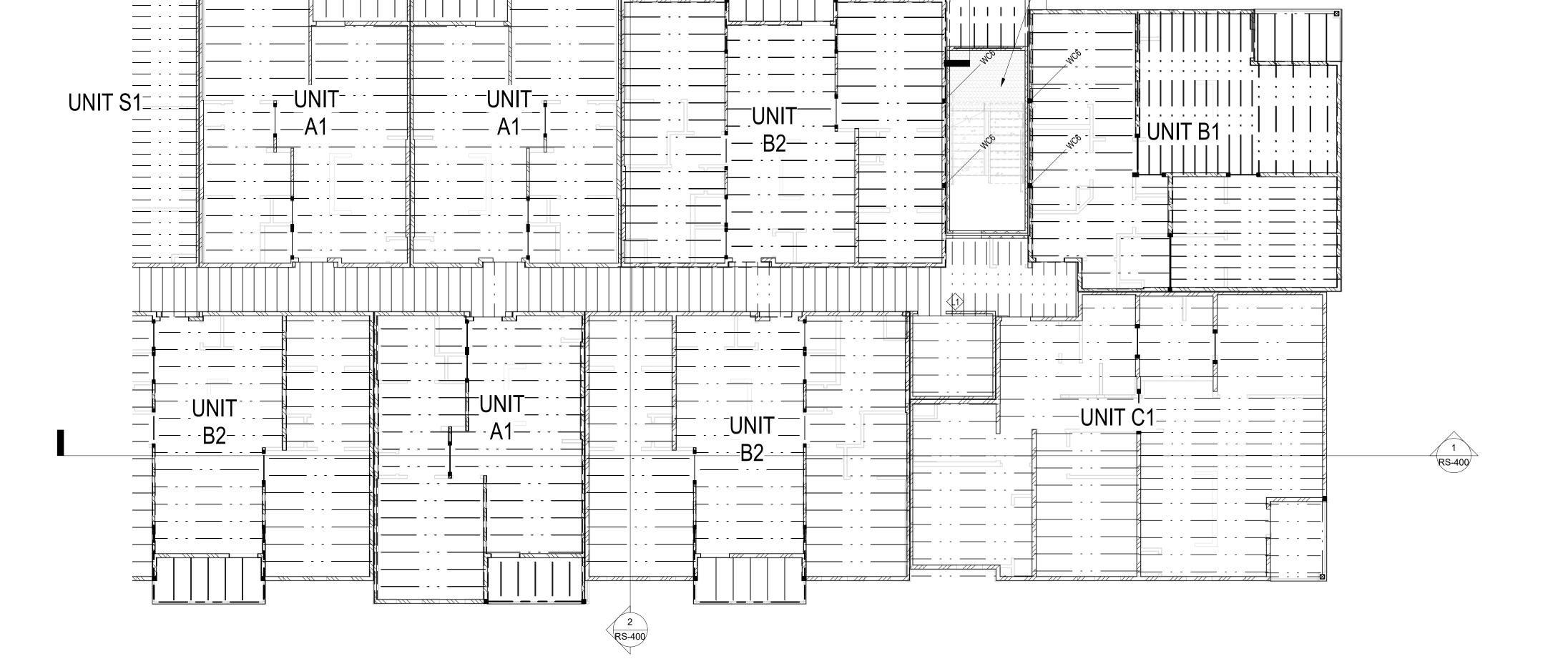
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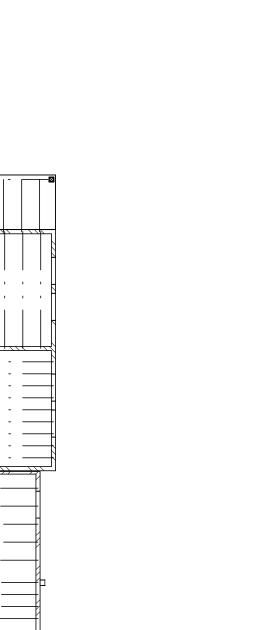
SECOND FLOOR PLAN - AREA B

RS-102B



SECOND FLOOR FRAMING PLAN - AREA B 1/8" = 1'-0"

PRE-ENGINEERED STEEL STAIR, BY DELEGATED ENGINEER.



Baker Barrios

ORLANDO

189 S. ORANGE AVE., SUITE 1700

ORLANDO, 189 S. ORANGE AVE., SUITE 1700

ORLANDO, FLORIDA 32801

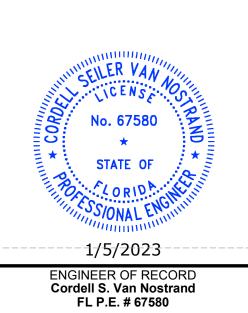
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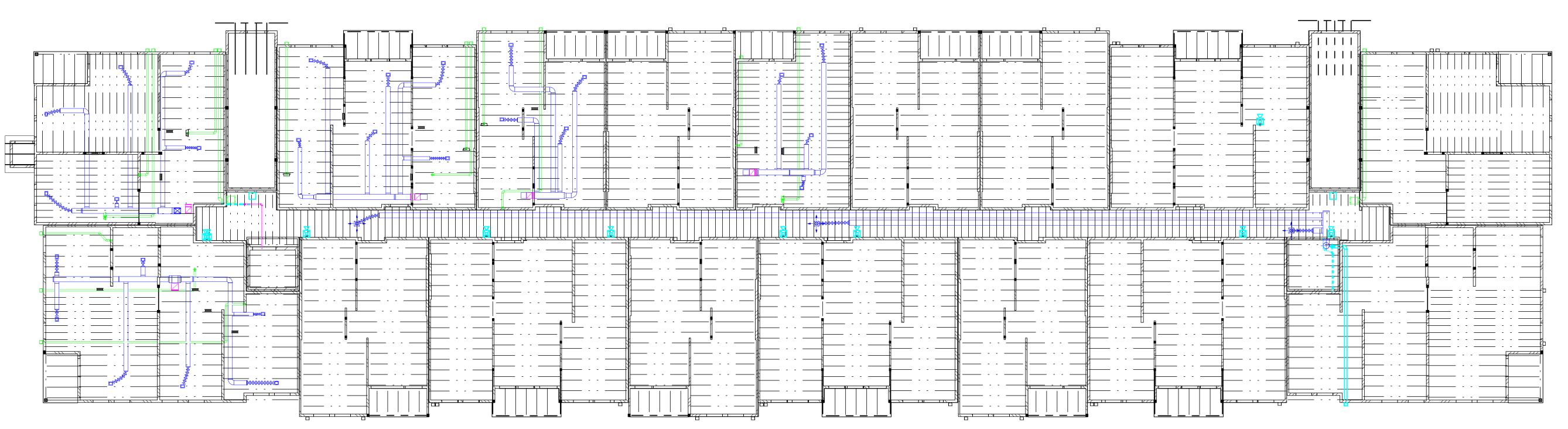
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7780 LIGHTARD KNOTT LN FORT MYERS, FL 33905
PROJECT NO:
22063

MECHINCAL COORDINATION PLAN

RS-102C



TYPICAL MECHANICAL COORDINATION PLAN 3/32" = 1'-0"

TYPICAL MECHANICAL COORDINATION PLAN NOTES:

- 1. THE APPROXIMATE LOCATION OF EACH DUCT FOR EACH UNIT TYPE HAVE BEEN PLACED ON THIS PLAN FOR COORDINATION PURPOSES. THE CONTRACTOR SHALL COORDINATE WITH THE MECHANICAL PLANS FOR EXACT DUCT SIZES, LOCATIONS, DIMENSIONS, AND ELEVATIONS.
- WHERE DUCTS RUN PERPENDICULAR TO THE TRUSS SPAN, THE DELEGATED TRUSS ENGINEER SHALL DESIGN FOR DUCTWORK TO RUN BETWEEN WEBS IF POSSIBLE, OR PROVIDE A CHASE WITHIN THE TRUSS SPACE WHEREVER NEEDED.
- 3. WHERE DUCTS RUN PARALLEL TO THE TRUSS SPAN, TRUSSES SHALL BE LOCATED TO AVOID DUCTS. MAXIMUM SPACING BETWEEN FLOOR TRUSSES IS 24" O.C. AT ALL LOCATIONS.

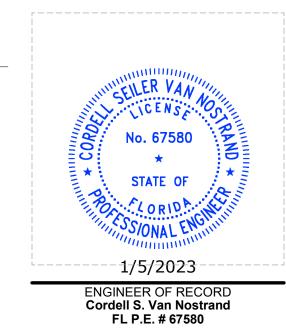
CONTRACTOR'S RESPONSIBILITY:

IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE ALL TRADES AND CONSULTANTS, CROSS REFERENCE THEIR DRAWINGS WITH THE OVERALL DESIGN, AND PROVIDE TO EACH A COMPLETE SET OF DRAWINGS AND SUBMITTALS TO INSURE COMPATIBILITY OF CONSTRUCTION PER DESIGN INTENT.

PRIOR TO TRUSS SHOP DRAWINGS AND TRUSS INSTALLATION, ALL PLUMBING, ELECTRICAL, AND MECHANICAL DRAWINGS SHALL BE COORDINATED WITH THE TRUSS LAYOUT TO ENSURE THAT THERE ARE NO CONFLICTS WITH DUCTS, RECESSED FIXTURES, PLUMBING PIPES, TRAPS, HOODS, CEILING STEPS/SLOPES, ETC. TRUSS LAYOUT SHALL BE MODIFIED AND/OR TRUSS CHASES SHALL BE ADDED TO AVOID CONFLICTS. TRUSS SPACING SHALL NOT EXCEED MAXIMUM NOTED IN PLAN NOTES, U.N.O.



Baker Barrios ORLANDO 189 S. ORANGE AVE., SUITE 1700 ORLANDO, FLORIDA 32801 407 926 3000 INFO@BAKERBARRIOS.COM **BAKERBARRIOS.COM** AA0002981 | LC26000427



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MILHAUS

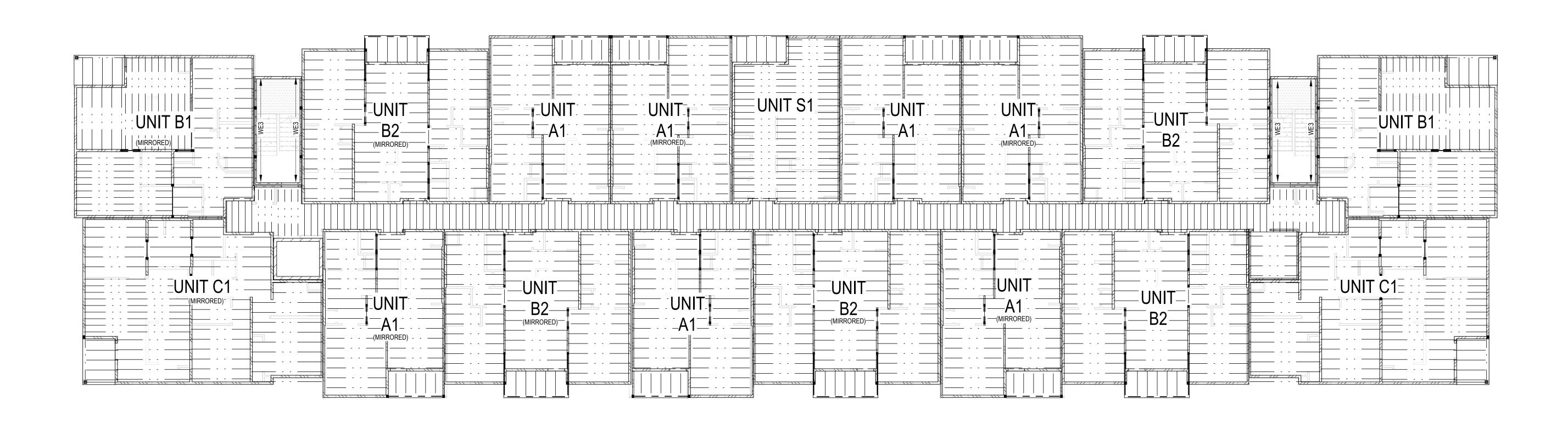
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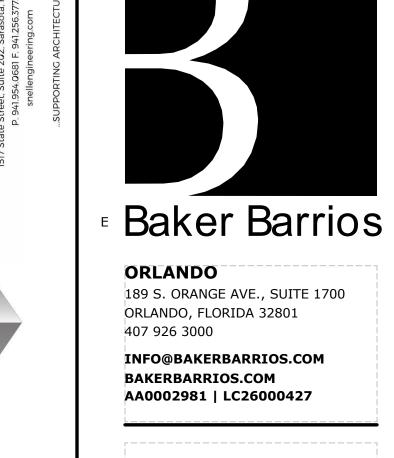
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THIRD FLOOR **OVERALL PLAN**

RS-103



THIRD FLOOR OVERALL PLAN
3/32" = 1'-0"





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PRE-ENGINEERED STEEL ——

STAIR, BY DELEGATED ENGINEER.

THIRD FLOOR FRAMING PLAN - AREA A
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THIRD FLOOR PLAN - AREA A

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1/5/2023 ENGINEER OF RECORD Cordell S. Van Nostrand FL P.E. # 67580

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- INDICATES A (2) 2x10 PT LEDGER FASTENED TO MASONRY BOND BEAM W/ 1/2"x6" TITEN HD ANCHORS @ 12" O.C. MAX STAGGERED. TOP OF LEDGER SET AT TOP OF FLOOR TRUSS AT FLOOR ELEVATIONS. BOTTOM OF LEDGER SET AT BOTTOM OF TRUSS AT ROOF ELEVATION.

INDICATES A 4" OR 6" LOAD BEARING WOOD FRAMED STUD WALL. SEE PLAN NOTES AND UNIT PLANS FOR MORE INFORMATION.

INDICATES AN 8" LOAD BEARING MASONRY WALL. PROVIDE 1#5 BAR IN GROUTED CELLS AT CORNERS, ENDS, INTERSECTIONS OF WALLS, UNDER POINT LOADS AND AT 24" O.C. MAX. ALL CELLS BELOW GRADE, OR IN CONTACT WITH SOIL, SHALL BE

COLUMN LEGEND INDICATES COLUMN MARK. COLUMNS ARE ABOVE, BELOW, OR THROUGH AS SHOWN.



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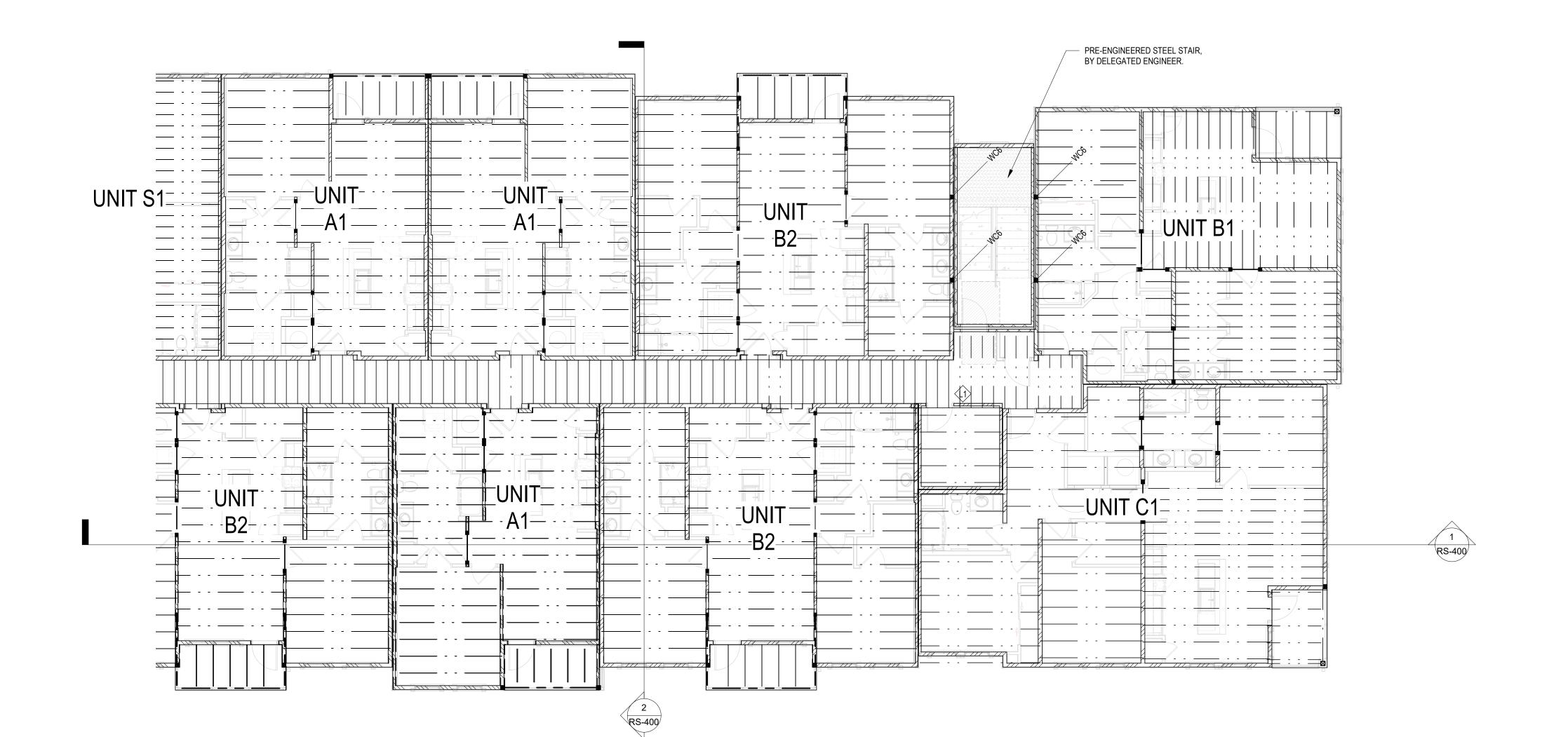
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7780 LIGHTARD KNOTT LN FORT MYERS, FL 33905 22063

THIRD FLOOR

PLAN - AREA B

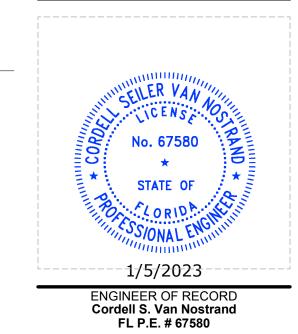
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THIRD FLOOR FRAMING PLAN - AREA B



Baker Barrios ORLANDO 189 S. ORANGE AVE., SUITE 1700 ORLANDO, FLORIDA 32801 407 926 3000 INFO@BAKERBARRIOS.COM **BAKERBARRIOS.COM** AA0002981 | LC26000427



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ANY PURPOSE WHATSOEVER WI' PERMISSION OF BAKER BARRIOS WARNING: REPRODUCTION HERI OFFENSE UNDER 18 U.S.C. SEC. DISCLOSURE MAY CONSTITUTE TMISAPPROPRIATION IN VIOLATIO			TED BY THIS DRAWING ARE PERTY OF BAKER BARRIOS. VERE CREATED, EVOLVED, AND AND IN CONNECTION WITH THE E OF THE IDEAS, DESIGNS, S SHALL BE USED BY OR ON, FIRM, OR CORPORATION FOR VER WITHOUT THE WRITTEN ARRIOS ARCHITECTS, INC. 1014 HEREOF IS A CRIMINAL 2. SEC. 506 UNAUTHORIZED ITUTE TRADE SECRET OLATION OF 1.C.24-2-31-1 ET. THE IDEAS, ARRANGEMENTS AND REIN MAY BE PATENTED OR BE
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FLOOR FRAMING PLAN NOTES:

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- ELEVATIONS SHOWN ARE RELATIVE TO THE INTERIOR GROUND FLOOR SLAB SURFACE SET AT 0'-0" (REF.). SEE CIVIL FOR TOP OF INTERIOR GROUND FLOOR SLAB ELEVATION.
- 3. SEE S-200 FOR ALL SCHEDULES.
- 4. ALL WOOD IN CONTACT WITH MASONRY, CONCRETE OR USED IN EXTERIOR APPLICATIONS SHALL BE PRESSURE TREATED WITH PRESERVATIVE. (P.T.) EXTERIOR CONNECTORS SHALL BE HOT DIPPED GALVANIZED
- 5. INTERIOR FLOOR FRAMING SHALL CONSIST OF 18" DEEP PRE-ENGINEERED FLOOR TRUSSES @ 24" O.C. MAX, U.N.O. ON PLAN. (TYPICAL ALL FLOOR FRAMING PLAN NOTES). FLOOR SHEATHING SHALL BE 23/32" APA RATED 48/24 PLYWOOD T&G SHEATHING ATTACHMENT IS TO BE GLUED AND SCREWED TO TRUSSES WITH #8 SCREWS AT 4" O.C. SPACING AT SUPPORTED PANEL EDGES AND 8" O.C. IN THE FIELD. ATTACH PLYWOOD AT ALL LEDGER LOCATIONS WITH #8 SCREWS AT 4" O.C. DESIGN ACCOMMODATES 7/8" GYPSUM TOPPING OVER 1/8" ACOUSTICAL MAT.
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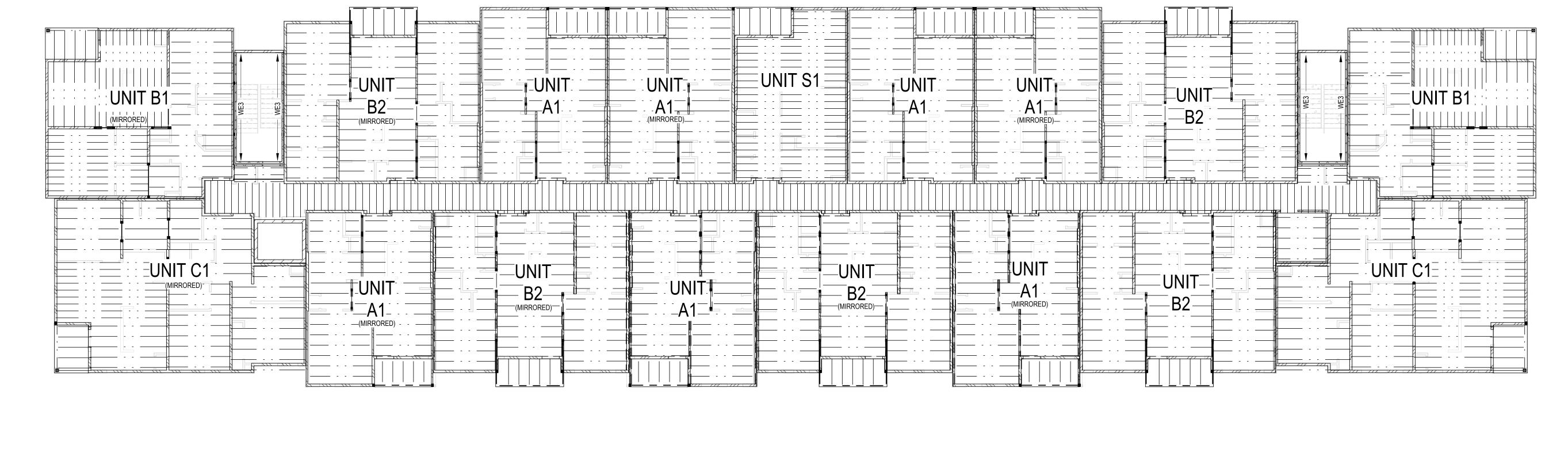
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FOURTH FLOOR OVERALL PLAN

RS-104



FOURTH FLOOR OVERALL PLAN 3/32" = 1'-0"



1/5/2023 ENGINEER OF RECORD Cordell S. Van Nostrand FL P.E. # 67580

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FOURTH FLOOR FRAMING PLAN - AREA A 1/8" = 1'-0"

PRE-ENGINEERED STEEL STAIR, BY DELEGATED

ENGINEER.

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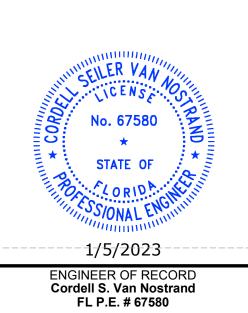
FOURTH FLOOR PLAN - AREA A

RS-104A



Baker Barrios

ORLANDO 189 S. ORANGE AVE., SUITE 1700 ORLANDO, FLORIDA 32801 407 926 3000 INFO@BAKERBARRIOS.COM **BAKERBARRIOS.COM** AA0002981 | LC26000427



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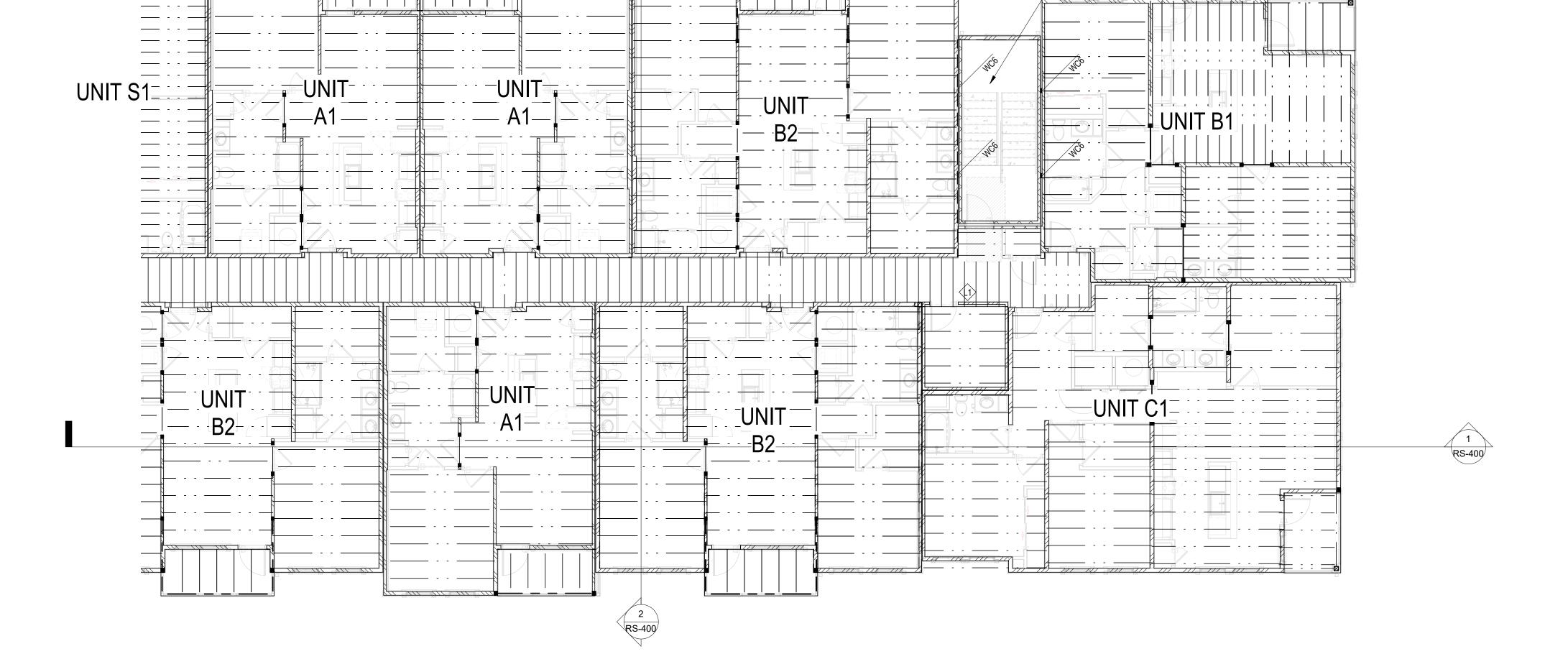
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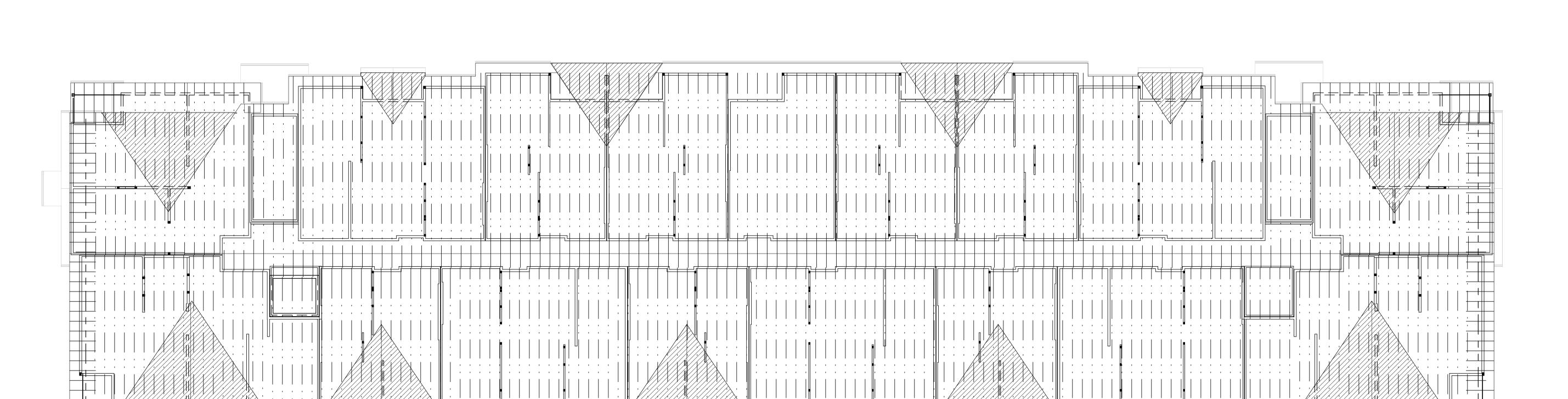
FOURTH FLOOR PLAN - AREA B

RS-104B



FOURTH FLOOR FRAMING PLAN - AREA B 1/8" = 1'-0"

PRE-ENGINEERED STEEL STAIR, BY DELEGATED ENGINEER.



ROOF FRAMING OVERALL PLAN
3/32" = 1'-0"

ORLANDO

189 S. ORANGE AVE., SUITE 1700
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407 926 3000
INFO@BAKERBARRIOS.COM
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1/5/2023

ENGINEER OF RECORD Cordell S. Van Nostrand FL P.E. # 67580

Baker Barrios

ALL IDEAS, DESIGNS, ARRANGEMENTS AND PLANS INDICATED OR REPRESENTED BY THIS DRAWING ARE OWNED BY AND THE PROPERTY OF BAKER BARRIOS ARCHITECTS, INC. AND WERE CREATED, EVOLVED, AND DEVELOPED FOR USE ON AND IN CONNECTION WITH THE SPECIFIED PROJECT. NONE OF THE IDEAS, DESIGNS, ARRANGEMENTS OR PLANS SHALL BE USED BY OR DISCLOSED TO ANY PERSON, FIRM, OR CORPORATION FOR ANY PURPOSE WHATSOEVER WITHOUT THE WRITTEN PERMISSION OF BAKER BARRIOS ARCHITECTS, INC. WARNING: REPRODUCTION HEREOF IS A CRIMINAL OFFENSE UNDER 18 U.S.C. SEC. 506 UNAUTHORIZED DISCLOSURE MAY CONSTITUTE TRADE SECRET MISAPPROPRIATION IN VIOLATION OF 1.C.24-2-31-1 ET. SEQ. AND OTHER LAWS. THE IDEAS, ARRANGEMENTS AND DESIGNS DISCLOSED HEREIN MAY BE PATENTED OR BE THE SUBJECT OF PENDING PATENT APPLICATION.

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ROOF FRAMING PLAN NOTES:

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- 2. ELEVATIONS SHOWN ARE RELATIVE TO THE INTERIOR GROUND FLOOR SLAB SURFACE SET AT 0'-0" (REF.). SEE CIVIL FOR TOP OF INTERIOR GROUND FLOOR SLAB ELEVATION.
- 3. SEE S-200 FOR ALL SCHEDULES.
- 4. ROOF FRAMING SHALL BE PRE-ENGINEERED WOOD ROOF TRUSSES @ 24" O.C. MAX. VERIFY CEILING HEIGHTS AND CONFIGURATIONS WITH THE ARCHITECTURAL PLANS. SEE ARCHITECTURAL PLANS FOR ROOF SLOPE AND EAVE DETAILS. TRUSS BEARING ELEVATION = 41'-0 3/4" UNLESS NOTED OTHERWISE ON PLAN.
- ROOF SHEATHING SHALL BE 19/32" APA RATED EXTERIOR PLYWOOD ROOF SHEATHING. ATTACHMENT FOR ZONES 1 & 2, AS DEPICTED IN THE COMPONENT AND CLADDING WIND SCHEDULE, SHALL BE MADE W/ 10d NAILS @ 6" O.C. AT SUPPORTED PANEL EDGES AND 12" O.C. IN THE FIELD. ATTACHMENT FOR ZONE 3 IS TO BE MADE W/ 10d NAILS @ 4" O.C. AT SUPPORTED PANEL EDGES AND IN THE FIELD. PROVIDE PLYWOOD "H" CLIPS @ UNSUPPORTED PLYWOOD PANEL EDGES.
- THE NOTE BELOW. THE THREADED ROD TIE DOWN SYSTEM SHALL BE INSTALLED FROM THE GROUND FLOOR PT SLAB EXTENDING UP TO THE DOUBLE TOP PLATE AT THE ROOF TRUSS BEARING ELEVATION (4'-0" MAX. THREADED ROD SPACING), DESIGNED FOR THE UPLIFT VALUES THAT ARE PROVIDED ON SHEETS S-010 AND S-011.

TRUSSES AND GIRDER TRUSSES SHALL BE HELD DOWN FOR UPLIFT LOADING VIA THE SPECIFIED SIMPSON UPLIFT CONNECTORS IN

- TYPICAL SINGLE PLY ROOF TRUSSES BEAR ONTO EXTERIOR WOOD WALLS, AND ONE OF THE 2X6 CORRIDOR WALL LINES WITH AN H10A UPLIFT CONNECTOR AT ALL LOCATIONS. TRUSSES ARE SEGMENTED AND STAGGERED AT THE INTERIOR CORRIDOR BEARING WALL LINE. H10-2 UPLIFT CONNECTORS SHALL BE USED FOR (2) PLY GIRDER TRUSS LOCATIONS.
- 8. THE TRUSS ENGINEER SHALL SPECIFY THE TRUSS-TO-TRUSS CONNECTIONS.
- 9. PLYWOOD SHEATHING SHALL BE CONTINUOUS ON THE TOP CHORD OF THE TRUSSES. ALL OVER-FRAMING OR VALLEY SETS SHALL BE PLACED ABOVE THE CONTINUOUS PLYWOOD.
- 10. ALL WOOD IN CONTACT WITH MASONRY, CONCRETE OR USED IN EXTERIOR APPLICATIONS SHALL BE PRESSURE TREATED WITH PRESERVATIVE. (P.T.) EXTERIOR CONNECTORS SHALL BE HOT DIPPED GALVANIZED.
- 11. PROVIDE 2x P.T. CONT FASCIA AT EDGE OF ROOF. PROVIDE MSTA24 ACROSS ALL SPLICE LOCATIONS.
- 11. PROVIDE 2x P. I. CONT FASCIA AT EDGE OF ROOF. PROVIDE MSTA24
 12. U=xxxx INDICATES TRUSS UPLIFT (IN lbs), IN EXCESS OF 1000 lbs.
- 13. R=xxxx INDICATES TRUSS GRAVITY LOAD (IN lbs), IN EXCESS OF 5000 lbs.
- 4. INDICATES A (2) 2X10 PT LEDGER FASTENED TO MASONRY BOND BEAM W/ 1/2"X6" TITEN HD ANCHORS @ 12" O.C. MAX. STAGGERED. TOP OF LEDGER SET AT TOP OF FLOOR TRUSS AT FLOOR ELEVATIONS. BOTTOM OF LEDGER SET AT BOTTOM OF TRUSS AT ROOF ELEVATION.

ROOF FRAMING OVERALL PLAN

7780 LIGHTARD KNOTT LN

FORT MYERS, FL 33905

22063

MILHAUS





Baker Barrios

ORLANDO

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> 1/5/2023 ENGINEER OF RECORD Cordell S. Van Nostrand FL P.E. # 67580

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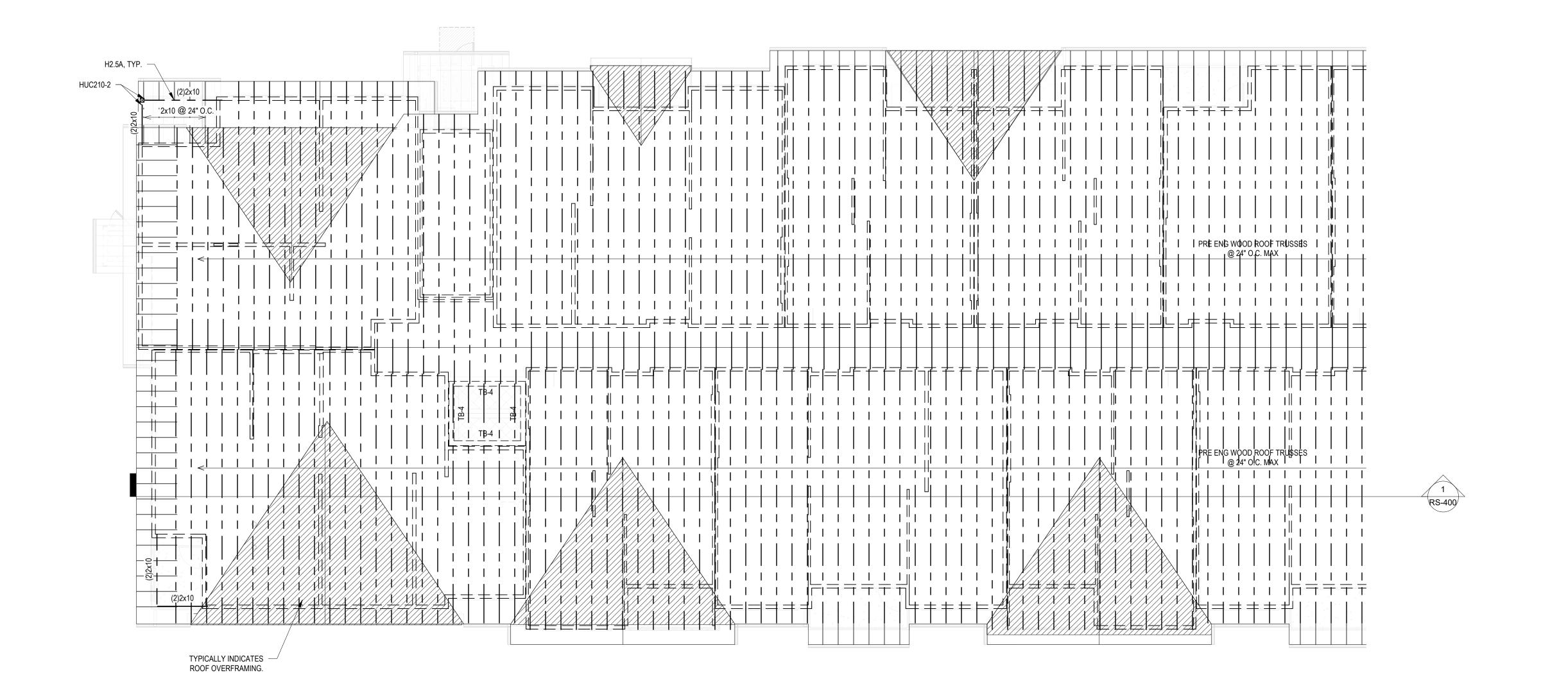
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ROOF FRAMING PLAN - AREA A

RS-105A



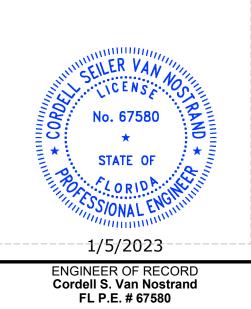
ROOF FRAMING PLAN - AREA A





ORLANDO 189 S. ORANGE AVE., SUITE 1700 ORLANDO, FLORIDA 32801

407 926 3000 INFO@BAKERBARRIOS.COM **BAKERBARRIOS.COM**



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- 3. SEE S-200 FOR ALL SCHEDULES.

ROOF FRAMING PLAN NOTES:

- ROOF FRAMING SHALL BE PRE-ENGINEERED WOOD ROOF TRUSSES @ 24" O.C. MAX. VERIFY CEILING HEIGHTS AND CONFIGURATIONS WITH THE ARCHITECTURAL PLANS. SEE ARCHITECTURAL PLANS FOR ROOF SLOPE AND EAVE DETAILS. TRUSS BEARING ELEVATION = 41'-0 3/4" UNLESS NOTED OTHERWISE ON PLAN.
- ROOF SHEATHING SHALL BE 19/32" APA RATED EXTERIOR PLYWOOD ROOF SHEATHING. ATTACHMENT FOR ZONES 1 & 2, AS DEPICTED IN THE COMPONENT AND CLADDING WIND SCHEDULE, SHALL BE MADE W/ 10d NAILS @ 6" O.C. AT SUPPORTED PANEL EDGES AND 12" O.C. IN THE FIELD. ATTACHMENT FOR ZONE 3 IS TO BE MADE W/ 10d NAILS @ 4" O.C. AT SUPPORTED PANEL EDGES AND IN THE FIELD. PROVIDE PLYWOOD "H" CLIPS @ UNSUPPORTED PLYWOOD PANEL EDGES.
- TRUSSES AND GIRDER TRUSSES SHALL BE HELD DOWN FOR UPLIFT LOADING VIA THE SPECIFIED SIMPSON UPLIFT CONNECTORS IN THE NOTE BELOW. THE THREADED ROD TIE DOWN SYSTEM SHALL BE INSTALLED FROM THE GROUND FLOOR PT SLAB EXTENDING UP TO THE DOUBLE TOP PLATE AT THE ROOF TRUSS BEARING ELEVATION (4'-0" MAX. THREADED ROD SPACING), DESIGNED FOR THE UPLIFT VALUES THAT ARE PROVIDED ON SHEETS S-010 AND S-011.
- TYPICAL SINGLE PLY ROOF TRUSSES BEAR ONTO EXTERIOR WOOD WALLS, AND ONE OF THE 2X6 CORRIDOR WALL LINES WITH AN H10A UPLIFT CONNECTOR AT ALL LOCATIONS. TRUSSES ARE SEGMENTED AND STAGGERED AT THE INTERIOR CORRIDOR BEARING WALL LINE. H10-2 UPLIFT CONNECTORS SHALL BE USED FOR (2) PLY GIRDER TRUSS LOCATIONS.
- 8. THE TRUSS ENGINEER SHALL SPECIFY THE TRUSS-TO-TRUSS CONNECTIONS.
- 9. PLYWOOD SHEATHING SHALL BE CONTINUOUS ON THE TOP CHORD OF THE TRUSSES. ALL OVER-FRAMING OR VALLEY SETS SHALL BE PLACED ABOVE THE CONTINUOUS PLYWOOD.
- 10. ALL WOOD IN CONTACT WITH MASONRY, CONCRETE OR USED IN EXTERIOR APPLICATIONS SHALL BE PRESSURE TREATED WITH PRESERVATIVE. (P.T.) EXTERIOR CONNECTORS SHALL BE HOT DIPPED GALVANIZED.
- 11. PROVIDE 2x P.T. CONT FASCIA AT EDGE OF ROOF. PROVIDE MSTA24 ACROSS ALL SPLICE LOCATIONS.
- 12. U=xxxx INDICATES TRUSS UPLIFT (IN lbs), IN EXCESS OF 1000 lbs.
- 13. R=xxxx INDICATES TRUSS GRAVITY LOAD (IN lbs), IN EXCESS OF 5000 lbs.
- INDICATES A (2) 2X10 PT LEDGER FASTENED TO MASONRY BOND BEAM W/ 1/2"X6" TITEN HD ANCHORS @ 12" O.C. MAX. STAGGERED. TOP OF LEDGER SET AT TOP OF FLOOR TRUSS AT FLOOR ELEVATIONS. BOTTOM OF LEDGER SET AT BOTTOM OF TRUSS AT ROOF ELEVATION.

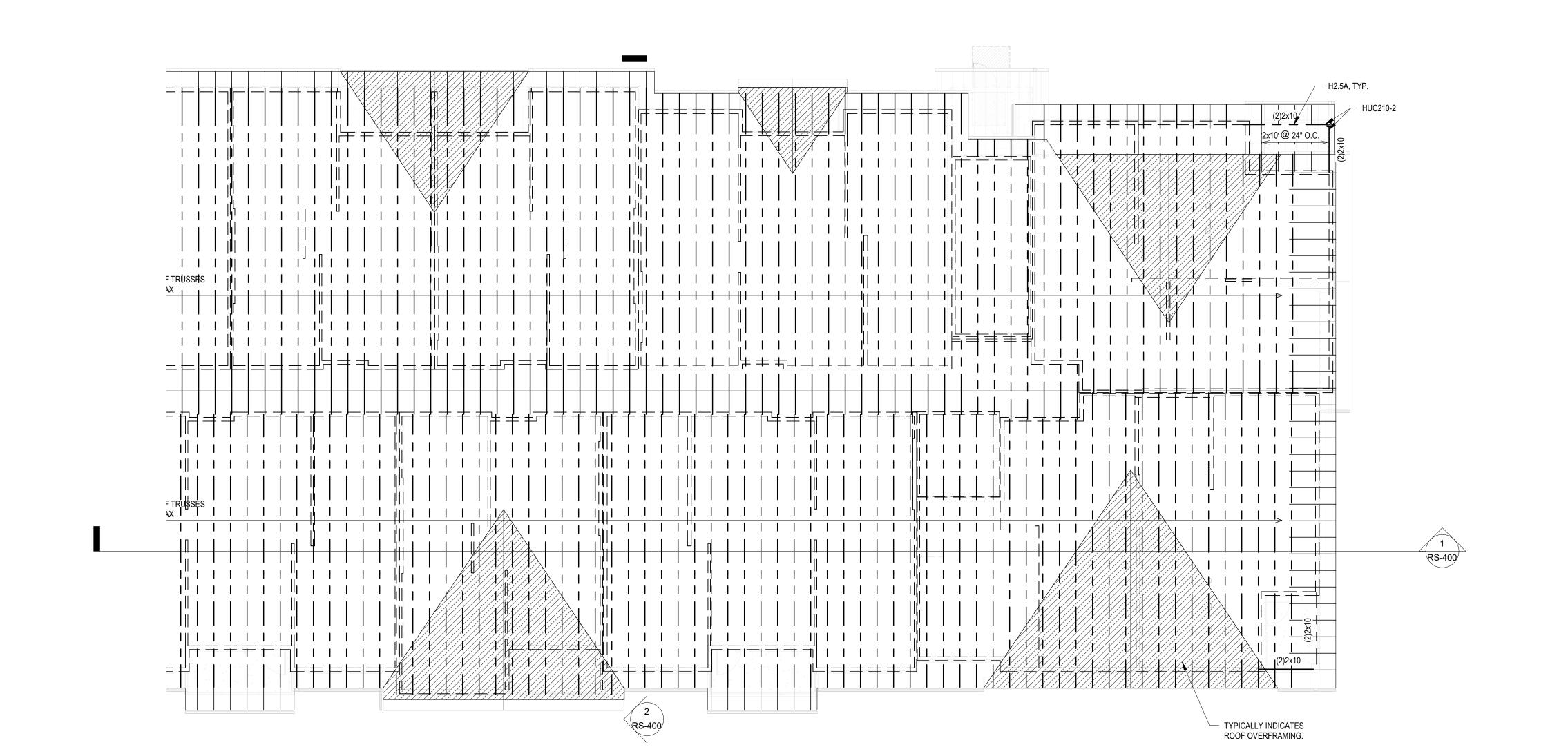
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ROOF FRAMING PLAN - AREA B

RS-105B



ROOF FRAMING PLAN - AREA B

1/8" = 1'-0"



ROPPED HEADER TYPE	SILL TYPE
H1	S1

H1

			_			
	DROPPED HEADER	SCHEDULE			SILL SCHEDU	JLE
MARK	DROPPED HEADER SIZE	CONN. @ EACH END		MARK	DROPPED HEADER SIZE	CONN. @ EACH END
H1	(2) 2x6	(3) 10d TOE NAILS		S1	(2) 2x6	(3) 10d TOE NAILS
H2	(3) 2x6	(2) A34 CLIP ANGLE		S2	(3) 2x6	(2) A34 CLIP ANGLE

MSTA18

MSTA18

(3) 2x10

WC2

4TH	(2) 2X6	(2) 2X6	(2) 2X6	(2) 2X4	6X6 PT	-
3RD	(2) 2X6	(2) 2X6	(2) 2X6	(3) 2X4	6X6 PT	(3) 2X4
2ND	(2) 2X6	(3) 2X6	(4) 2X6	(4) 2X4	6X6 PT	(3) 2X4
GROUND	(2) 2X6	(3) 2X6	(5) 2X6	3 1/2" x 5 1/4" PSL	6X6 PT	(4) 2X4
FLOOR COLUMN	WC1	WC2	WC3	WC4	WC5	WC6

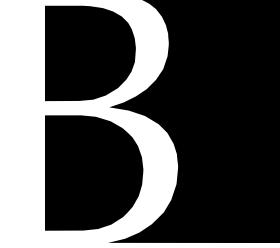
			WALL FRAMING	SCHEDULE		
4TH	2X4 @ 12" O.C.	2X6 @ 16" O.C.	2X6 @ 12" O.C.	2X6 @ 16" O.C.	(2) 2X4 @ 16" O.C.	2X4 @ 16" O.C.
3RD	(2) 2X4 @ 16" O.C.	2X6 @ 16" O.C.	(2) 2X6 @ 16" O.C.	2X6 @ 12" O.C.	(2) 2X4 @ 16" O.C.	2X4 @ 12" O.C.
2ND	(2) 2X4 @ 12" O.C.	2X6 @ 12" O.C.	(2) 2X6 @ 12" O.C.	(2) 2X6 @ 16" O.C.	(2) 2X4 @ 12" O.C.	2X4 @ 12" O.C.
GROUND	(3) 2X4 @ 12" O.C.	(2) 2X6 @ 16" O.C.	(3) 2X6 @ 12" O.C.	(2) 2X6 @ 12" O.C.	(2) 2X4 @ 12" O.C.	(2) 2x4 @ 16" O.C.
FLOOR	W1	W2	W3	WE1	WE2	WE3

EXTERIOR

INTERIOR

WALL

- 6. ALL CORRIDOR WALLS TO BE 2x6, UNLESS NOTED OTHERWISE. 7. SEE UNIT PLANS FOR UNIT WALL AND FLOOR FRAMING.
- 8. $\langle \chi \rangle$ INDICATES EXTERIOR OPENING TYPE. SEE TYPICAL DETAIL AND SCHEDULES FOR FRAMING AND COLUMN INFORMATION.
- 9. WHERE TRUSSES RUN PARALLEL WITH EXTERIOR WOOD WALL, PROVIDE TRUSS BRACING AT 24" O.C. MAX. SEE SECTION.
- INTERIOR FLOOR FRAMING SHALL CONSIST OF 18" DEEP PRE-ENGINEERED FLOOR TRUSSES AT 24" O.C. MAX. DELEGATED ENGINEER SHALL REDUCE TRUSS SPACING (FROM 24" O.C.) AS REQUIRED TO MEET DEFLECTION REQUIREMENTS PROVIDED IN STRUCTURAL NOTES ON S-001. FLOOR SHEATHING SHALL BE 23/32" APA RATED 48/24 PLYWOOD T&G SHEATHING. SHEATHING ATTACHMENT IS TO BE GLUED AND SCREWED TO TRUSSES WITH #8 WOOD SCREWS AT 4" O.C. SPACING AT SUPPORTED PANEL EDGES AND 8" O.C. IN THE FIELD. ATTACH PLYWOOD AT ALL LEDGER LOCATIONS WITH #8 SCREWS @ 4" O.C. MAX. DESIGN ACCOMMODATES 7/8" GYPSUM TOPPING OVER 1/8 ACOUSTICAL MAT.
- BALCONY FLOOR FRAMING SHALL CONSIST OF 15" DEEP PRE-ENGINEERED FLOOR TRUSSES AT 24" O.C. MAX. PROVIDE PLYWOOD FLOOR SHEATHING AND ATTACHMENT PER NOTE ABOVE. SEE SECTIONS FOR MORE INFORMATION INCLUDING GYPSUM TOPPING THICKNESS AND SLOPE."



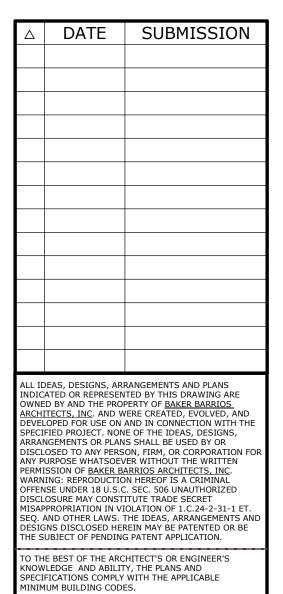
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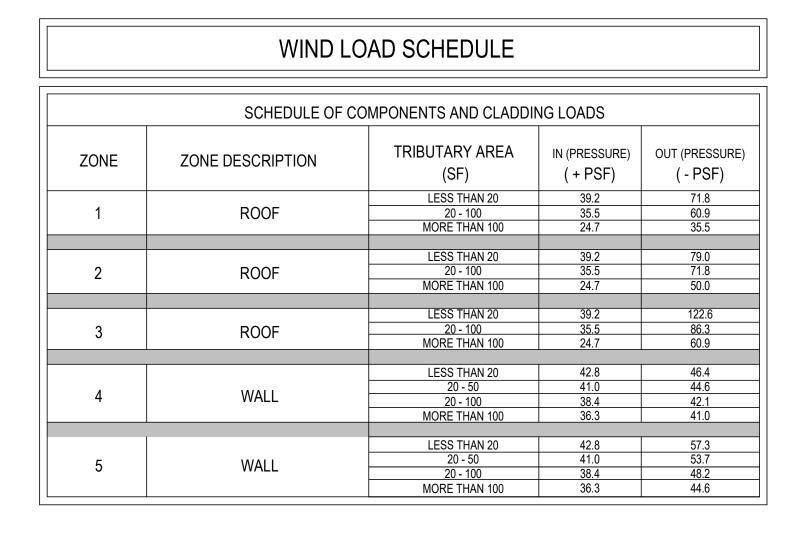
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UNIT PLANS



NOTE: WIND PRESSURES SHOWN ARE BASED ON Vasd

CODE =	ASCE 7-16		
ULTIMATE WIND SPEED Vult =	152 MPH		
ALLOWABLE WIND SPEED Vasd =	118 MPH		
RISK CATEGORY =	П		
EXPOSURE =	С		
ENCLOSURE CLASSIFICATION =	ENCLOSED		
INTERNAL PRESSURE COEFFICIENT (GCpi) =	±0.18		
a =	10.0 FT		
2a =	20.0 FT		
	HIP RO	OF (27° < Θ ≤ 45°	
2 2		SCALE: N.T.S.	
σ <u>3</u> <u>2</u> <u>3</u>	Interior Zones ZONE 1 - ROOF	End Zones ZONE 2 - ROOF	Corner Zone ZONE 3 - ROOF

WIND LOAD SCHEDULE					
	SCHEDULE OF C	OMPONENTS AND CLADDI	NG LOADS		
ZONE	ZONE DESCRIPTION	TRIBUTARY AREA (SF)	IN (PRESSURE) (+PSF)	OUT (PRESSURE	
1	ROOF	LESS THAN 20 20 - 100 MORE THAN 100	28.4 21.9 15.5	54.2 54.2 39.7	
2	ROOF	LESS THAN 20 20 - 100 MORE THAN 100	28.4 21.9 15.5	86.5 76.8 51.0	
3	ROOF	LESS THAN 20 20 - 100 MORE THAN 100	28.4 21.9 15.5	122.0 83.3 63.9	
4	WALL	LESS THAN 20 20 - 50 20 - 100 MORE THAN 100	38.1 36.5 34.2 32.3	41.3 39.7 37.4 36.5	
5	WALL	LESS THAN 20 20 - 50 20 - 100 MORE THAN 100	38.1 36.5 34.2 32.3	51.0 47.8 42.9 39.7	

NOTE: WIND PRESSURES SHOWN ARE BASED ON Vasd

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INTERNAL PRESSURE COEFFICIENT (GCpi) =	±0.18		
a =	10.0 FT		
2a =	20.0 FT		
2 2 3	h 5	4	5
	5 4 5	5	5
	5 4 5	OF (7° < Θ ≤ 27°	5
	5 4 5	5	5



WOOD COLUMN SCHEDULE

 MARK
 ELEV.
 SIZE (WxH)
 REINF. BOT.
 REINF. TOP
 STIRRUPS

 TB-1
 10' - 8"
 7 5/8 X 16
 2#5
 2#5

SEE STEEL STAIR DRAWINGS FOR CONNECTION OF LANDING BEAM AT EACH FLOOR LEVEL.

(2) GROUTED MASONRY K.O. COURSE

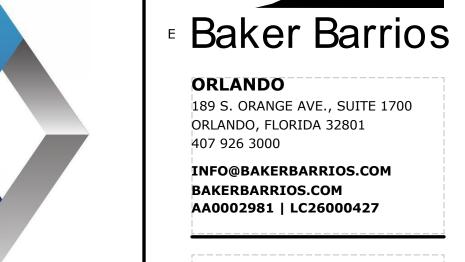
(2) GROUTED MASONRY K.O. COURSE
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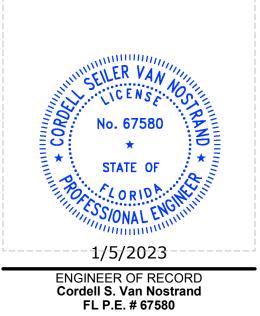
CONCRETE TIE BEAM SCHEDULE

TYPE ATTACHMENT

TB-2 21' - 11 1/2" 7 5/8 X 16 2#5
TB-3 32' - 0" 7 5/8 X 16 2#5
TB-4 42' - 8" 7 5/8 X 16 2#5

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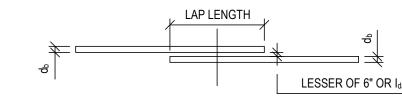
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SCHEDULES



	TE	TENSION (CLASS 'B') LAP SPLICE LENGTH			COMPRESSION LAP / SPLICE LENGTH
BAR SIZE	TOP BARS		OTHER BARS		ALL BARS
	3000 PSI	4000 PSI	3000 PSI	4000 PSI	ALL CONCRETE WITH fc3 3000 psi
#3	28	24	22	19	12
#4	37	32	29	25	15
#5	47	40	36	31	19
#6	56	48	43	37	23
#7	81	70	63	54	27
#8	93	80	72	62	30
#9	105	91	81	70	34
#10	118	102	91	79	38
#11	131	113	101	87	43
#14	121	105	93	81	-



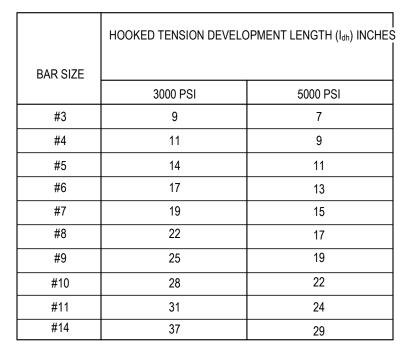
- REFER TO "HOOKED REINFORCEMENT TENSION DEVELOPMENT LENGTH SCHEDULE" WHEN THE STRAIGHT DEVELOPMENT LENGTH IN TENSION CANNOT BE ACCOMMODATED IN THE CONCRETE SECTION. ALWAYS USE TENSION LAP SPLICE LENGTH VALUES, UNLESS THE PLANS OR DETAILS NOTE OTHERWISE. TABULATED DEVELOPMENT AND LAP SPLICE LENGTHS ARE BASED ON REINFORCING STEEL YIELD STRENGTH Fy=60 KSI, NORMAL WEIGHT CONCRETE, AND CLASS B LAPS. TOP BARS ARE DEFINED AS HORIZONTAL BARS WITH MORE THAN 12 INCHES OF FRESH CONCRETE CAST IN THE MEMBER
- BELOW THE DEVELOPMENT LENGTH OR SPLICE. TOP BAR FACTOR DOES NOT APPLY TO BARS IN WALLS. WHEN DIFFERENT BAR DIAMETERS ARE SPLICED, USE SMALLER BAR LAP SPLICE LENGTH. ALL TABULATED VALUES ARE MINIMUM LENGTH, IN CASE OF CONFLICT WITH PLANS, SECTIONS, OR DETAILS USE LONGER LENGTH.
- d/b = BAR DIAMETER I/d = DEVELOPMENT, LAP OR SPLICE LENGTH. ADJUST TABULATED LENGTH BY THE FOLLOWING FACTORS WHERE APPLICABLE. NOTE THAT FACTORS ARE CUMULATIVE
 - LIGHT WEIGHT CONCRETE: 3 OR LESS BUNDLED BARS: 4 OR MORE BUNDLED BARS: CLEAR SPACING LESS THAN 2d/b AND CLEAR COVER LESS THAN d/b CLASS A LAP SPLICE
- EPOXY COATED BARS WELDED AND/OR MECHANICAL SPLICES MAY BE USED AT THE GENERAL CONTRACTOR'S OPTION PROVIDED THAT THE SPLICE IS CAPABLE OF DEVELOPING AT LEAST 125% OF THE YIELD STRENGTH OF THE LARGER BAR IN TENSION. WHERE WELDED AND/OR MECHANICAL SPLICES ARE TO BE USED. THE GENERAL CONTRACTOR SHALL SUBMIT FULL DATA OF THE PROPOSED MATERIAL, PROCEDURES, AND INSTALLATION INSTRUCTIONS TO THE ENGINEER FOR REVIEW AS A SHOP DRAWING SUBMISSION.

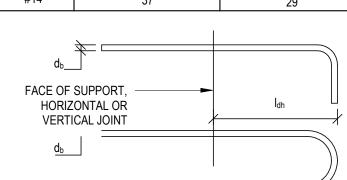
USE MECHANICAL COUPLERS FOR #14 AND LARGER BARS. LAP SPLICES IN CONCRETE MASONRY SHALL BE AS SPECIFIED IN STRUCTURAL NOTES.

TYP. STRAIGHT REINFORCEMENT DEVELOPMENT

SCALE: NTS

AND SPLICE LENGTH SCHEDULE



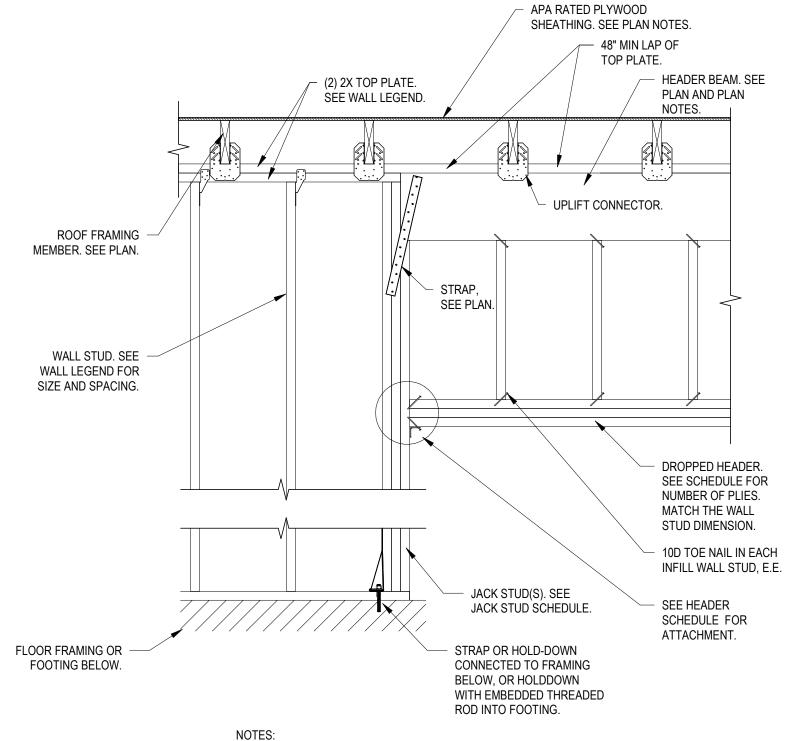


SEE TYPICAL TIE AND STIRRUP HOOKS DETAIL FOR ADDITIONAL INFORMATION. TABULATED DEVELOPMENT LENGTH ARE BASED ON REINFORCING STEEL YIELD STRENGTH F/y = 60 KSI AND NORMAL WEIGHT CONCRETE. ALL TABULATED VALUES ARE MINIMUM LENGTHS. IN CASE OF CONFLICT WITH THE PLANS, SECTIONS, OR DETAILS, USE THE LONGER LENGTH. d_b = BAR DIAMETER Idh = DEVELOPMENT LENGTH ADJUST TABULATED LENGTHS BY THE FOLLOWING FACTORS WHERE APPLICABLE. NOTE THAT THE FACTORS ARE CUMULATIVE. REINFORCING BAR STRENGTH OTHER THAN 60 KSI: (F_y/60 KSI) LIGHT WEIGHT CONCRETE: 1.30

1.20

TYP. HOOKED REINFORCEMENT TENSION **DEVELOPMENT LENGTH SCHEDULE** SCALE: NTS

EPOXY COATED BARS



PLYWOOD SHEATHING, IF APPLICABLE, IS NOT SHOWN FOR CLARITY. A MINIMUM OF (1)JACK AND (1)KING STUD ARE REQUIRED AT ALL OPENINGS, UNLESS NOTED OTHERWISE ON PLAN.

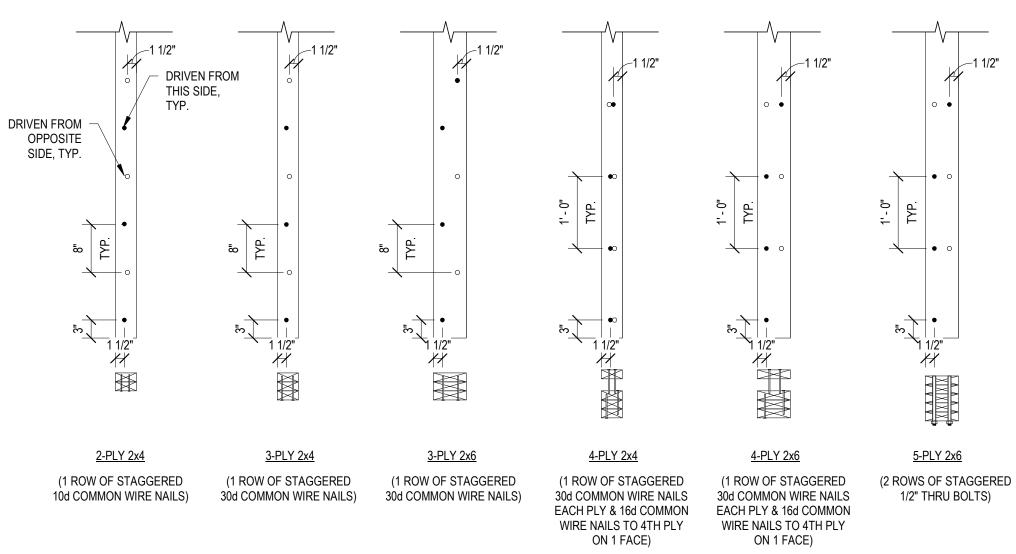
WHERE MULTIPLE JAMB STUDS ARE SPECIFIED EACH JAMB STUD IS TO BE STRAPPED ACCORDING TO PLAN NOTES. WHERE BUILT-UP STUD COLUMNS ARE NOTED ON PLAN, BUILT-UP COLUMN INCLUDES JACK STUDS.

TYPICAL WOOD WALL OPENING DETAIL

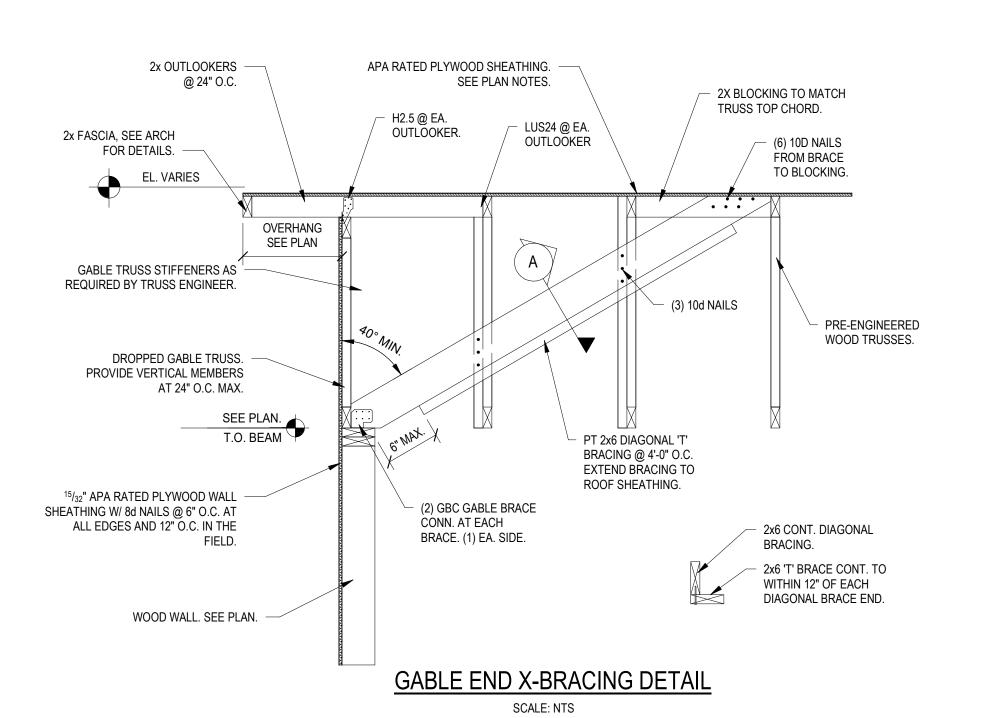
NOTES:

ALLOWABLE PENETRATION DIMENSIONS

SHOWN ARE FOR PENETRATIONS NOT HIGHER THAN 36" ABOVE FLOOR LEVEL. ONLY ONE PENETRATION IS ALLOWED







PER STUD. CONTACT STRUCTURAL ENGINEER FOR REVIEW OF REQUESTED PENETRATIONS 7 3 1/2" FOR 2X4 5 1/2" FOR 2X6 BEYOND THESE LIMITS. NAIL STOP BY CONTRACTOR PENETRATION THROUGH WOOD AS REQUESTED. STUD, MAKE TOP AND BOTTOM OF OPENING ROUNDED. GAP 'H' REQUIRED ABOVE AND BELOW FOR DIFFERENTIAL CONDUIT OR HORIZONTAL MOVEMENT, REFER TO TABLE PLUMBING RUN. BELOW. TOP OF OPENING SHALL BE NO MORE THAN 36" ABOVE THE **WOOD SHRINKAGE TABLE** SILL PLATĘ BELOW SHRINKAGE STORY LEVEL (INCHES) TOP OF OPENING SHALL BE NO MORE THAN 36" ABOVE THE LEVEL 3 - LEVEL R LEVEL 2 - LEVEL 3 3/4" SILL PLATE BELOW 7/8" MIN. LEVEL 1 - LEVEL 2 1/2" NOTE: H = SHRINKAGE (FROM TABLE)PLUS ANTICIPATED MAX OPENING IN BEARING MOVEMENT OF CONDUIT OR OR EXTERIOR STUD: PIPE (PER MEP ENGINEER) 1 1/2" FOR 2X4 2 1/4" FOR 2X6

ALLOWABLE SLOTTED PLUMBING PENETRATION

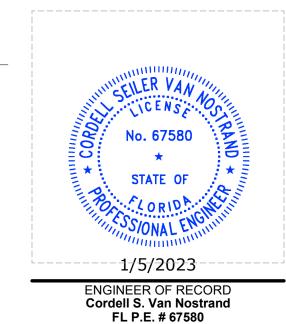
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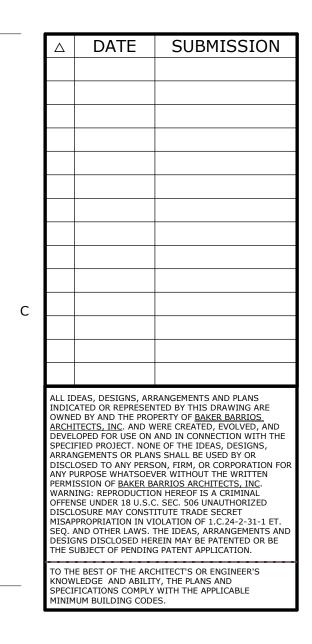
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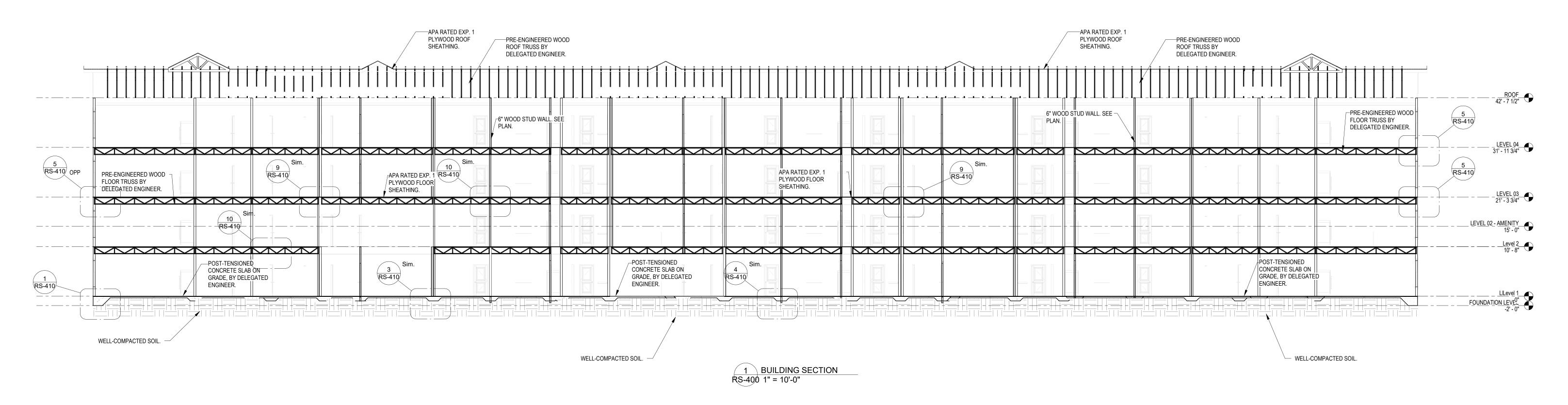
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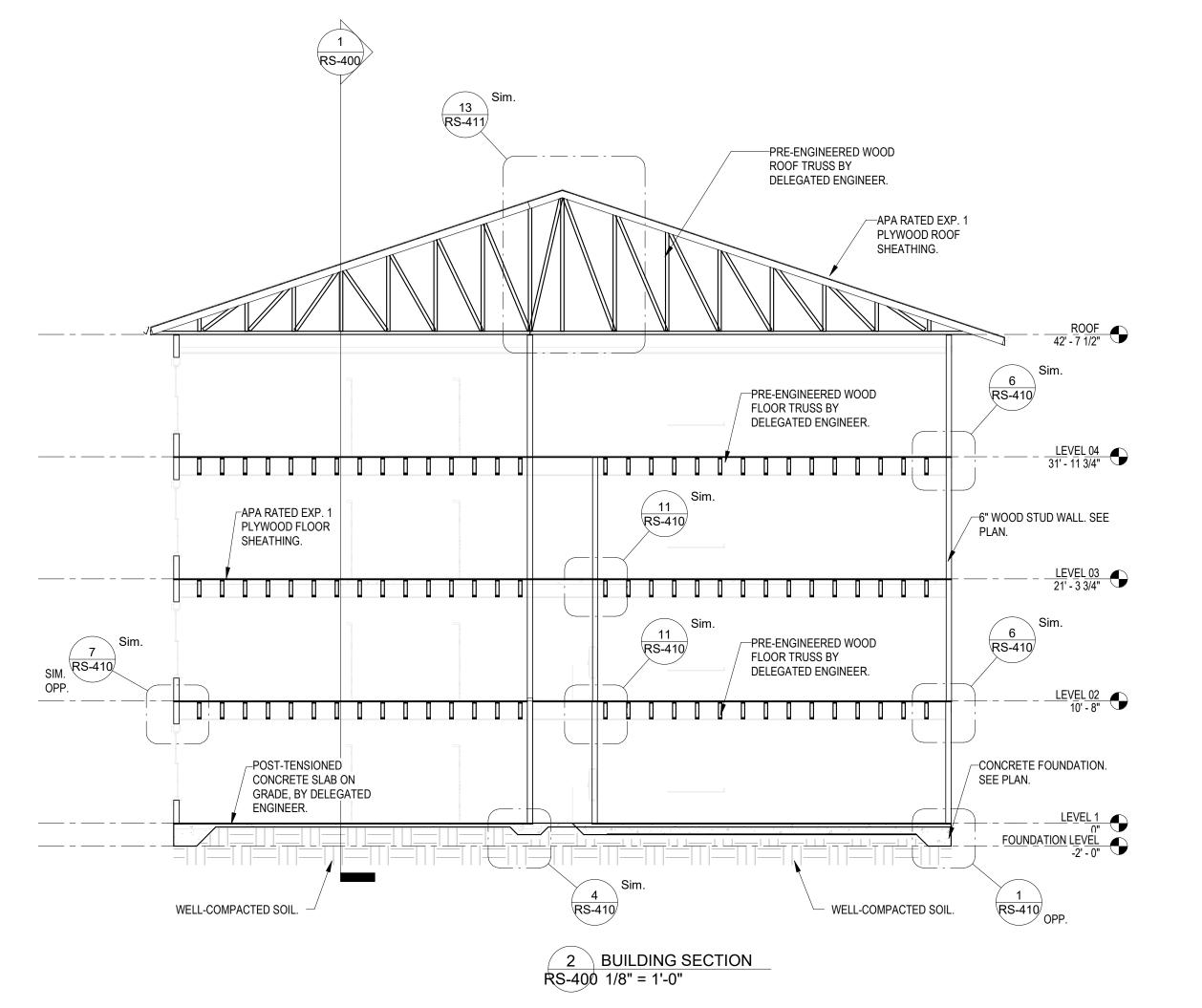
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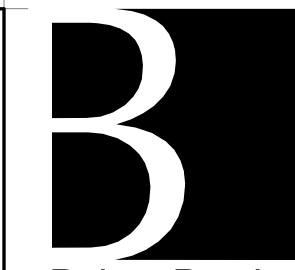
TYPICAL DETAILS **RS-300**











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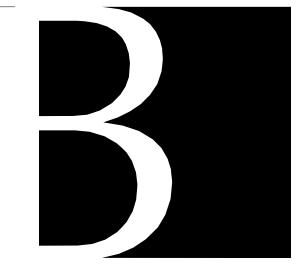
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BUILDING

SECTIONS



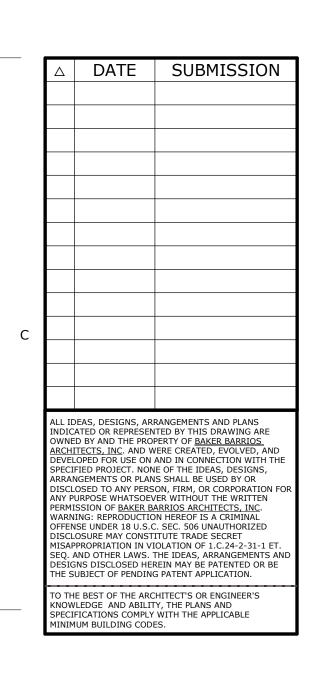
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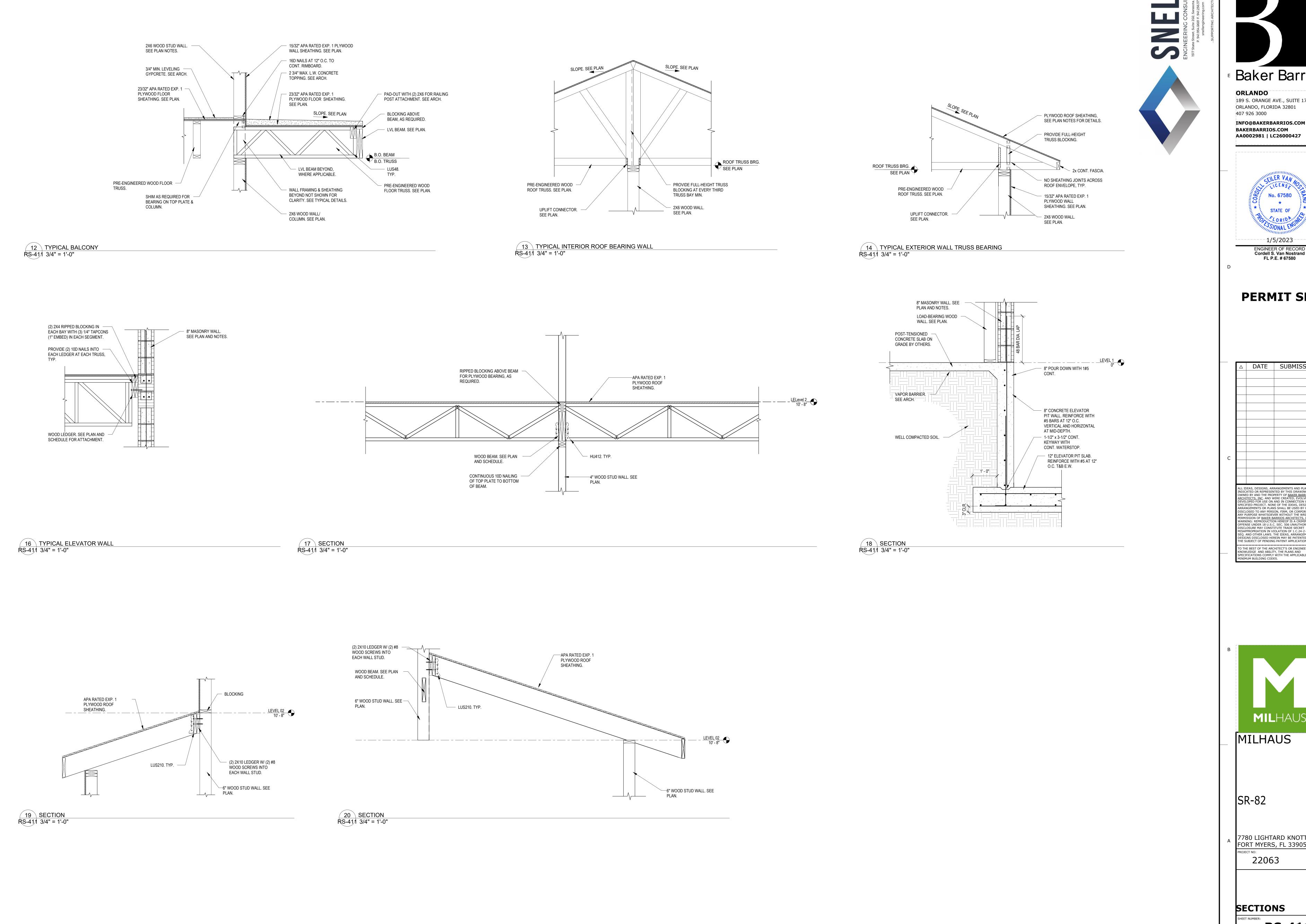


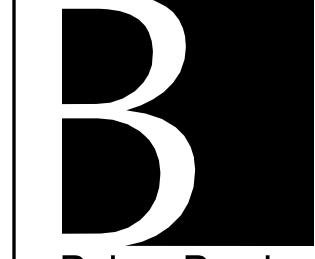
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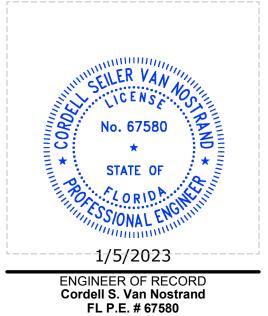
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-	KNOW SPECI	LEDGE AND ABILIT	WITH THE APPLICABLE



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SECTIONS

GENERAL NOTES:

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

ALL DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK. DO NOT SCALE DRAWINGS.

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS.

ELECTRONIC VERSIONS OF THE STRUCTURAL DRAWINGS ARE THE SOLE, COPYRIGHTED PROPERTY OF SNELL ENGINEERING AND ARE NOT TO BE USED OR TRANSFERRED WITHOUT THE EXPRESS, WRITTEN PERMISSION OF SNELL ENGINEERING. **DESIGN LOADS**:

THE STRUCTURAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE -BUILDING 7TH EDITION (2020). THE FOLLOWING SUPERIMPOSED LOADINGS HAVE BEEN UTILIZED:

ING / TH EDITION (2020). THE FOLLOWING SC	JPERIMPUSED LUADINGS	HAVE BI	EEN UTILIZED:
WIND:			
ASCE 7-16			
ULTIMATE WIND SPEED	-		52 MPH
ALLOWABLE WIND SPEED	-	1	18 MPH
EXPOSURE C			
ENCLOSED STRUCTURE	ICNIT		1.0.40
INTERNAL PRESSURE COEFFICI RISK FACTOR II	ENI -	+	-/- 0.18
SEE WIND SCHEDULE FOR PRES	CCLIDEC		
SEE WIND SCHEDOLE FOR FILE	JOUNLO		
ROOF:			
LIVE LOAD	-	2	20 PSF.
LIVE LOAD (CONCENTRATED)	-	3	800 LBS.
DEAD LOAD	-	2	25 PSF.
DEAD LOAD (AVAILABLE TO RES	SIST UPLIFT) -	5	SPSF.
FLOOD:			
FEMA A/V ZONE			
FLOOD DESIGN CLASS	-	I	I
BFE ELEVATION	-	1	1.00 NAVD
FREEBOARD	-	1	FT
PROPOSED LOWEST FLOOR ELE			2.00 NAVD
BOT. OF LOWEST HORIZ. STRUC			2.00 NAVD
REQ'D MIN. DRY FLOOD-PROOF			12.00 NAVD
			ESIGNED, CONNECTED AND ANCHORED TO
•			IENT DUE TO STRUCTURAL LOADS AND 'ATION IN CONFORMANCE WITH ASCE 24-14
FBC-R 322.1.2/FBC 1612.1.	QUAL TO THE DESIGN FLOC		ATION IN CONFORMANCE WITH ASCE 24-14
SEISMIC:			
RISK CATEGORY	_	ı	1
SEISMIC IMPORTANCE FACTOR	le -		1.0

IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO CONFIRM ALL FINAL WEIGHTS OF EQUIPMENT (MECHANICAL, CHILLED WATER, KITCHEN, SPA/POOL, ETC.), CLADDING, FINISHES, ETC. PRIOR TO ERECTING STRUCTURE SUPPORTING THESE ITEMS. ALL WEIGHTS ACCOUNTED FOR IN THE STRUCTURAL DESIGN ARE LISTED ABOVE OR ON THE PLANS.

SITE CLASS D

Ss - 1.094 g

SEISMIC DESIGN CATEGORY A

S1

SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC.

ANY COMPONENT NOTED AS "DELEGATED" SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF FLORIDA AND NOT BY THE SER. SIGNED AND SEALED DRAWINGS SHALL BE PROVIDED TO THE ARCHITECT AND SEOR FOR REVIEW AS A SHOP DRAWING; CALCULATIONS WILL BE PROVIDED IF REQUESTED.

ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW NOTATION WILL BE RETURNED UNCHECKED. EVERY EFFORT WILL BE MADE TO RETURN THE SHOP DRAWINGS WITHIN TEN BUSINESS DAYS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO SUBMIT THE SHOP DRAWINGS ALLOWING FOR AN ADEQUATE REVIEW PERIOD.

ONE SET OF PRINTS WILL BE RETAINED BY THE ENGINEER AND ONE BY THE ARCHITECT, THE CONTRACTOR SHALL RECEIVE THE REMAINING PRINTS FOR SUBMITTAL TO THE BUILDING DEPARTMENT AND AS REQUIRED FOR DISTRIBUTION.

IN ALL INSTANCES THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN A REQUEST FOR INFORMATION (RFI) OR SIMILAR DOCUMENTATION BY THE ENGINEER. EVERY EFFORT WILL BE MADE TO RETURN THE RFIS WITHIN TWO BUSINESS DAYS. SUBMITTALS SHALL CLEARLY IDENTIFY THE SPECIFIC PROJECT, APPLICABLE CODES AND DESIGN CRITERIA. AND DETAILING OF ALL COMPONENTS NECESSARY TO ENSURE PROPER INSTALLATION OF THE COMPONENTS AND SYSTEM.

SHOP DRAWINGS SHOULD BE SUBMITTED FOR ALL COMPONENTS OF THE STRUCTURAL FRAMING SYSTEM, AS REQUIRED BY THE ARCHITECT, AND AS NOTED ELSEWHERE IN THESE NOTES, INCLUDING, BUT NOT LIMITED TO:

CONCRETE MIX DESIGNS CONCRETE REINFORCEMENT

PRE-ENGINEERED WOOD ROOF TRUSSES ANY PROPOSED MANUFACTURER CHANGE FROM THE BASIS OF DESIGN

IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE ALL TRADES AND CONSULTANTS, CROSS REFERENCE THEIR DRAWINGS WITH THE OVERALL DESIGN, AND PROVIDE TO EACH A COMPLETE SET OF DRAWINGS AND SUBMITTALS TO ENSURE COMPATIBILITY OF CONSTRUCTION PER DESIGN INTENT.

FOUNDATIONS:

ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2,000 PSF ON COMPACTED FILL OR COMPACTED NATIVE SOIL. BEFORE CONSTRUCTION COMMENCES AND IF REQUIRED BY THE JURISDICTION, SOIL BEARING CAPACITY SHALL BE VERIFIED BY A SUBSURFACE INVESTIGATION, AS WELL AS FIELD AND LABORATORY TESTS PERFORMED BY A CERTIFIED TESTING LABORATORY, WHOSE REPORT SHALL INCLUDE ANALYSIS AND RECOMMENDATIONS FOR SITE PREPARATION IN ORDER TO BEAR THE FOUNDATION LOADS. ABOVE REPORT SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW BEFORE FOUNDATION CONSTRUCTION BEGINS.

PENETRATIONS:

NO PENETRATIONS SHALL BE MADE IN ANY STRUCTURAL MEMBERS WITHOUT PREVIOUS APPROVAL OF THE EOR, EXCEPT THOSE PENETRATIONS SHOWN ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHOULD SUBMIT DRAWINGS SHOWING: ANY LOCATION WHERE OPENINGS, PENETRATIONS, OR ANY PLACE MORE THAN 3 PIPES OR CONDUITS ARE LOCATED IN A SLAB SYSTEM; ANY LOCATION IN A BEAM OR COLUMN WHERE MORE THAN 4% OF THE MEMBER SECTION IS REMOVED IN ANY

ORIENTATION. PLUMBING SLEEVES:

MINIMUM SLEEVE SPACING SHALL BE THREE DIAMETERS CENTER TO CENTER OF THE LARGER SLEEVE OR 6" CLEAR BETWEEN SLEEVES, WHICHEVER IS GREATER. PRIOR TO CONSTRUCTION SLEEVE LOCATIONS AND SIZES SHALL BE APPROVED BY THE ENGINEER. CONDUITS, PIPES AND SLEEVES SHALL BE PLACED AND SPACED IN ACCORDANCE WITH ACI 318 6.3.

REINFORCING STEEL

SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. SECURE APPROVAL OF SHOP DRAWINGS PRIOR TO COMMENCING FABRICATION.

REINFORCING BARS SHOWN IN SECTIONS DEPICT TYPICAL CONFIGURATION AND ARE NOT SPECIFIC TO THE CONCRETE MEMBER CUT ON PLAN; SEE PLAN AND SCHEDULES FOR ALL BAR SIZE AND QUANTITIES. TAKE-OFFS AND QUANTITIES SHALL BE OBTAINED FROM THE SCHEDULES AND PLANS, NOT FROM SECTIONS. WHERE HOOKS, LAP LENGTHS, ETC. ARE SHOWN IN SECTIONS, ALL LENGTHS AND DETAILS SHALL MEET ACI REQUIREMENTS FOR REINFORCING DETAILS. HOOKS SHALL BE ORIENTED AS REQUIRED TO FIT WITHIN THE CONCRETE MEMBER.

WELDED WIRE FABRIC:

TO CONFORM TO ASTM A-185, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. MINIMUM LAP SHALL BE ONE SPACE PLUS TWO INCHES.

WOOD FRAMING CONNECTORS:

ALL CONNECTORS SHALL BE GALVANIZED. CONNECTOR MODEL NUMBERS SHOWN ARE STRONG-TIE CONNECTORS AS MANUFACTURED BY SIMPSON STRONG-TIE CO., 5956 W. LAS POSITAS BLVD., P.O. BOX 10789, PLEASANTON, CA 94588, 800-999-5099. WWW.STRONGTIE.COM. SUBSTITUTIONS ARE ACCEPTABLE WITH THE APPROVAL OF THE SEOR. UNLESS SHOWN OTHERWISE, INSTALL THE *LARGEST* FASTENER SIZE AND *MAXIMUM* NUMBER OF FASTENERS SHOWN IN LATEST SIMPSON CATALOG. WHERE SDS SCREWS ARE SPECIFIED IN THE SIMPSON CATALOG, SDS SCREWS MUST BE USED UNLESS EXPRESSLY SHOWN IN THE DRAWINGS OTHERWISE. ALL ROOF AND UPPER-LEVEL UPLIFT CONNECTORS SHALL BE LOCATED ON THE SAME SIDE OF THE WALL

CONCRETE:

ALL CONCRETE SHALL MEET ACI 318 'BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE' AND ACI 301 'SPECIFICATIONS FOR STRUCTURAL CONCRETE', INCLUDING, BUT NOT LIMITED TO, MIX DESIGN, STRENGTH, COVER, PLACEMENT, CURING, TESTING, FORMS, FLATNESS, ETC. SEE THE ARCHITECTURAL SPECIFICATIONS AND PLANS FOR ANY ADDITIONAL REQUIREMENTS.

ALL CONCRETE SHALL BE AN APPROVED MIX DESIGN PROPORTIONED TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW

3000 PSI FOR FOUNDATIONS AND SLABS ON GRADE.

WITH A PLASTIC AND WORKABLE MIX:

SUBMIT PROPOSED MIX DESIGN FOR EACH PORTION OF WORK PRIOR TO USE. ALL MIX DESIGNS SHALL BE ACCOMPANIED BY A MINIMUM OF 15 FIELD STRENGTH TEST RECORDS, AS NOTED IN ACI 301 4.2.3.2(A); NO OTHER DOCUMENTATION OF AVERAGE COMPRESSIVE STRENGTH WILL BE ACCEPTED WITHOUT PRIOR WRITTEN APPROVAL BY THE SER. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. MIX SHALL MEET THE REQUIREMENTS OF ASTM C33 FOR COARSE AGGREGATE. FOR ALL FLATWORK, AT LEAST 75% OF LARGE AGGREGATE SHALL CONSIST OF #57 STONE. CONCRETE SHALL COMPLY WITH ALL THE REQUIREMENTS OF ASTM STANDARD C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED.

ALL CONCRETE MIX DESIGNS SHALL INCLUDE A WRITTEN DESCRIPTION INDICATING WHERE EACH PARTICULAR MIX IS TO BE PLACED WITHIN THE STRUCTURE.

CONCRETE SHALL BE PLACED AND CURED ACCORDING TO ALL STANDARDS AND SPECIFICATIONS, INCLUDING ALL ACI

THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1-1/2) HOURS. IF FOR ANY REASON THERE IS A LONGER DELAY THAN THAT STATED ABOVE, THE CONCRETE SHALL BE DISCARDED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LAB TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE. ALL SLABS SHALL BE CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1-D AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE ALL SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY. CALCIUM CHLORIDES SHALL NOT BE UTILIZED; OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER.

THE GENERAL CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD A MINIMUM OF 48 HOURS PRIOR PLACEMENT OF ANY STRUCTURAL CONCRETE.

UNLESS NOTED OTHERWISE ON PLANS, THE FOLLOWING CONCRETE CLEAR COVER SHALL BE PROVIDED FOR ALL NON-PRESTRESSED CONCRETE REINFORCEMENT PER ACI 318:

CONCRETE CAST AGAINST EARTH:	ALL BARS	-	3"
CONCRETE EXPOSED TO EARTH (FORMED FACE):	ALL BARS	-	2"
CONCRETE EXPOSED TO WEATHER:	#6 BARS AND GREATER	-	2"
	#5 BARS AND SMALLER	-	1 1/2"
WHERE NOT EXPOSED TO EARTH OR WEATHER:			
SLABS, WALLS, AND JOISTS:	#14 & #18 BARS	-	1 1/2"
	#11 BARS AND SMALLER	-	3/4"
BEAMS AND COLUMNS:	ALL BARS	-	1 1/2"

A) ASTM C143 - "STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE." MAXIMUM SLUMP SHALL BE 4-6 INCHES, PRIOR TO ADDING A SUPER PLASTICIZER.

B) ASTM C39 - "STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS." A SEPARATE TEST SHALL BE CONDUCTED FOR EACH CLASS, FOR EVERY 50 CUBIC YARDS (OR FRACTION THEREOF), PLACED PER DAY. REQUIRED CYLINDER(S) QUANTITIES AND TEST AGE AS FOLLOWS: 2 AT 28 DAYS

ONE ADDITIONAL RESERVE CYLINDER TO BE TESTED UNDER THE DIRECTION OF THE ENGINEER, IF REQUIRED. IF 28 DAY STRENGTH IS ACHIEVED, THE ADDITIONAL CYLINDER(S) MAY BE DISCARDED.

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POUR STRUCTURAL CONCRETE WITHIN THE FOLLOWING TOLERANCES:
       VARIATION FROM PLUMB:
                                                      1/4" IN 10'-0'
       VARIATION FROM LEVEL IN TOPS OF PILASTERS: 1/8" IN 10'-0"
       VARIATION FOOTINGS:
       PLAN DIMENSIONS:
                                                      +2", - 1/2"
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CHEMICAL ANCHORS:

SHALL BE AN EQUAL TWO-PART EPOXY POLYMER INJECTION SYSTEM, SUCH AS SIMPSON SET-XP "STRUCTURAL ANCHORING ADHESIVE", HILTI HIT-HY 200 OR ENGINEER APPROVED SUBSTITUTION, INSTALLED IN ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS. INSTALLERS SHALL BE TRAINED BY THE MANUFACTURER'S REPRESENTATIVE. BRUSH AND BLOW OUT ALL HOLES.

STRUCTURAL WOOD COMPONENTS (BEAMS, JOISTS, RAFTERS, ETC.) SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE FIBER STRESSES FOR NO. 2 SOUTHERN PINE CONFORMING TO NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION WITH 2015 NDS SUPPLEMENT, AS FOLLOWS:

SHEAR	FV	= 175 PSI.
BENDING 2X6	FB	= 1,000 PSI.
BENDING 2X8	FB	= 925 PSI.
BENDING 2X10	FB	= 800 PSI.
BENDING 2X12	FB	= 750 PSI.

WOOD IN CONTACT WITH CONCRETE OR MASONRY, AND AT OTHER LOCATIONS AS SHOWN ON STRUCTURAL DRAWINGS, SHALL BE PROTECTED OR PRESSURE TREATED IN ACCORDANCE WITH AITC-109. MEMBER SIZES SHOWN ARE NOMINAL UNLESS NOTED

ALL NAILS SHOWN ON PLANS ASSUME COMMON WIRE NAILS UNLESS SPECIFICALLY NOTED ON DRAWINGS. BOLTS FOR WOOD CONSTRUCTION AT INTERIOR LOCATIONS SHALL CONFORM TO ASTM A307. THREADED RODS AT INTERIOR LOCATIONS SHALL CONFORM TO ASTM A36. EXTERIOR BOLTS AND THREADED RODS PROTECTED FROM MOISTURE AND WEATHER SHALL BE HOT-DIP GALVANIZED. EXPOSED EXTERIOR BOLTS AND THREADED RODS SHALL BE AISI 316 STAINLESS STEEL. ALL NAILS LARGER THAN 10D AND SCREWS LARGER THAN 7 GAUGE SHALL BE PREDRILLED AS NEEDED TO PREVENT SPLITTING OF THE WOOD. BOLT HOLES IN WOOD MEMBERS SHALL BE A MINIMUM OF 1/32" LARGER THAN THE BOLT DIAMETER, BUT NO GREATER THAN 1/16" LARGER. A METAL PLATE OR WASHER NO SMALLER THAN A STANDARD CUT WASHER SHALL BE LOCATED BETWEEN THE BOLT HEAD AND THE WOOD AND BETWEEN THE NUT AND THE WOOD OF ALL BOLTS.

ENGINEERED WOOD TRUSS SYSTEMS SHALL BE DESIGNED BY SUPPLIER'S SPECIALTY ENGINEER TO CONFIGURATION AND LOAD-CARRYING CAPACITY SHOWN ON DRAWINGS AND SPECIFICATIONS. ALL INDIVIDUAL TRUSS MEMBERS, TRUSS PLATE CONNECTIONS, TRUSS-TO-TRUSS CONNECTIONS, COMMON TRUSSES, AND GIRDER TRUSSES SHALL BE DESIGNED FOR COMPONENT AND CLADDING WIND LOADING, EXCEPT THOSE TRUSSES EXCEEDING 700 SQUARE FEET IN TRIBUTARY AREA. ALTERNATE TRUSS LAYOUTS ARE ACCEPTABLE ONLY AS A CHANGE ORDER WHICH WILL INCLUDE ENGINEERING CHARGES FOR REDESIGN OF THE STRUCTURE BY THE SEOR. SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS SHALL SHOW AND SPECIFY ALL CONNECTOR TYPES UTILIZED WITHIN TRUSSES, AS WELL AS CONNECTORS UTILIZED IN ALL OTHER CONNECTIONS AND ATTACHMENTS BETWEEN TRUSSES OR COMPONENTS SUPPLIED AS PART OF THE ENGINEERED TRUSS SYSTEM. AN ERECTION DRAWING SHALL BE INCLUDED, IDENTIFYING ALL TRUSS SYSTEM COMPONENTS, AS WELL AS ALL PERMANENT BRACING REQUIRED FOR TRUSS DESIGN.

TRUSSES AT ALL BEARING LOCATIONS SHALL BE STABILIZED. THE GENERAL CONTRACTOR SHALL FOLLOW ALL REQUIREMENTS BY THE DELEGATED TRUSS ENGINEERING PACKAGE. AT A MINIMUM: FOR TRUSS HEEL DEPTHS LESS THAN 8", ONLY BLOCKING BY THE TRUSS ENGINEERING IS REQUIRED; FOR TRUSS HEEL DEPTHS BETWEEN 8" AND 12", USE FULL HEIGHT SAWN-LUMBER BLOCKING; FOR TRUSS HEEL DEPTHS GREATER THAN 12", USE TRUSS BLOCKING OR A SHEATHED KNEEWALL. SEE THE STRUCTURAL DRAWINGS FOR ANY ADDITIONAL BLOCK REQUIREMENTS FOR THE LATERAL-FORCE RESISTING SYSTEM.

ENGINEERED SHOP DRAWINGS SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A REGISTERED PROFESSIONAL ENGINEER, LICENSED IN THE STATE WHERE THE STRUCTURE WILL BE ERECTED, AS THE DELEGATED (SPECIALTY) ENGINEER. PLUMBING, ELECTRICAL, AND MECHANICAL DRAWINGS SHALL BE COORDINATED WITH THE TRUSS LAYOUT TO ENSURE THAT THERE ARE NO CONFLICTS WITH DUCTS, RECESSED FIXTURES, PLUMBING PIPES, TRAPS, HOODS, CEILING STEPS/SLOPES, ETC. TRUSS LAYOUT SHALL BE MODIFIED AND/OR TRUSS CHASES SHALL BE ADDED TO AVOID CONFLICTS. TRUSS SPACING SHALL NOT EXCEED MAXIMUM NOTED IN PLAN NOTES, U.N.O.

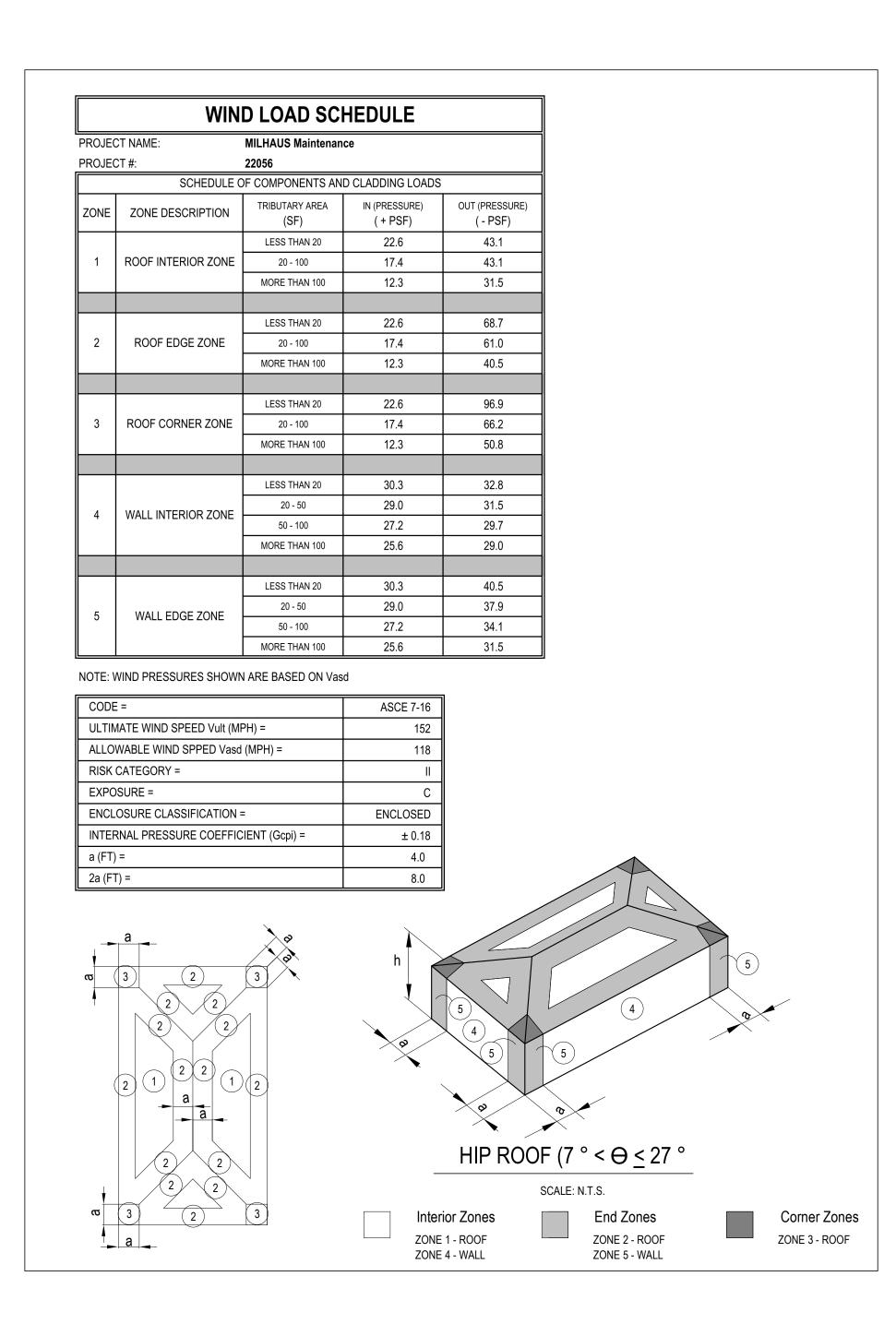
THE FOLLOWING LOAD DURATION FACTORS SHALL BE USED:

FULLOWING LUAD DURATION FACTORS SH	ALL BE USED:
DEAD LOAD	0.90
DEAD LOAD + FLOOR LIVE LOAD	1.00
DEAD LOAD + ROOF LIVE LOAD	1.25
DEAD LOAD + WIND LOAD	1.60

ALL TRUSSES SHALL BE DESIGNED FOR A MAXIMUM DEFLECTION OF L/360 FOR LIVE LOAD AND L/240, NOT TO EXCEED 1", FOR TOTAL LOAD; THE MAXIMUM DEFLECTION DUE TO TOTAL LOAD OF 1" IS INCLUSIVE OF ALL DEAD LOAD, SELF-WEIGHT, SUPERIMPOSED DEAD LOAD, AND LIVE LOAD, INCLUDING CREEP.

PLYWOOD ROOF, FLOOR, AND WALL SHEATHING ARE DESIGNED AS DIAPHRAGMS/SHEAR WALLS AND SHALL COMPLY WITH APPLICABLE PROVISIONS OF CHAPTER 23 OF THE BUILDING CODE AND SHALL BE FASTENED IN ACCORDANCE WITH THE MINIMUM. REQUIREMENTS OF BUILDING CODE TABLES, UNLESS SHOWN OTHERWISE. PLYWOOD SHALL BE INSTALLED WITH THE STRENGTH AXIS OF EACH PANEL PERPENDICULAR TO THE SUPPORTS IN ALL CASES. PLYWOOD ROOF PANELS SHALL BE INSTALLED AS SHOWN IN CASES 1 THROUGH 4 IN TABLE 2306.2.1 (CONT.). BLOCKING SHALL BE PROVIDED BETWEEN ALL WOOD ROOF FRAMING MEMBERS AT ALL RIDGES AND VALLEYS FOR FULL PLYWOOD EDGE SUPPORT. AT ROOF VENT LOCATIONS, PROVIDE 2X4 BLOCKING, ON THE FLAT, ON ALTERNATING SIDES OF THE VENT BETWEEN ROOF FRAMING MEMBERS.

ALL WOOD SHEAR WALLS SHALL HAVE ALL PLYWOOD EDGES FULLY BLOCKED WITH THE SAME STUD SIZE AS THE WALLS, WITH THE BLOCKING INSTALLED SO THAT THE PLYWOOD IS NAILED INTO THE NARROW STUD FACE. FASTEN THE SHEATHING WITH THE SAME NAIL SIZE AND SPACING TO EACH PLY OF DOUBLE TOP AND BOTTOM WALL PLATES. AS APPLICABLE. FASTEN THE SHEATHING WITH THE SAME NAIL SIZE AND SPACING TO MULTI-PLY COLUMNS AT ENDS OF WALLS; WHERE SOLID COLUMNS ARE USED AT ENDS OF WALLS, FASTEN THE SHEATHING WITH THE SAME NAIL SIZE AND SPACING IN VERTICAL ROWS WITH 1-1/2" ROW SPACING FOR FULL HEIGHT OF COLUMN. WHERE SHEAR WALL PANEL EDGE NAILING IS 3" OR LESS, THE BLOCKING AT PANEL EDGES SHALL BE 3" NOMINAL OR GREATER AND THE NAILING SHALL BE STAGGERED. MULTIPLE PLY STUDS MAY BE USED AS THE PANEL EDGE BLOCKING IN LIEU OF 3" NOMINAL BLOCKING; FASTEN THE PLIES WITH NAILS HAVING THE SAME LENGTH AS THE TOTAL BLOCKING THICKNESS WITH SPACING TO MATCH THE PLYWOOD PANEL EDGE NAILING, STAGGERED. PANELS SHALL NOT BE LESS THAN 4FTX8FT, EXCEPT AT BOUNDARIES AND CHANGES IN FRAMING.



- ADD CRIPPLE STUD ABOVE

QUANTITY) FOR MULTI-STORY

DOUBLE TOP PLATE WITH

CRIPPLE STUDS TO MATCH

AND ATTACHMENT TO TOP

WALL STUD SIZE, SPACING

SEE PLAN NOTES.

EACH WAY.

PLYWOOD SHEATHING, IF APPLICABLE, IS NOT SHOWN FOR CLARITY.

WHERE MULTIPLE JAMB STUDS ARE SPECIFIED EACH JAMB STUD IS

WHERE BUILT-UP STUD COLUMNS ARE NOTED ON PLAN, BUILT-UP

A MINIMUM OF (1)JACK AND (1)KING STUD ARE REQUIRED AT ALL

OPENINGS, UNLESS NOTED OTHERWISE ON PLAN.

TO BE STRAPPED ACCORDING TO PLAN NOTES.

COLUMN INCLUDES JACK STUDS.

TYPICAL WOOD WALL OPENING DETAIL

LSTA12 AT EACH STUD,

U.N.O. ON PLAN.

STAGGERED SPLICE LOCATIONS.

HEADER BEAM. SEE PLAN AND PLAN NOTES. FILL OUT

SCREWS AT 6" O.C. AT PERIMETER AND IN THE FIELD,

WITH PLYWOOD TO MATCH WALL WIDTH. IF ATTACHING

TO THE OUTSIDE FACE OF THE BEAM, USE #8X2" WOOD

JACK STUDS (MATCH

BUILDINGS.

SPH4/6 AT EACH KING STUD WRAPPED OVER TOP

10D TOE NAILS AT 3" O.C. EACH

HEADER BEAM STRAPS

STRAP OR HOLDDOWN -

ROD INTO FOOTING.

FLOOR FRAMING OR

FOOTING BELOW.

CONNECTED TO FRAMING

BELOW, OR HOLDDOWN WITH EMBEDDED THREADED

(LSTA15 MIN.). SEE PLAN.

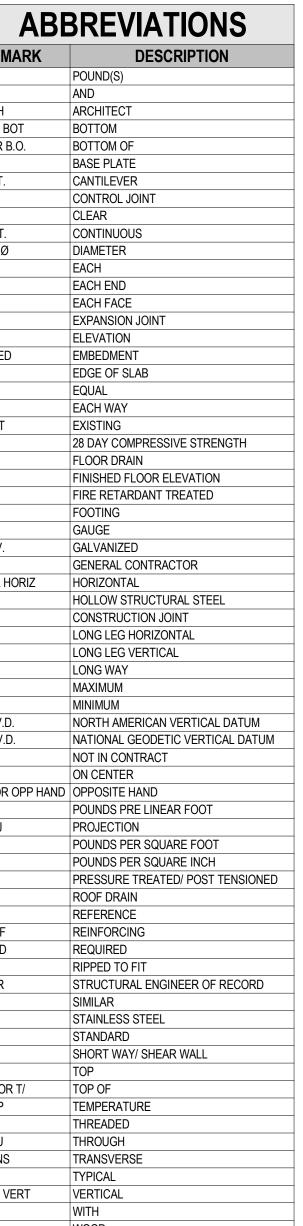
SIDE OF HEADER TO KING STUDS.

JACK STUD(S). SEE JACK STUD SCHEDULE.

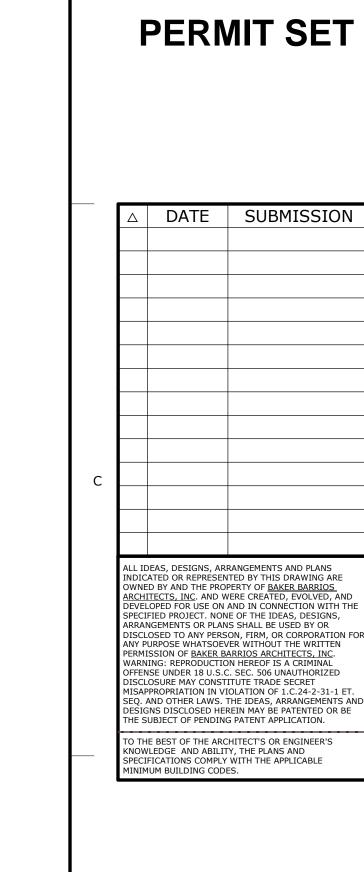
PLATE.

KING STUD(S).









ORLANDO

407 926 3000

This item has been digitally signed and

sealed by Cordell S. Van Nostrand on 01/05/22 using a Digital Signature.

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INFO@BAKERBARRIOS.COM

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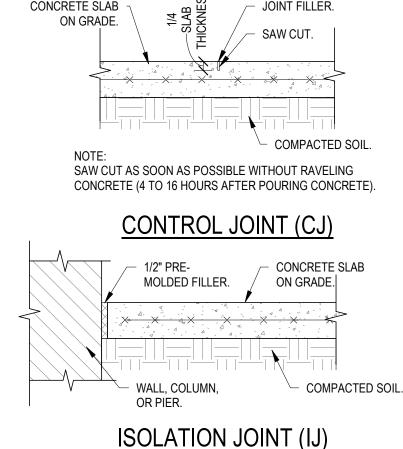
NGINEER OF RECORD

Cordell S. Van Nostrand

FL P.E. # 67580

ORLANDO, FLORIDA 32801

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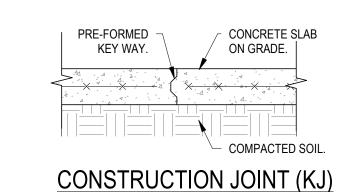
MAINTENANCE - STRUCTURAL

NOTES, TYPICAL DETAILS

FLOOR & ROOF FRAMING

PLANS, SECTIONS

MAINTENANCE - GROUND





ISR-82

7780 LIGHTARD KNOTT LN FORT MYERS, FL 33905

MAINTENANCE -STRUCTURAL **NOTES, TYPICAL**

DETAILS

220035.00

3 AS-2





Baker Barrios ORLANDO

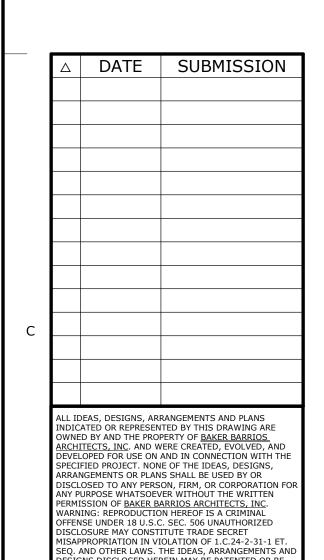
> 189 S. ORANGE AVE., SUITE 1700 ORLANDO, FLORIDA 32801 407 926 3000 INFO@BAKERBARRIOS.COM **BAKERBARRIOS.COM**

01/05/22 ENGINEER OF RECORD

PERMIT SET

Cordell S. Van Nostrand

FL P.E. # 67580



HE SUBJECT OF PENDING PATENT APPLICATION.

TO THE BEST OF THE ARCHITECT'S OR ENGINEER'S KNOWLEDGE AND ABILITY, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES.



MILHAUS

SR-82

TYPICALLY INDICATES 1/8" SAWCUT

REMARKS

ANCHOR BOLTS EPOXY-SET 8" INTO FOOTING BELOW.

WOOD COLUMN SCHEDULE

HTT4

WC1

(3) 2x6

CONTROL JOINT. SEE PLAN NOTES.

7780 LIGHTARD KNOTT LN FORT MYERS, FL 33905 220035.00

MAINTENANCE -**GROUND FLOOR &** ROOF FRAMING PLANS, SECTIONS

AS-2

ROOF FRAMING PLAN NOTES:

/ APA RATED EXP. 1

PRE-ENGINEERED WOOD

— 6" WOOD STUD WALL. SEE

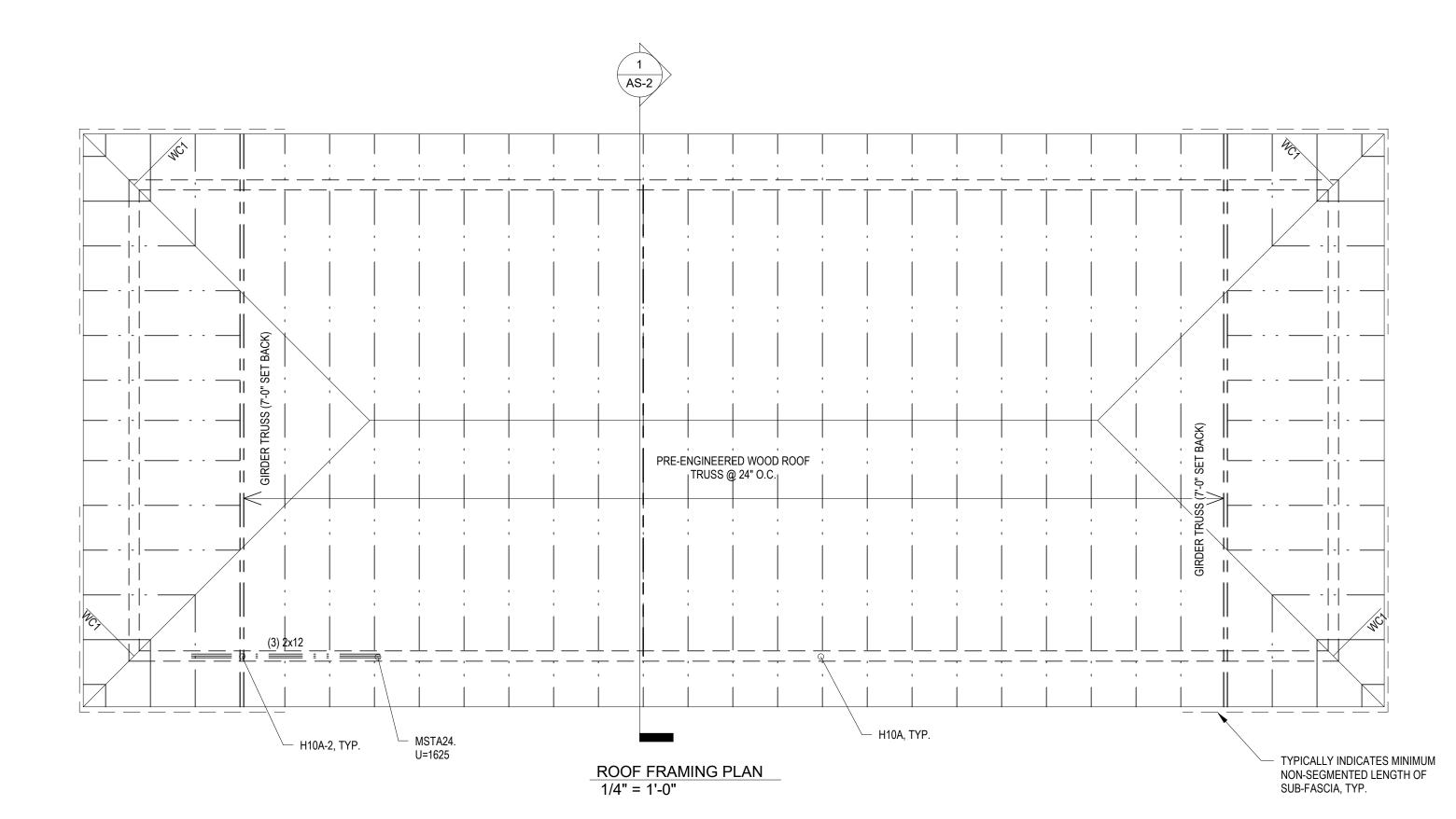
CONCRETE FOUNDATION.

SEE PLAN.

ROOF TRUSS BY DELEGATED ENGINEER.

PLYWOOD ROOF SHEATHING.

- 1. THE ROOF FRAMING SYSTEM SHALL CONSIST OF WOOD ROOF TRUSSES BY DELEGATED ENGINEER AT 24" O.C. VERIFY CEILING HEIGHTS AND CONFIGURATIONS WITH THE ARCHITECTURAL PLANS. SEE ARCHITECTURAL PLANS FOR ROOF SLOPE AND EAVE DETAILS. TRUSS BEARING ELEVATION = 9'-0" UNLESS NOTED OTHERWISE ON PLAN.
- - INDICATES WOOD TRUSS SPAN DIRECTION AND EXTENTS. VERIFY ALL GIRDER LOCATIONS AND PLIES WITH TRUSS ENGINEERING AND NOTIFY EOR OF ANY DISCREPANCIES.
- ROOF SHEATHING SHALL BE 19/32" APA RATED EXTERIOR PLYWOOD ROOF SHEATHING. ATTACHMENT FOR ZONES 1 & 2, AS DEPICTED IN THE COMPONENT AND CLADDING WIND SCHEDULE, SHALL BE MADE W/ 10D NAILS @ 6" O.C. AT SUPPORTED PANEL EDGES AND 12" O.C. IN THE FIELD. ATTACHMENT FOR ZONE 3 IS TO BE MADE W/ 10D NAILS @ 4" O.C. AT SUPPORTED PANEL EDGES AND IN THE FIELD. PROVIDE PLYWOOD "H" CLIPS @ UNSUPPORTED PLYWOOD PANEL EDGES.
- 4. ALL WOOD TRUSSES SHALL BE ATTACHED TO THE STRUCTURE WITH THE SPECIFIED SIMPSON CONNECTOR.
- THE TRUSS ENGINEER SHALL SPECIFY THE TRUSS-TO-TRUSS CONNECTIONS.
- PLYWOOD SHEATHING SHALL BE CONTINUOUS ON THE TOP CHORD OF THE TRUSSES. ALL OVER-FRAMING OR VALLEY SETS SHALL BE PLACED ABOVE THE CONTINUOUS PLYWOOD.
- 7. ALL WOOD IN CONTACT WITH MASONRY, CONCRETE OR USED IN EXTERIOR APPLICATIONS SHALL BE PRESSURE TREATED WITH PRESERVATIVE. (P.T.)
- EXTERIOR CONNECTORS SHALL BE HOT DIPPED GALVANIZED. PROVIDE 2X CONT. FASCIA AT EDGE OF ROOF. PROVIDE MSTA24 ACROSS ALL
- SPLICE LOCATIONS.
- INDICATES TRUSS UPLIFT (IN lbs), IN EXCESS OF 1000 lbs. INDICATES TRUSS GRAVITY LOAD (IN lbs), IN EXCESS OF 5000 lbs.
- ATTACH ALL TRUSSES TO WOOD WALLS AND HEADER BEAMS WITH H10A, U.N.O. ON PLAN.

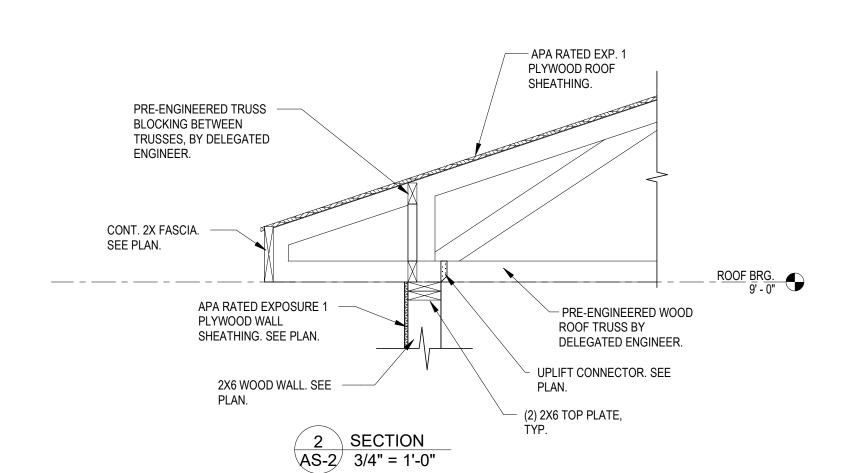


AS-2

4" CONCRETE SLAB ON GRADE.

TE24

FOUNDATION AND GROUND FLOOR PLAN 1/4" = 1'-0"



─ 4" CONCRETE SLAB ON

GRADE. SEE PLAN.

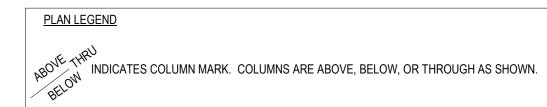
FOUNDATION PLAN NOTES:

- VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL AND MEP FOR ADDITIONAL INFORMATION.
- ELEVATIONS ARE CALLED OUT RELATIVE TO THE MAIN INTERIOR GROUND FLOOR SLAB, SET AT 0'-0" (REF.).
- SEE ARCHITECTURAL DRAWINGS FOR ALL SLOPES, DROPS AND DRAIN LOCATIONS IN FLOOR SLAB. MAINTAIN 4" MINIMUM SLAB DEPTH. THICKEN SLAB TO 8" WITHIN 4" OF ALL SLAB STEPS; MAINTAIN 4" MINIMUM SLAB DEPTH ELSEWHERE.
- GROUND FLOOR SHALL BE 4" CONCRETE SLAB-ON-GRADE, U.N.O. REINFORCE W/ 6X6 W1.4XW1.4 W.W.F. AT MID-DEPTH.
- PROVIDE CONTROL JOINTS IN ALL SLABS ON GRADE. CONTROL JOINTS SHALL BE TOOLED OR SAWCUT AS SOON AS POSSIBLE WITHOUT RAVELING. THE PATTERN SHALL BE APPROXIMATELY SQUARE AND LIMITED TO AN AREA OF 144 SF. SEE PLAN FOR LOCATIONS.

WALL LEGEND

INDICATES A 2X6 SO. PINE #2 EXTERIOR SHEAR WALL WITH STUDS AT 16" O.C. MAX. PROVIDE (1) JACK STUD AND (2) KING STUDS ON EACH SIDE OF EACH OPENING. EACH STUD IN WALL TO HAVE AN SP1 TO SINGLE BOTTOM PLATE AND SP2 TO DOUBLE-TOP PLATE. PROVIDE HORIZONTAL BLOCKING AT 4'-0" O.C. MAX. ATTACH BOTTOM PLATE W/ 1/2" J BOLTS EMBEDDED 8" INTO CONCRETE BELOW @ 18" O.C. MAX. PROVIDE (3) 2X10 HEADERS OVER OPENINGS UNLESS NOTED OTHERWISE. PROVIDE FULL BLOCKING AT ALL PLYWOOD JOINTS. PROVIDE 15/32" APA RATED EXPOSURE 1 PLYWOOD SHEATHING ON BOTH SIDES OF WALLWITH 10D NAILS AT 4" O.C. AT ALL EDGES AND AT 12" O.C. IN THE FIELD. INSTALL EXPOSED PLYWOOD GRAIN PERPENDICULAR TO STUDS. SEE TYPICAL DETAIL ON S-1 FOR MORE INFORMATION.

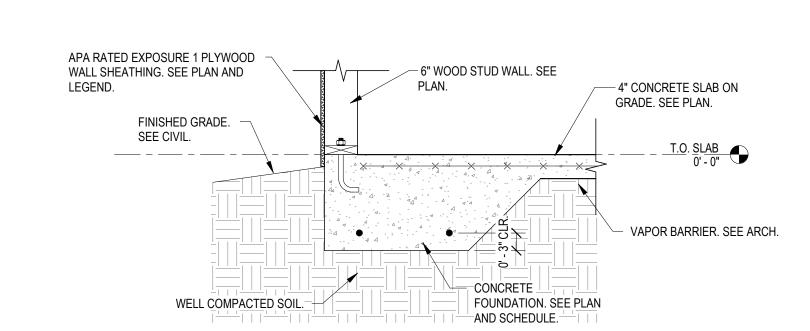
PLAN LEGEND INDICATES COLUMN MARK. COLUMNS ARE ABOVE, BELOW, OR THROUGH AS SHOWN.



FOUNDATION SCHEDULE REMARKS T.O. FTG. = T.O. SLAB. ENSURE CONT. BOTTOM OF FOOTING IS 12" MIN. BELOW ADJACENT GRADE.

FOUNDATION SCHEDULE NOTES:

PROVIDE CORNER BARS AT ALL FOOTING INTERSECTIONS AND CORNERS. PROVIDE 3" CLEAR COVER FOR ALL REINFORCING UNLESS DETAILED OTHERWISE. BOTTOM OF ALL FOUNDATIONS SHALL BE BELOW THE FROST LINE BASED ON FINISHED GRADE. GC SHALL COORDINATE WITH CIVIL FOR FINISHED GRADES. IN NO CASE SHALL FOUNDATOINS BE LESS THAN 12" BELOW ADJACENT GRADE, EXCEPT FOR NON-LOADING BEARING "TE" FOOTINGS.



3 SECTION AS-2 3/4" = 1'-0"

GENERAL NOTES:

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

ALL DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK. DO NOT SCALE DRAWINGS.

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS.

ELECTRONIC VERSIONS OF THE STRUCTURAL DRAWINGS ARE THE SOLE, COPYRIGHTED PROPERTY OF SNELL ENGINEERING AND ARE NOT TO BE USED OR TRANSFERRED WITHOUT THE EXPRESS, WRITTEN PERMISSION OF SNELL ENGINEERING.

DESIGN LOADS:

THE STRUCTURAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE -

	ITION (2020). THE FOLLOWING SUPERIMPOSED I		
WIND:			
	ASCE 7-16		
	ULTIMATE WIND SPEED	-	152 MPH
	ALLOWABLE WIND SPEED	-	118 MPH
	EXPOSURE C OPEN STRUCTURE		
	INTERNAL PRESSURE COEFFICIENT	_	+/- 0.18
	RISK FACTOR II	-	+/- 0.10
	SEE WIND SCHEDULE FOR PRESSURES		
ROOF:	LINELOAD		00 POF
	LIVE LOAD (CONCENTRATER)	-	20 PSF.
	LIVE LOAD (CONCENTRATED)	-	300 LBS.
	DEAD LOAD (A)(A)(A)(A)(A)(A)(A)(A)(A)(A)(A)(A)(A)(-	25 PSF.
	DEAD LOAD (AVAILABLE TO RESIST UPLIFT)	-	5 PSF.
FLOOD:			
	FEMA A/V ZONE		
	FLOOD DESIGN CLASS	-	II
	BFE ELEVATION	-	11.00 NAVD
	FREEBOARD	-	1FT
	PROPOSED LOWEST FLOOR ELEVATION		12.00 NAVD
	BOT. OF LOWEST HORIZ. STRUCT. MEMBER		12.00 NAVD
	REQ'D MIN. DRY FLOOD-PROOF ELEVATION		12.00 NAVD
	STRUCTURAL SYSTEMS OF BUILDINGS AND ST RESIST FLOTATION, COLLAPSE OR PERMANEN		
			EVATION IN CONFORMANCE WITH ASCE 24-14 AND
	FBC-R 322.1.2/FBC 1612.1.	OIGIVI LOOD LL	EVATION IN COM CHURNOL WITH ACCE 24-14 AND
SEISMIC	·		
SEISIVIIC	,. RISK CATEGORY	_	II
	SEISMIC IMPORTANCE FACTOR le	-	1.0
	SITE CLASS D		

IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO CONFIRM ALL FINAL WEIGHTS OF EQUIPMENT (MECHANICAL, CHILLED WATER, KITCHEN, SPA/POOL, ETC.), CLADDING, FINISHES, ETC. PRIOR TO ERECTING STRUCTURE SUPPORTING THESE ITEMS. ALL WEIGHTS ACCOUNTED FOR IN THE STRUCTURAL DESIGN ARE LISTED ABOVE OR ON THE PLANS.

SHOP DRAWING REVIEW:

SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO

ANY COMPONENT NOTED AS "DELEGATED" SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF FLORIDA AND NOT BY THE SER. SIGNED AND SEALED DRAWINGS SHALL BE PROVIDED TO THE ARCHITECT AND SEOR FOR REVIEW

AS A SHOP DRAWING; CALCULATIONS WILL BE PROVIDED IF REQUESTED. ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW NOTATION WILL BE RETURNED UNCHECKED. EVERY EFFORT WILL BE MADE TO RETURN THE SHOP

DRAWINGS WITHIN TEN BUSINESS DAYS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO SUBMIT THE SHOP DRAWINGS ALLOWING

ONE SET OF PRINTS WILL BE RETAINED BY THE ENGINEER AND ONE BY THE ARCHITECT, THE CONTRACTOR SHALL RECEIVE THE REMAINING PRINTS FOR SUBMITTAL TO THE BUILDING DEPARTMENT AND AS REQUIRED FOR DISTRIBUTION.

IN ALL INSTANCES THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN A REQUEST FOR INFORMATION (RFI) OR SIMILAR DOCUMENTATION BY THE ENGINEER. EVERY EFFORT WILL BE MADE TO RETURN THE RFIS WITHIN TWO BUSINESS DAYS. SUBMITTALS SHALL CLEARLY IDENTIFY THE SPECIFIC PROJECT, APPLICABLE CODES AND DESIGN CRITERIA, AND DETAILING OF ALL COMPONENTS NECESSARY TO ENSURE PROPER INSTALLATION OF THE COMPONENTS

SHOP DRAWINGS SHOULD BE SUBMITTED FOR ALL COMPONENTS OF THE STRUCTURAL FRAMING SYSTEM, AS REQUIRED BY THE ARCHITECT, AND AS NOTED ELSEWHERE IN THESE NOTES, INCLUDING, BUT NOT LIMITED TO: CONCRETE MIX DESIGNS CONCRETE REINFORCEMENT

PRE-ENGINEERED WOOD ROOF TRUSSES ANY PROPOSED MANUFACTURER CHANGE FROM THE BASIS OF DESIGN

SEISMIC DESIGN CATEGORY A

S1

Sd1 - 0.395 g

Ss - 1.094 g

Sds - 0.775 g

QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC.

FOR AN ADEQUATE REVIEW PERIOD.

IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE ALL TRADES AND CONSULTANTS, CROSS REFERENCE THEIR DRAWINGS WITH THE OVERALL DESIGN, AND PROVIDE TO EACH A COMPLETE SET OF DRAWINGS AND SUBMITTALS TO ENSURE COMPATIBILITY OF CONSTRUCTION PER DESIGN INTENT.

FOUNDATIONS:

ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2,000 PSF ON COMPACTED FILL OR COMPACTED NATIVE SOIL. BEFORE CONSTRUCTION COMMENCES AND IF REQUIRED BY THE JURISDICTION, SOIL BEARING CAPACITY SHALL BE VERIFIED BY A SUBSURFACE INVESTIGATION, AS WELL AS FIELD AND LABORATORY TESTS PERFORMED BY A CERTIFIED TESTING LABORATORY, WHOSE REPORT SHALL INCLUDE ANALYSIS AND RECOMMENDATIONS FOR SITE PREPARATION IN ORDER TO BEAR THE FOUNDATION LOADS. ABOVE REPORT SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW BEFORE FOUNDATION CONSTRUCTION BEGINS.

PENETRATIONS:

NO PENETRATIONS SHALL BE MADE IN ANY STRUCTURAL MEMBERS WITHOUT PREVIOUS APPROVAL OF THE EOR, EXCEPT THOSE PENETRATIONS SHOWN ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHOULD SUBMIT DRAWINGS SHOWING: ANY LOCATION WHERE OPENINGS, PENETRATIONS, OR ANY PLACE MORE THAN 3 PIPES OR CONDUITS ARE LOCATED IN A SLAB SYSTEM; ANY LOCATION IN A BEAM OR COLUMN WHERE MORE THAN 4% OF THE MEMBER SECTION IS REMOVED IN ANY

MINIMUM SLEEVE SPACING SHALL BE THREE DIAMETERS CENTER TO CENTER OF THE LARGER SLEEVE OR 6" CLEAR BETWEEN SLEEVES, WHICHEVER IS GREATER. PRIOR TO CONSTRUCTION SLEEVE LOCATIONS AND SIZES SHALL BE APPROVED BY THE ENGINEER. CONDUITS, PIPES AND SLEEVES SHALL BE PLACED AND SPACED IN ACCORDANCE WITH ACI 318 6.3.

SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. SECURE APPROVAL OF SHOP DRAWINGS PRIOR TO COMMENCING FABRICATION.

REINFORCING BARS SHOWN IN SECTIONS DEPICT TYPICAL CONFIGURATION AND ARE NOT SPECIFIC TO THE CONCRETE MEMBER CUT ON PLAN; SEE PLAN AND SCHEDULES FOR ALL BAR SIZE AND QUANTITIES. TAKE-OFFS AND QUANTITIES SHALL BE OBTAINED FROM THE SCHEDULES AND PLANS, NOT FROM SECTIONS. WHERE HOOKS, LAP LENGTHS, ETC. ARE SHOWN IN SECTIONS, ALL LENGTHS AND DETAILS SHALL MEET ACI REQUIREMENTS FOR REINFORCING DETAILS. HOOKS SHALL BE ORIENTED AS REQUIRED TO FIT WITHIN THE CONCRETE MEMBER.

WELDED WIRE FABRIC:

TO CONFORM TO ASTM A-185, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. MINIMUM LAP SHALL BE ONE SPACE PLUS TWO INCHES.

ALL CONCRETE SHALL MEET ACI 318 'BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE' AND ACI 301 'SPECIFICATIONS FOR STRUCTURAL CONCRETE', INCLUDING, BUT NOT LIMITED TO, MIX DESIGN, STRENGTH, COVER, PLACEMENT, CURING, TESTING, FORMS, FLATNESS, ETC. SEE THE ARCHITECTURAL SPECIFICATIONS AND PLANS FOR ANY ADDITIONAL REQUIREMENTS.

ALL CONCRETE SHALL BE AN APPROVED MIX DESIGN PROPORTIONED TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW WITH A PLASTIC AND WORKABLE MIX:

3000 PSI FOR FOUNDATIONS AND SLABS ON GRADE.

SUBMIT PROPOSED MIX DESIGN FOR EACH PORTION OF WORK PRIOR TO USE. ALL MIX DESIGNS SHALL BE ACCOMPANIED BY A MINIMUM OF 15 FIELD STRENGTH TEST RECORDS. AS NOTED IN ACI 301 4.2.3.2(A): NO OTHER DOCUMENTATION OF AVERAGE COMPRESSIVE STRENGTH WILL BE ACCEPTED WITHOUT PRIOR WRITTEN APPROVAL BY THE SER. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. MIX SHALL MEET THE REQUIREMENTS OF ASTM C33 FOR COARSE AGGREGATE. FOR ALL FLATWORK, AT LEAST 75% OF LARGE AGGREGATE SHALL CONSIST OF #57 STONE. CONCRETE SHALL COMPLY WITH ALL THE REQUIREMENTS OF ASTM STANDARD C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED.

ALL CONCRETE MIX DESIGNS SHALL INCLUDE A WRITTEN DESCRIPTION INDICATING WHERE EACH PARTICULAR MIX IS TO BE PLACED WITHIN THE STRUCTURE.

CONCRETE SHALL BE PLACED AND CURED ACCORDING TO ALL STANDARDS AND SPECIFICATIONS, INCLUDING ALL ACI REQUIREMENTS.

THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1-1/2) HOURS. IF FOR ANY REASON THERE IS A LONGER DELAY THAN THAT STATED ABOVE, THE CONCRETE SHALL BE DISCARDED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LAB TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE. ALL SLABS SHALL BE CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1-D AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE. ALL SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY. CALCIUM CHLORIDES SHALL NOT BE UTILIZED; OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER.

THE GENERAL CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD A MINIMUM OF 48 HOURS PRIOR PLACEMENT OF ANY STRUCTURAL CONCRETE.

UNLESS NOTED OTHERWISE ON PLANS, THE FOLLOWING CONCRETE CLEAR COVER SHALL BE PROVIDED FOR ALL NON-PRESTRESSED CONCRETE REINFORCEMENT PER ACI 318:

CON	CRETE CAST AGAINST EARTH:	ALL BARS	-	3"
CON	CRETE EXPOSED TO EARTH (FORMED FACE):	ALL BARS	-	2"
CON	CRETE EXPOSED TO WEATHER:	#6 BARS AND GREATER	-	2"
		#5 BARS AND SMALLER	-	1 1/2"
WHE	RE NOT EXPOSED TO EARTH OR WEATHER:			
	SLABS, WALLS, AND JOISTS:	#14 & #18 BARS	-	1 1/2"
		#11 BARS AND SMALLER	-	3/4"

A) ASTM C143 - "STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE." MAXIMUM SLUMP SHALL BE 4-6 INCHES, PRIOR TO ADDING A SUPER PLASTICIZER. B) ASTM C39 - "STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS." A SEPARATE TEST SHALL BE CONDUCTED FOR EACH CLASS, FOR EVERY 50 CUBIC YARDS (OR FRACTION THEREOF), PLACED PER DAY. REQUIRED CYLINDER(S) QUANTITIES AND TEST AGE AS FOLLOWS: 1 AT 7 DAYS 2 AT 28 DAYS

ALL BARS

- 1 1/2"

ONE ADDITIONAL RESERVE CYLINDER TO BE TESTED UNDER THE DIRECTION OF THE ENGINEER, IF REQUIRED. IF 28 DAY STRENGTH IS ACHIEVED, THE ADDITIONAL CYLINDER(S) MAY BE DISCARDED.

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POUR STRUCTURAL CONCRETE WITHIN THE FOLLOWING TOLERANCES:
       VARIATION FROM PLUMB:
                                                     1/4" IN 10'-0"
       VARIATION FROM LEVEL IN TOPS OF PILASTERS: 1/8" IN 10'-0"
       VARIATION FOOTINGS:
       PLAN DIMENSIONS:
       THICKNESS:
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CHEMICAL ANCHORS:

OTHERWISE.

BEAMS AND COLUMNS:

SHALL BE AN EQUAL TWO-PART EPOXY POLYMER INJECTION SYSTEM, SUCH AS SIMPSON SET-XP "STRUCTURAL ANCHORING ADHESIVE", HILTI HIT-HY 200 OR ENGINEER APPROVED SUBSTITUTION, INSTALLED IN ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS. INSTALLERS SHALL BE TRAINED BY THE MANUFACTURER'S REPRESENTATIVE. BRUSH AND BLOW OUT ALL HOLES.

STRUCTURAL WOOD COMPONENTS (BEAMS, JOISTS, RAFTERS, ETC.) SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE FIBER STRESSES FOR NO. 2 SOUTHERN PINE CONFORMING TO NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION WITH 2015 NDS SUPPLEMENT, AS FOLLOWS:

SHEAR	FV	= 175 PSI.
BENDING 2X6	FB	= 1,000 PSI.
BENDING 2X8	FB	= 925 PSI.
BENDING 2X10	FB	= 800 PSI.
BENDING 2X12	FB	= 750 PSI.

WOOD AND BETWEEN THE NUT AND THE WOOD OF ALL BOLTS.

PERMANENT BRACING REQUIRED FOR TRUSS DESIGN.

WOOD IN CONTACT WITH CONCRETE OR MASONRY, AND AT OTHER LOCATIONS AS SHOWN ON STRUCTURAL DRAWINGS, SHALL BE PROTECTED OR PRESSURE TREATED IN ACCORDANCE WITH AITC-109. MEMBER SIZES SHOWN ARE NOMINAL UNLESS NOTED

ALL NAILS SHOWN ON PLANS ASSUME COMMON WIRE NAILS UNLESS SPECIFICALLY NOTED ON DRAWINGS. BOLTS FOR WOOD CONSTRUCTION AT INTERIOR LOCATIONS SHALL CONFORM TO ASTM A307. THREADED RODS AT INTERIOR LOCATIONS SHALL CONFORM TO ASTM A36. EXTERIOR BOLTS AND THREADED RODS PROTECTED FROM MOISTURE AND WEATHER SHALL BE HOT-DIP GALVANIZED. EXPOSED EXTERIOR BOLTS AND THREADED RODS SHALL BE AISI 316 STAINLESS STEEL. ALL NAILS LARGER THAN 10D AND SCREWS LARGER THAN 7 GAUGE SHALL BE PREDRILLED AS NEEDED TO PREVENT SPLITTING OF THE WOOD. BOLT HOLES IN WOOD MEMBERS SHALL BE A MINIMUM OF 1/32" LARGER THAN THE BOLT DIAMETER, BUT NO GREATER THAN 1/16" LARGER. A METAL PLATE OR WASHER NO SMALLER THAN A STANDARD CUT WASHER SHALL BE LOCATED BETWEEN THE BOLT HEAD AND THE

ENGINEERED WOOD TRUSS SYSTEMS SHALL BE DESIGNED BY SUPPLIER'S SPECIALTY ENGINEER TO CONFIGURATION AND LOAD-CARRYING CAPACITY SHOWN ON DRAWINGS AND SPECIFICATIONS. ALL INDIVIDUAL TRUSS MEMBERS, TRUSS PLATE CONNECTIONS, TRUSS-TO-TRUSS CONNECTIONS, COMMON TRUSSES, AND GIRDER TRUSSES SHALL BE DESIGNED FOR COMPONENT AND CLADDING WIND LOADING, EXCEPT THOSE TRUSSES EXCEEDING 700 SQUARE FEET IN TRIBUTARY AREA. ALTERNATE TRUSS LAYOUTS ARE ACCEPTABLE ONLY AS A CHANGE ORDER WHICH WILL INCLUDE ENGINEERING CHARGES FOR REDESIGN OF THE STRUCTURE BY THE SEOR. SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS SHALL SHOW AND SPECIFY ALL CONNECTOR TYPES UTILIZED WITHIN TRUSSES, AS WELL AS CONNECTORS UTILIZED IN ALL OTHER CONNECTIONS AND ATTACHMENTS BETWEEN TRUSSES OR COMPONENTS SUPPLIED AS PART OF THE ENGINEERED TRUSS SYSTEM. AN ERECTION DRAWING SHALL BE INCLUDED, IDENTIFYING ALL TRUSS SYSTEM COMPONENTS, AS WELL AS ALL

TRUSSES AT ALL BEARING LOCATIONS SHALL BE STABILIZED. THE GENERAL CONTRACTOR SHALL FOLLOW ALL REQUIREMENTS BY THE DELEGATED TRUSS ENGINEERING PACKAGE. AT A MINIMUM: FOR TRUSS HEEL DEPTHS LESS THAN 8", ONLY BLOCKING BY THE TRUSS ENGINEERING IS REQUIRED; FOR TRUSS HEEL DEPTHS BETWEEN 8" AND 12", USE FULL HEIGHT SAWN-LUMBER BLOCKING; FOR TRUSS HEEL DEPTHS GREATER THAN 12", USE TRUSS BLOCKING OR A SHEATHED KNEEWALL. SEE THE STRUCTURAL DRAWINGS FOR ANY ADDITIONAL BLOCK REQUIREMENTS FOR THE LATERAL-FORCE RESISTING SYSTEM.

ENGINEERED SHOP DRAWINGS SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A REGISTERED PROFESSIONAL ENGINEER. LICENSED IN THE STATE WHERE THE STRUCTURE WILL BE ERECTED, AS THE DELEGATED (SPECIALTY) ENGINEER. PLUMBING. ELECTRICAL, AND MECHANICAL DRAWINGS SHALL BE COORDINATED WITH THE TRUSS LAYOUT TO ENSURE THAT THERE ARE NO CONFLICTS WITH DUCTS, RECESSED FIXTURES, PLUMBING PIPES, TRAPS, HOODS, CEILING STEPS/SLOPES, ETC. TRUSS LAYOUT SHALL BE MODIFIED AND/OR TRUSS CHASES SHALL BE ADDED TO AVOID CONFLICTS. TRUSS SPACING SHALL NOT EXCEED MAXIMUM NOTED IN PLAN NOTES, U.N.O.

THE FOLLOWING LOAD DURATION FACTORS SHALL BE USED:

DEAD LOAD	0.90
DEAD LOAD + FLOOR LIVE LOAD	1.00
DEAD LOAD + ROOF LIVE LOAD	1.25
DEAD LOAD + WIND LOAD	1.60

ALL TRUSSES SHALL BE DESIGNED FOR A MAXIMUM DEFLECTION OF L/360 FOR LIVE LOAD AND L/240, NOT TO EXCEED 1", FOR TOTAL LOAD; THE MAXIMUM DEFLECTION DUE TO TOTAL LOAD OF 1" IS INCLUSIVE OF ALL DEAD LOAD, SELF-WEIGHT, SUPERIMPOSED DEAD LOAD, AND LIVE LOAD, INCLUDING CREEP.

PLYWOOD ROOF, FLOOR, AND WALL SHEATHING ARE DESIGNED AS DIAPHRAGMS/SHEAR WALLS AND SHALL COMPLY WITH APPLICABLE PROVISIONS OF CHAPTER 23 OF THE BUILDING CODE AND SHALL BE FASTENED IN ACCORDANCE WITH THE MINIMUM REQUIREMENTS OF BUILDING CODE TABLES, UNLESS SHOWN OTHERWISE. PLYWOOD SHALL BE INSTALLED WITH THE STRENGTH AXIS OF EACH PANEL PERPENDICULAR TO THE SUPPORTS IN ALL CASES. PLYWOOD ROOF PANELS SHALL BE INSTALLED AS SHOWN IN CASES 1 THROUGH 4 IN TABLE 2306.2.1 (CONT.). BLOCKING SHALL BE PROVIDED BETWEEN ALL WOOD ROOF FRAMING MEMBERS AT ALL RIDGES AND VALLEYS FOR FULL PLYWOOD EDGE SUPPORT. AT ROOF VENT LOCATIONS, PROVIDE 2X4 BLOCKING, ON THE FLAT, ON ALTERNATING SIDES OF THE VENT BETWEEN ROOF FRAMING MEMBERS.

ALL WOOD SHEAR WALLS SHALL HAVE ALL PLYWOOD EDGES FULLY BLOCKED WITH THE SAME STUD SIZE AS THE WALLS, WITH THE BLOCKING INSTALLED SO THAT THE PLYWOOD IS NAILED INTO THE NARROW STUD FACE. FASTEN THE SHEATHING WITH THE SAME NAIL SIZE AND SPACING TO EACH PLY OF DOUBLE TOP AND BOTTOM WALL PLATES, AS APPLICABLE. FASTEN THE SHEATHING WITH THE SAME NAIL SIZE AND SPACING TO MULTI-PLY COLUMNS AT ENDS OF WALLS; WHERE SOLID COLUMNS ARE USED AT ENDS OF WALLS, FASTEN THE SHEATHING WITH THE SAME NAIL SIZE AND SPACING IN VERTICAL ROWS WITH 1-1/2" ROW SPACING FOR FULL HEIGHT OF COLUMN. WHERE SHEAR WALL PANEL EDGE NAILING IS 3" OR LESS, THE BLOCKING AT PANEL EDGES SHALL BE 3" NOMINAL OR GREATER AND THE NAILING SHALL BE STAGGERED. MULTIPLE PLY STUDS MAY BE USED AS THE PANEL EDGE BLOCKING IN LIEU OF 3" NOMINAL BLOCKING; FASTEN THE PLIES WITH NAILS HAVING THE SAME LENGTH AS THE TOTAL BLOCKING THICKNESS WITH SPACING TO MATCH THE PLYWOOD PANEL EDGE NAILING, STAGGERED. PANELS SHALL NOT BE LESS THAN 4FTX8FT, EXCEPT AT BOUNDARIES AND CHANGES IN FRAMING.

ALL CONNECTORS SHALL BE GALVANIZED. CONNECTOR MODEL NUMBERS SHOWN ARE STRONG-TIE CONNECTORS AS MANUFACTURED BY SIMPSON STRONG-TIE CO., 5956 W. LAS POSITAS BLVD., P.O. BOX 10789, PLEASANTON, CA 94588, 800-999-5099, WWW.STRONGTIE.COM. SUBSTITUTIONS ARE ACCEPTABLE WITH THE APPROVAL OF THE SEOR. UNLESS SHOWN OTHERWISE, INSTALL THE *LARGEST* FASTENER SIZE AND *MAXIMUM* NUMBER OF FASTENERS SHOWN IN LATEST SIMPSON CATALOG. WHERE SDS SCREWS ARE SPECIFIED IN THE SIMPSON CATALOG, SDS SCREWS MUST BE USED UNLESS EXPRESSLY SHOWN IN THE DRAWINGS OTHERWISE. ALL ROOF AND UPPER-LEVEL UPLIFT CONNECTORS SHALL BE LOCATED ON THE SAME SIDE OF THE WALL AS THE EXTERIOR SHEATHING

ALL COLUMN BASE AND HOLD-DOWN CONNECTORS (HDU, HTT, LTT, ETC.) FOR STRUCTURAL COMPOSITE LUMBER (PSL, LVL, LSL, ETC.) SHALL BE INSTALLED IN THE WIDE FACE OF THE COLUMN, AND NOT THE NARROW FACE. THE NARROW FACE IS THE SURFACE THAT SHOWS THE VERTICAL THIN EDGES OF THE STRUCTURAL COMPOSITE LUMBER LAYERS.

FOR ALL CONNECTORS REQUIRING A THREADED ROD ATTACHMENT TO CONCRETE OR GROUT-FILLED MASONRY, PROVIDE A THREADED ROD DIAMETER AS SPECIFIED IN THE SIMPSON CATALOG WITH A DOUBLE-NUT AND OVERSIZED WASHER EMBEDDED A MINIMUM OF 9" (UNLESS NOTED OTHERWISE ON THE PLANS) INTO THE MEMBER PRIOR TO THE POUR.

	WIND LC	AD SCHEDULE		
	SCHEDULE OF CO	OMPONENTS AND CLADDI	NG LOADS	
ZONE	ZONE DESCRIPTION	TRIBUTARY AREA (SF)	IN (PRESSURE) (+PSF)	OUT (PRESSURE (- PSF)
1	ROOF INTERIOR ZONE	LESS THAN 20 20 - 100 MORE THAN 100	12.3 11.5 10.0	48.2 45.6 37.9
2	ROOF EDGE ZONE	LESS THAN 20 20 - 100 MORE THAN 100	12.3 11.5 10.0	63.6 59.7 50.8
3	ROOF CORNER ZONE	LESS THAN 20 20 - 100 MORE THAN 100	12.3 11.5 10.0	86.7 79.0 59.7
4	WALL INTERIOR ZONE	LESS THAN 20 20 - 50 20 - 100 MORE THAN 100	27.7 26.5 24.9 23.5	30.0 28.8 27.2 26.5
5	WALL EDGE ZONE	LESS THAN 20 20 - 50 20 - 100 MORE THAN 100	27.7 26.5 24.9 23.5	36.9 34.6 31.2 28.8

NOTE: WIND PRESSURES SHOWN ARE BASED ON Vasd

ASCE 7-16			1
ALLOWABLE WIND SPEED Vasd = 118 MPH RISK CATEGORY = II EXPOSURE = C ENCLOSURE CLASSIFICATION = OPEN INTERNAL PRESSURE COEFFICIENT (GCpi) =	CODE =	ASCE 7-16	
RISK CATEGORY = II EXPOSURE = C ENCLOSURE CLASSIFICATION = OPEN INTERNAL PRESSURE COEFFICIENT (GCpi) = ±0.18 a = 10 FT 2a = 20 FT HIP ROOF (7 ° < ⊖ ≤ 27 ° SCALE: N.T.S.	ULTIMATE WIND SPEED Vult =	152 MPH	
EXPOSURE = C C ENCLOSURE CLASSIFICATION = OPEN INTERNAL PRESSURE COEFFICIENT (GCpi) =	ALLOWABLE WIND SPEED Vasd =	118 MPH	
ENCLOSURE CLASSIFICATION = OPEN INTERNAL PRESSURE COEFFICIENT (GCpi) = ±0.18 a = 10 FT 2a = 20 FT HIP ROOF (7° < Θ ≤ 27° SCALE: N.T.S.	RISK CATEGORY =	II	
INTERNAL PRESSURE COEFFICIENT (GCpi) = #0.18 a = 10 FT	EXPOSURE =	С	
a = 10 FT	ENCLOSURE CLASSIFICATION =	OPEN	
2a = 20 FT A	INTERNAL PRESSURE COEFFICIENT (GCpi) =	±0.18	
a 3 2 3 3	a =	10 FT	
HIP ROOF (7° < ⊖ ≤ 27° SCALE: N.T.S.	2a =	20 FT	
	2 2 2 1 2 2 1 2 2 2 2 2 2 2 2 2 2 2 2 2	5	ROOF (7° < Θ ≤ 27°
	m 2		

ZONE 1 - ROOF

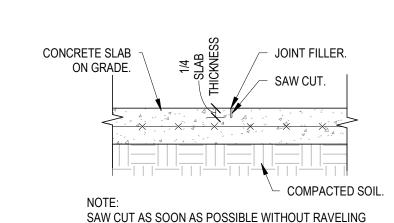
ZONE 4 - WALL

ZONE 2 - ROOF

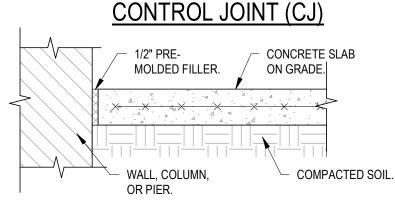
ZONE 5 - WALL

Corner Zones

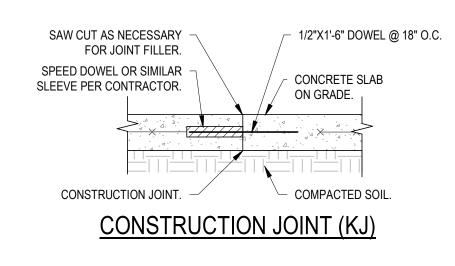
ZONE 3 - ROOF



CONCRETE (4 TO 16 HOURS AFTER POURING CONCRETE).



ISOLATION JOINT (IJ)



MARK	DESCRIPTION
#	POUND(S)
" &	AND
ARCH	ARCHITECT
B OR BOT	BOTTOM
B/ OR B.O.	BOTTOM OF
BP	BASE PLATE
CANT.	CANTILEVER
CJ	CONTROL JOINT
CLR	CLEAR
CONT.	CONTINUOUS
DIA / Ø	DIAMETER
EA. EE	EACH END
EF .	EACH FACE
EJ	EXPANSION JOINT
EL	ELEVATION
EMBED	EMBEDMENT
EOS	EDGE OF SLAB
EQ	EQUAL
EW	EACH WAY
EXIST	EXISTING
F'C	28 DAY COMPRESSIVE STRENGTH
FD	FLOOR DRAIN
FFE	FINISHED FLOOR ELEVATION
FRT	FIRE RETARDANT TREATED
FTG.	FOOTING
GALV	GAUVANIZED
GALV. GC	GALVANIZED GENERAL CONTRACTOR
H OR HORIZ	HORIZONTAL
HSS	HOLLOW STRUCTURAL STEEL
KJ	CONSTRUCTION JOINT
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LW	LONG WAY
MAX	MAXIMUM
MIN	MINIMUM
N.A.V.D.	NORTH AMERICAN VERTICAL DATUM
N.G.V.D.	NATIONAL GEODETIC VERTICAL DATU
NIC	NOT IN CONTRACT
O.C.	ON CENTER
OH OR OPP HAND PLF	OPPOSITE HAND POUNDS PRE LINEAR FOOT
PROJ	PROJECTION
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	PRESSURE TREATED/ POST TENSIONE
RD	ROOF DRAIN
REF	REFERENCE
REINF	REINFORCING
REQ'D	REQUIRED
RTF	RIPPED TO FIT
SEOR	STRUCTURAL ENGINEER OF RECORD
SIM	SIMILAR
SS	STAINLESS STEEL
STD SW	STANDARD SHORT WAY/ SHEAR WALL
T Sw	TOP
T.O. OR T/	TOP OF
TEMP	TEMPERATURE
THR	THREADED
THRU	THROUGH
TRANS	TRANSVERSE
TYP	TYPICAL
V OR VERT	VERTICAL
W/	WITH
WD	WOOD
WP	WORKING POINT

ARCH	ARCHITECT
B OR BOT	ВОТТОМ
B/ OR B.O.	BOTTOM OF
BP	BASE PLATE
CANT.	CANTILEVER
CJ	CONTROL JOINT
CLR	CLEAR
CONT.	CONTINUOUS
DIA / Ø	DIAMETER
EA.	EACH
EE	EACH END
EF	EACH FACE
EJ	EXPANSION JOINT
EL	ELEVATION
EMBED	EMBEDMENT
EOS	EDGE OF SLAB
EQ	EQUAL EQUAL
EW	EACH WAY
EXIST	EXISTING
F'C	28 DAY COMPRESSIVE STRENGTH
FD	FLOOR DRAIN
FFE	FINISHED FLOOR ELEVATION
FRT	FIRE RETARDANT TREATED
FTG.	FOOTING
GA.	GAUGE
ga. Galv.	GALVANIZED
GC	GENERAL CONTRACTOR
H OR HORIZ	HORIZONTAL
HSS	HOLLOW STRUCTURAL STEEL
KJ	CONSTRUCTION JOINT
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LW	LONG WAY
MAX	MAXIMUM
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N.A.V.D.	NORTH AMERICAN VERTICAL DATUM
N.G.V.D.	NATIONAL GEODETIC VERTICAL DATUM
NIC	NOT IN CONTRACT
O.C.	ON CENTER
OH OR OPP HAND	OPPOSITE HAND
PLF	POUNDS PRE LINEAR FOOT
PROJ	PROJECTION
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PT	PRESSURE TREATED/ POST TENSIONED
RD	ROOF DRAIN
REF	REFERENCE
REINF	REINFORCING
REQ'D	REQUIRED
RTF	RIPPED TO FIT
SEOR	STRUCTURAL ENGINEER OF RECORD
SIM	SIMILAR
SS	STAINLESS STEEL
STD	STANDARD
SW	· · · · · · · · · · · · · · · · · · ·
	SHORT WAY/ SHEAR WALL
T O OD T	TOP
T.O. OR T/	TOP OF
TEMP	TEMPERATURE
THR	THREADED
THRU	THROUGH
TRANS	TRANSVERSE
TYP	TYPICAL
• • •	VERTICAL
V OR VERT	-
W/	WITH
WD	WOOD
	WORKING POINT
WP	VVOINTINGTOINT

SHEET LIST

MAIL KIOSK - STRUCTURAL

MAIL KIOSK - FLOOR AND

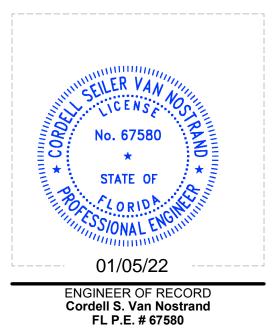
ROOF FRAMING PLANS,

SECTIONS





ORLANDO 189 S. ORANGE AVE., SUITE 1700 ORLANDO, FLORIDA 32801 407 926 3000 **INFO@BAKERBARRIOS.COM BAKERBARRIOS.COM** AA0002981 | LC26000427



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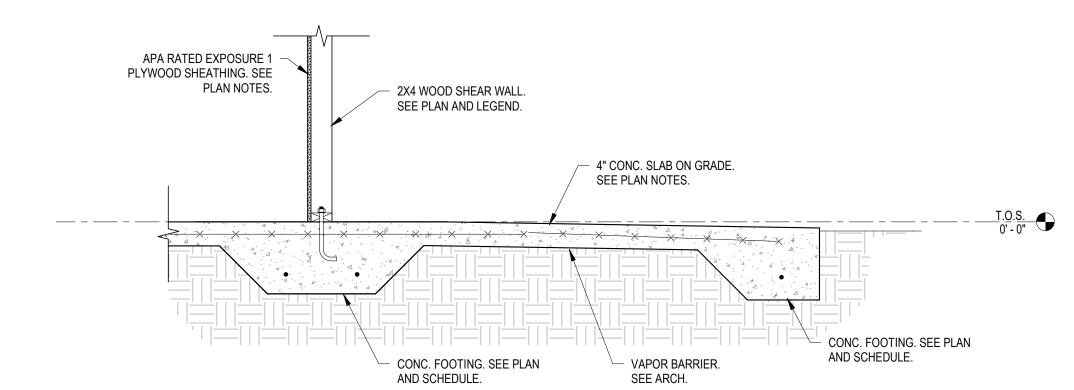


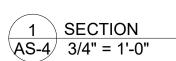
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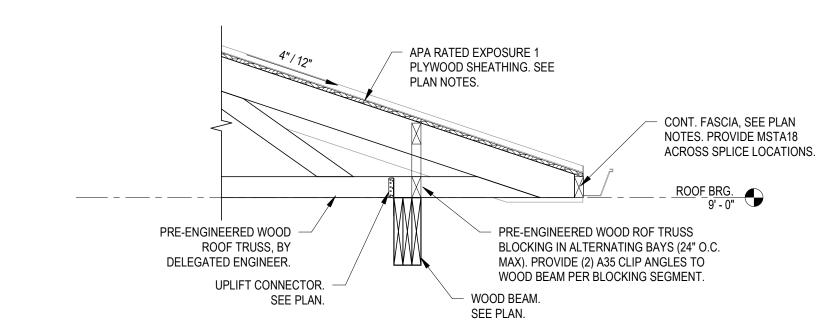
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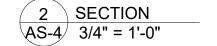
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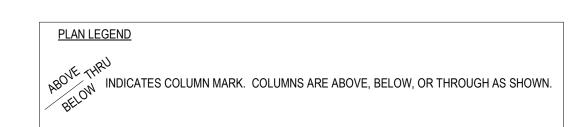
MAIL KIOSK -STRUCTURAL NOTES

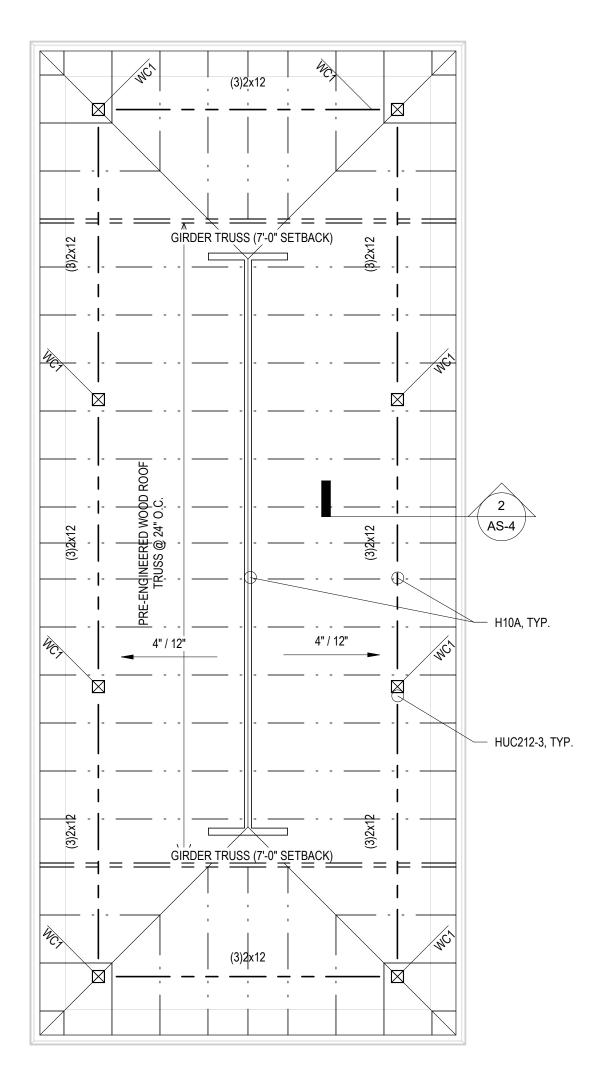












ROOF FRAMING PLAN

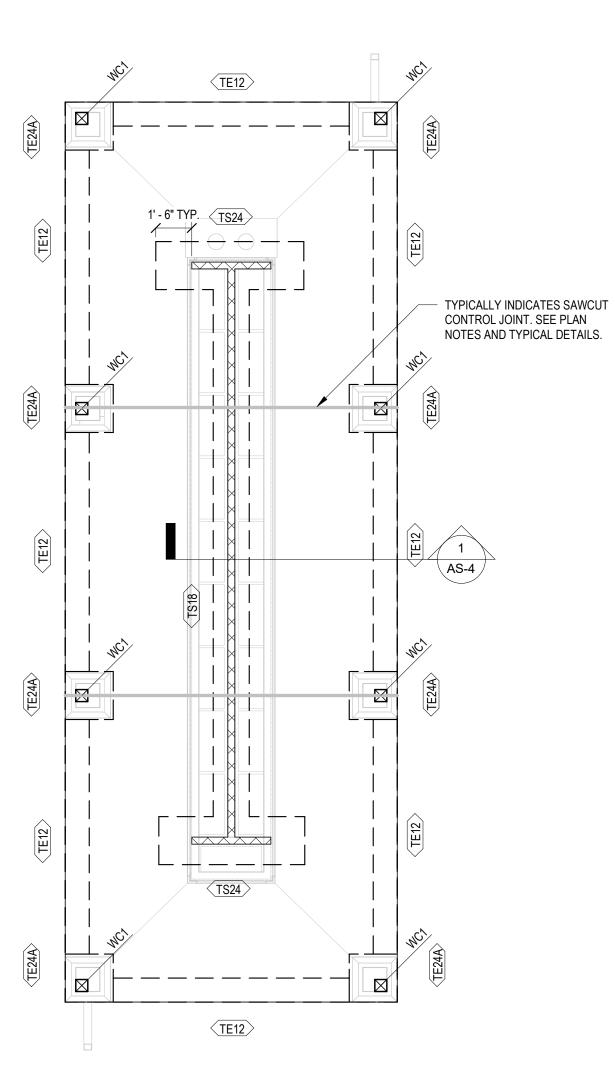
ROOF FRAMING PLAN NOTES:

EOR OF ANY DISCREPANCIES.

- 1. THE ROOF FRAMING SYSTEM SHALL CONSIST OF WOOD ROOF TRUSSES BY DELEGATED ENGINEER AT 24" O.C. VERIFY CEILING HEIGHTS AND CONFIGURATIONS WITH THE ARCHITECTURAL PLANS. SEE ARCHITECTURAL PLANS FOR ROOF SLOPE AND EAVE DETAILS. TRUSS BEARING ELEVATION = 9'-0" UNLESS NOTED OTHERWISE ON PLAN.
- 2. - INDICATES WOOD TRUSS SPAN DIRECTION AND EXTENTS.

 VERIFY ALL GIRDER LOCATIONS AND PLIES WITH TRUSS ENGINEERING AND NOTIFY
- 3. ROOF SHEATHING SHALL BE 19/32" APA RATED EXTERIOR PLYWOOD ROOF SHEATHING. ATTACHMENT FOR ZONES 1 & 2, AS DEPICTED IN THE COMPONENT AND CLADDING WIND SCHEDULE, SHALL BE MADE W/ 10D NAILS @ 6" O.C. AT SUPPORTED PANEL EDGES AND 12" O.C. IN THE FIELD. ATTACHMENT FOR ZONE 3 IS TO BE MADE W/ 10D NAILS @ 4" O.C. AT SUPPORTED PANEL EDGES AND IN THE FIELD. PROVIDE PLYWOOD "H" CLIPS @ UNSUPPORTED PLYWOOD PANEL EDGES.
 - ALL WOOD TRUSSES SHALL BE ATTACHED TO THE STRUCTURE WITH THE SPECIFIED SIMPSON CONNECTOR.
- 5. THE TRUSS ENGINEER SHALL SPECIFY THE TRUSS-TO-TRUSS CONNECTIONS.
- PLYWOOD SHEATHING SHALL BE CONTINUOUS ON THE TOP CHORD OF THE TRUSSES. ALL OVER-FRAMING OR VALLEY SETS SHALL BE PLACED ABOVE THE CONTINUOUS PLYWOOD.
- 7. ALL WOOD IN CONTACT WITH MASONRY, CONCRETE OR USED IN EXTERIOR APPLICATIONS SHALL BE PRESSURE TREATED WITH PRESERVATIVE. (P.T.) EXTERIOR CONNECTORS SHALL BE HOT DIPPED GALVANIZED.
- 8. PROVIDE 2X CONT. FASCIA AT EDGE OF ROOF. PROVIDE MSTA24 ACROSS ALL SPLICE LOCATIONS.
- 9. ATTACH ALL TRUSSES TO WOOD WALLS OR WOOD BEAMS WITH H10A, U.N.O. ON

WOO	DD COLU	IMN SCHED	ULE
MARK	TYPE	ATTACHMENT	REMARKS
\MC1	6Y6 DT	CDT667	-



FOUNDATION & GROUND FLOOR PLAN

FOUNDATION & GROUND FLOOR PLAN NOTES:

- VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL AND MEP FOR ADDITIONAL INFORMATION.
- 2. ELEVATIONS ARE CALLED OUT RELATIVE TO THE TOP OF MAIN GROUND FLOOR SLAB, SET AT 0'-0" (REF.).
- 3. SEE ARCHITECTURAL DRAWINGS FOR ALL SLOPES, DROPS AND DRAIN LOCATIONS IN FLOOR SLAB. MAINTAIN 4" MINIMUM SLAB DEPTH. THICKEN SLAB TO 8" WITHIN 4" OF ALL SLAB STEPS; MAINTAIN 4" MINIMUM SLAB DEPTH ELSEWHERE.
- 4. SEE ARCHITECTURAL DRAWINGS FOR ALL SLOPES, DROPS AND DRAIN LOCATIONS IN FLOOR SLAB. MAINTAIN 4" MINIMUM SLAB DEPTH. THICKEN SLAB TO 8" WITHIN 4" OF ALL SLAB STEPS; MAINTAIN 4" MINIMUM SLAB DEPTH ELSEWHERE.
- GROUND FLOOR SHALL BE 4" CONCRETE SLAB-ON-GRADE, U.N.O. REINFORCE W/ 6X6 W1.4XW1.4 W.W.F. AT MID-DEPTH.
- PROVIDE CONTROL JOINTS IN ALL SLABS ON GRADE. CONTROL JOINTS SHALL BE TOOLED OR SAWCUT AS SOON AS POSSIBLE WITHOUT RAVELING. THE PATTERN SHALL BE APPROXIMATELY SQUARE AND LIMITED TO AN AREA OF 144 SF. SEE PLAN

WALL LEGEND

INDICATES A 2X4 SO. PINE #2 EXTERIOR SHEAR WALL WITH STUDS AT 16" O.C. MAX. PROVIDE (1) JACK STUD AND (2) KING STUDS ON EACH SIDE OF EACH OPENING. EACH STUD IN WALL TO HAVE AN SP1 TO SINGLE BOTTOM PLATE AND SP2 TO DOUBLE-TOP PLATE. PROVIDE HORIZONTAL BLOCKING AT 4'-0" O.C. MAX. ATTACH BOTTOM PLATE W/ 1/2" J BOLTS EMBEDDED 8" INTO CONCRETE BELOW @ 18" O.C. MAX. PROVIDE (2) 2X10 HEADERS OVER OPENINGS UNLESS NOTED OTHERWISE. PROVIDE FULL BLOCKING AT ALL PLYWOOD JOINTS. PROVIDE 15/32" APA RATED EXPOSURE 1 PLYWOOD SHEATHING ON ONE SIDE OF WALL (MIN.) WITH 10D NAILS AT 3" O.C. AT ALL EDGES AND AT 12" O.C. IN THE FIELD. INSTALL EXPOSED PLYWOOD GRAIN PERPENDICULAR TO STUDS. INSTALL (3) 2X4 COLUMNS AT ENDS AND INTERSECTIONS OF WOOD SHEAR WALLS, WITH HTT4 UPLIFT CONNECTOR AT BASE.

	FC	UNDAT	TION SC	HEDULE	
MARK	WIDTH	LENGTH	DEPTH	REINFORCING	REMARKS
TE12	1' - 0"	CONT.	1' - 0"	1#5 CONT	T.O. FTG. = T.O. SLAB
TE24A	2' - 0"	2' - 0"	1' - 3"	3#5 B.E.W.	T.O. FTG. = T.O. SLAB
TS18	1' - 6"	CONT.	1' - 0"	2#5 BOT CONT	T.O. FTG. = T.O. SLAB
TS24	2' - 0"	CONT.	1' - 0"	2#5 BOT CONT	T.O. FTG. = T.O. SLAB

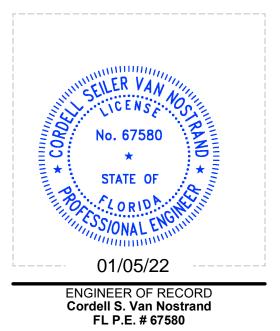
FOUNDATION SCHEDULE NOTES:

PROVIDE CORNER BARS AT ALL FOOTING INTERSECTIONS.
 PROVIDE 3" CLEAR COVER FOR ALL REINFORCING UNLESS DETAILED OTHERWISE.
 BOTTOM OF ALL FOUNDATIONS SHALL BE BELOW THE FROST LINE BASED ON FINISHED GRADE. GC SHALL COORDINATED WITH CIVIL FOR FINISHED GRADES. IN NO CASE SHALL FOUNDATIONS BE LESS THAN 12" BELOW ADJACENT GRADE, EXCEPT FOR NON-LOAD BEARING TE FOOTINGS.



ORLANDO

189 S. ORANGE AVE., SUITE 1700
ORLANDO, FLORIDA 32801
407 926 3000
INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
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MAIL KIOSK -FLOOR AND ROOF FRAMING PLANS, SECTIONS

STRUCTURAL NOTES GENERAL NOTES:

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

ALL DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK. DO NOT SCALE DRAWINGS.

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS.

ELECTRONIC VERSIONS OF THE STRUCTURAL DRAWINGS ARE THE SOLE, COPYRIGHTED PROPERTY OF SNELL ENGINEERING AND ARE NOT TO BE USED OR TRANSFERRED WITHOUT THE EXPRESS, WRITTEN PERMISSION OF SNELL ENGINEERING.

<u>DESIGN LOADS</u>:

THE STRUCTURAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE – BUILDING 7TH EDITION (2020). THE FOLLOWING SUPERIMPOSED LOADINGS HAVE BEEN UTILIZED:

WIND:			
	ASCE 7-16		
	ULTIMATE WIND SPEED	-	152 MPH
	ALLOWABLE WIND SPEED	-	118 MPH
	EXPOSURE C		
	ENCLOSED STRUCTURE INTERNAL PRESSURE COEFFICIENT		+/- 0.18
	RISK FACTOR II	-	+/- 0.10
	SEE WIND SCHEDULE FOR PRESSURES		
ROOF:			
11001.	LIVE LOAD	_	20 PSF.
	LIVE LOAD (CONCENTRATED)	_	300 LBS.
	DEAD LOAD	_	25 PSF.
	DEAD LOAD (AVAILABLE TO RESIST UPLIFT)	-	5 PSF.
FLOOD:			
	FEMA A/V ZONE		
	FLOOD DESIGN CLASS	-	II
	BFE ELEVATION	-	11.00 NAVD
	FREEBOARD	-	1 FT
	PROPOSED LOWEST FLOOR ELEVATION	-	12.00 NAVD
	BOT. OF LOWEST HORIZ. STRUCT. MEMBER	-	12.00 NAVD
	REQ'D MIN. DRY FLOOD-PROOF ELEVATION	-	12.00 NAVD
	STRUCTURAL SYSTEMS OF BUILDINGS AND STRUC		
	RESIST FLOTATION, COLLAPSE OR PERMANENT LA		
	FBC-R 322.1.2/FBC 1612.1.	SN FLOOD EI	LEVATION IN CONFORMANCE WITH ASCE 24-14 AND
SEISMIC	· ·		
OLIOWIC	RISK CATEGORY -	II	
	SEISMIC IMPORTANCE FACTOR le -	1.0	
	SITE CLASS D	•	
	SEISMIC DESIGN CATEGORY A		

IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO CONFIRM ALL FINAL WEIGHTS OF EQUIPMENT (MECHANICAL, CHILLED WATER, KITCHEN, SPA/POOL, ETC.), CLADDING, FINISHES, ETC. PRIOR TO ERECTING STRUCTURE SUPPORTING THESE ITEMS. ALL WEIGHTS ACCOUNTED FOR IN THE STRUCTURAL DESIGN ARE LISTED ABOVE OR ON THE PLANS.

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SHOP DRAWING REVIEW:

FOR AN ADEQUATE REVIEW PERIOD.

Ss - 1.094 g

Sds - 0.775 g

SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC.

ANY COMPONENT NOTED AS "DELEGATED" SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF FLORIDA AND NOT BY THE SER. SIGNED AND SEALED DRAWINGS SHALL BE PROVIDED TO THE ARCHITECT AND SEOR FOR REVIEW

ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW NOTATION WILL BE RETURNED UNCHECKED. EVERY EFFORT WILL BE MADE TO RETURN THE SHOP DRAWINGS WITHIN TEN BUSINESS DAYS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO SUBMIT THE SHOP DRAWINGS ALLOWING

ONE SET OF PRINTS WILL BE RETAINED BY THE ENGINEER AND ONE BY THE ARCHITECT, THE CONTRACTOR SHALL RECEIVE THE REMAINING PRINTS FOR SUBMITTAL TO THE BUILDING DEPARTMENT AND AS REQUIRED FOR DISTRIBUTION.

IN ALL INSTANCES THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN A REQUEST FOR INFORMATION (RFI) OR SIMILAR DOCUMENTATION BY THE ENGINEER. EVERY EFFORT WILL BE MADE TO RETURN THE RFIS WITHIN TWO BUSINESS DAYS. SUBMITTALS SHALL CLEARLY IDENTIFY THE SPECIFIC PROJECT, APPLICABLE CODES AND DESIGN CRITERIA, AND DETAILING OF ALL COMPONENTS NECESSARY TO ENSURE PROPER INSTALLATION OF THE COMPONENTS

SHOP DRAWINGS SHOULD BE SUBMITTED FOR ALL COMPONENTS OF THE STRUCTURAL FRAMING SYSTEM, AS REQUIRED BY THE ARCHITECT, AND AS NOTED ELSEWHERE IN THESE NOTES, INCLUDING, BUT NOT LIMITED TO: CONCRETE MIX DESIGNS CONCRETE REINFORCEMENT

PRE-ENGINEERED WOOD ROOF TRUSSES ANY PROPOSED MANUFACTURER CHANGE FROM THE BASIS OF DESIGN

AS A SHOP DRAWING; CALCULATIONS WILL BE PROVIDED IF REQUESTED.

IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE ALL TRADES AND CONSULTANTS, CROSS REFERENCE THEIR DRAWINGS WITH THE OVERALL DESIGN, AND PROVIDE TO EACH A COMPLETE SET OF DRAWINGS AND SUBMITTALS TO ENSURE COMPATIBILITY OF CONSTRUCTION PER DESIGN INTENT.

FOUNDATIONS:

ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2,000 PSF ON COMPACTED FILL OR COMPACTED NATIVE SOIL. BEFORE CONSTRUCTION COMMENCES AND IF REQUIRED BY THE JURISDICTION, SOIL BEARING CAPACITY SHALL BE VERIFIED BY A SUBSURFACE INVESTIGATION, AS WELL AS FIELD AND LABORATORY TESTS PERFORMED BY A CERTIFIED TESTING LABORATORY, WHOSE REPORT SHALL INCLUDE ANALYSIS AND RECOMMENDATIONS FOR SITE PREPARATION IN ORDER TO BEAR THE FOUNDATION LOADS. ABOVE REPORT SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW BEFORE FOUNDATION CONSTRUCTION BEGINS.

PENETRATIONS:

NO PENETRATIONS SHALL BE MADE IN ANY STRUCTURAL MEMBERS WITHOUT PREVIOUS APPROVAL OF THE EOR, EXCEPT THOSE PENETRATIONS SHOWN ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHOULD SUBMIT DRAWINGS SHOWING: ANY LOCATION WHERE OPENINGS, PENETRATIONS, OR ANY PLACE MORE THAN 3 PIPES OR CONDUITS ARE LOCATED IN A SLAB SYSTEM; ANY LOCATION IN A BEAM OR COLUMN WHERE MORE THAN 4% OF THE MEMBER SECTION IS REMOVED IN ANY

PLUMBING SLEEVES:

MINIMUM SLEEVE SPACING SHALL BE THREE DIAMETERS CENTER TO CENTER OF THE LARGER SLEEVE OR 6" CLEAR BETWEEN SLEEVES, WHICHEVER IS GREATER. PRIOR TO CONSTRUCTION SLEEVE LOCATIONS AND SIZES SHALL BE APPROVED BY THE ENGINEER. CONDUITS, PIPES AND SLEEVES SHALL BE PLACED AND SPACED IN ACCORDANCE WITH ACI 318 6.3.

REINFORCING STEEL:

SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. SECURE APPROVAL OF SHOP DRAWINGS PRIOR TO COMMENCING FABRICATION.

REINFORCING BARS SHOWN IN SECTIONS DEPICT TYPICAL CONFIGURATION AND ARE NOT SPECIFIC TO THE CONCRETE MEMBER CUT ON PLAN; SEE PLAN AND SCHEDULES FOR ALL BAR SIZE AND QUANTITIES. TAKE-OFFS AND QUANTITIES SHALL BE OBTAINED FROM THE SCHEDULES AND PLANS, NOT FROM SECTIONS. WHERE HOOKS, LAP LENGTHS, ETC. ARE SHOWN IN SECTIONS, ALL LENGTHS AND DETAILS SHALL MEET ACI REQUIREMENTS FOR REINFORCING DETAILS. HOOKS SHALL BE ORIENTED AS REQUIRED TO FIT WITHIN THE CONCRETE MEMBER.

WELDED WIRE FABRIC

TO CONFORM TO ASTM A-185, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. MINIMUM LAP SHALL BE ONE SPACE PLUS TWO INCHES.

ALL CONCRETE SHALL MEET ACI 318 'BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE' AND ACI 301 'SPECIFICATIONS FOR STRUCTURAL CONCRETE', INCLUDING, BUT NOT LIMITED TO, MIX DESIGN, STRENGTH, COVER, PLACEMENT, CURING, TESTING, FORMS, FLATNESS, ETC. SEE THE ARCHITECTURAL SPECIFICATIONS AND PLANS FOR ANY ADDITIONAL REQUIREMENTS.

ALL CONCRETE SHALL BE AN APPROVED MIX DESIGN PROPORTIONED TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW WITH A PLASTIC AND WORKABLE MIX:

3000 PSI FOR FOUNDATIONS AND SLABS ON GRADE.

SUBMIT PROPOSED MIX DESIGN FOR EACH PORTION OF WORK PRIOR TO USE. ALL MIX DESIGNS SHALL BE ACCOMPANIED BY A MINIMUM OF 15 FIELD STRENGTH TEST RECORDS, AS NOTED IN ACI 301 4.2.3.2(A); NO OTHER DOCUMENTATION OF AVERAGE COMPRESSIVE STRENGTH WILL BE ACCEPTED WITHOUT PRIOR WRITTEN APPROVAL BY THE SER. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. MIX SHALL MEET THE REQUIREMENTS OF ASTM C33 FOR COARSE AGGREGATE. FOR ALL FLATWORK, AT LEAST 75% OF LARGE AGGREGATE SHALL CONSIST OF #57 STONE. CONCRETE SHALL COMPLY WITH ALL THE REQUIREMENTS OF ASTM STANDARD C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED.

ALL CONCRETE MIX DESIGNS SHALL INCLUDE A WRITTEN DESCRIPTION INDICATING WHERE EACH PARTICULAR MIX IS TO BE PLACED WITHIN THE STRUCTURE.

CONCRETE SHALL BE PLACED AND CURED ACCORDING TO ALL STANDARDS AND SPECIFICATIONS, INCLUDING ALL ACI REQUIREMENTS.

THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1-1/2) HOURS. IF FOR ANY REASON THERE IS A LONGER DELAY THAN THAT STATED ABOVE, THE CONCRETE SHALL BE DISCARDED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LAB TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE. ALL SLABS SHALL BE CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1-D AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE. ALL SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY. CALCIUM CHLORIDES SHALL NOT BE UTILIZED; OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER.

THE GENERAL CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD A MINIMUM OF 48 HOURS PRIOR PLACEMENT OF ANY STRUCTURAL CONCRETE.

UNLESS NOTED OTHERWISE ON PLANS, THE FOLLOWING CONCRETE CLEAR COVER SHALL BE PROVIDED FOR ALL NON-PRESTRESSED CONCRETE REINFORCEMENT PER ACI 318:

CONCRETE CAST AGAINST EARTH:	ALL BARS	-	3"
CONCRETE EXPOSED TO EARTH (FORMED FACE):	ALL BARS	-	2"
CONCRETE EXPOSED TO WEATHER:	#6 BARS AND GREATER	-	2"
	#5 BARS AND SMALLER	-	1 1
WHERE NOT EXPOSED TO EARTH OR WEATHER:			
SLABS, WALLS, AND JOISTS:	#14 & #18 BARS	-	1 1
	#11 BARS AND SMALLER	-	3/4
BEAMS AND COLUMNS:	ALL BARS	-	1 1

A) ASTM C143 - "STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE." MAXIMUM SLUMP SHALL BE 4-6 INCHES, PRIOR TO ADDING A SUPER PLASTICIZER.

B) ASTM C39 - "STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS." A SEPARATE TEST SHALL BE CONDUCTED FOR EACH CLASS, FOR EVERY 50 CUBIC YARDS (OR FRACTION THEREOF), PLACED PER DAY. REQUIRED CYLINDER(S) QUANTITIES AND TEST AGE AS FOLLOWS:

1 AT 7 DAYS
2 AT 28 DAYS

ONE ADDITIONAL RESERVE CYLINDER TO BE TESTED UNDER THE DIRECTION OF THE ENGINEER, IF REQUIRED. IF 28 DAY STRENGTH IS ACHIEVED, THE ADDITIONAL CYLINDER(S) MAY BE DISCARDED.

POUR STRUCTURAL CONCRETE WITHIN THE FOLLOWING TOL	ERANCES:
VARIATION FROM PLUMB:	1/4" IN 10'
VARIATION FROM LEVEL IN TOPS OF PILASTERS:	1/8" IN 10'
VARIATION FOOTINGS:	
PLAN DIMENSIONS:	+2", - 1/2"
THICKNESS:	- O"

CHEMICAL ANCHORS

SHALL BE AN EQUAL TWO-PART EPOXY POLYMER INJECTION SYSTEM, SUCH AS SIMPSON SET-XP "STRUCTURAL ANCHORING ADHESIVE", HILTI HIT-HY 200 OR ENGINEER APPROVED SUBSTITUTION, INSTALLED IN ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS. INSTALLERS SHALL BE TRAINED BY THE MANUFACTURER'S REPRESENTATIVE. BRUSH AND BLOW OUT ALL HOLES.

WOOD:

STRUCTURAL WOOD COMPONENTS (BEAMS, JOISTS, RAFTERS, ETC.) SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE FIBER STRESSES FOR NO. 2 SOUTHERN PINE CONFORMING TO NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION WITH 2015 NDS SUPPLEMENT, AS FOLLOWS:

SHLAN	I V	_	175 F SI.
BENDING 2X6	6 FB	= 1	,000 PSI.
BENDING 2X8	B FB	=	925 PSI.
BENDING 2X1	0 FB	=	800 PSI.
BENDING 2X1	2 FB	=	750 PSI.

WOOD IN CONTACT WITH CONCRETE OR MASONRY, AND AT OTHER LOCATIONS AS SHOWN ON STRUCTURAL DRAWINGS, SHALL BE PROTECTED OR PRESSURE TREATED IN ACCORDANCE WITH AITC-109. MEMBER SIZES SHOWN ARE NOMINAL UNLESS NOTED OTHERWISE

ALL NAILS SHOWN ON PLANS ASSUME COMMON WIRE NAILS UNLESS SPECIFICALLY NOTED ON DRAWINGS. BOLTS FOR WOOD CONSTRUCTION AT INTERIOR LOCATIONS SHALL CONFORM TO ASTM A307. THREADED RODS AT INTERIOR LOCATIONS SHALL CONFORM TO ASTM A36. EXTERIOR BOLTS AND THREADED RODS PROTECTED FROM MOISTURE AND WEATHER SHALL BE HOT-DIP GALVANIZED. EXPOSED EXTERIOR BOLTS AND THREADED RODS SHALL BE AISI 316 STAINLESS STEEL. ALL NAILS LARGER THAN 10D AND SCREWS LARGER THAN 7 GAUGE SHALL BE PREDRILLED AS NEEDED TO PREVENT SPLITTING OF THE WOOD. BOLT HOLES IN WOOD MEMBERS SHALL BE A MINIMUM OF 1/32" LARGER THAN THE BOLT DIAMETER, BUT NO GREATER THAN 1/16" LARGER. A METAL PLATE OR WASHER NO SMALLER THAN A STANDARD CUT WASHER SHALL BE LOCATED BETWEEN THE BOLT HEAD AND THE WOOD AND BETWEEN THE NUT AND THE WOOD OF ALL BOLTS.

ENGINEERED WOOD TRUSS SYSTEMS SHALL BE DESIGNED BY SUPPLIER'S SPECIALTY ENGINEER TO CONFIGURATION AND LOAD-CARRYING CAPACITY SHOWN ON DRAWINGS AND SPECIFICATIONS. ALL INDIVIDUAL TRUSS MEMBERS, TRUSS PLATE CONNECTIONS, TRUSS-TO-TRUSS CONNECTIONS, COMMON TRUSSES, AND GIRDER TRUSSES SHALL BE DESIGNED FOR COMPONENT AND CLADDING WIND LOADING, EXCEPT THOSE TRUSSES EXCEEDING 700 SQUARE FEET IN TRIBUTARY AREA. ALTERNATE TRUSS LAYOUTS ARE ACCEPTABLE ONLY AS A CHANGE ORDER WHICH WILL INCLUDE ENGINEERING CHARGES FOR REDESIGN OF THE STRUCTURE BY THE SEOR. SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS SHALL SHOW AND SPECIFY ALL CONNECTOR TYPES UTILIZED WITHIN TRUSSES, AS WELL AS CONNECTORS UTILIZED IN ALL OTHER CONNECTIONS AND ATTACHMENTS BETWEEN TRUSSES OR COMPONENTS SUPPLIED AS PART OF THE ENGINEERED TRUSS SYSTEM. AN ERECTION DRAWING SHALL BE INCLUDED, IDENTIFYING ALL TRUSS SYSTEM COMPONENTS, AS WELL AS ALL PERMANENT BRACING REQUIRED FOR TRUSS DESIGN.

TRUSSES AT ALL BEARING LOCATIONS SHALL BE STABILIZED. THE GENERAL CONTRACTOR SHALL FOLLOW ALL REQUIREMENTS BY THE DELEGATED TRUSS ENGINEERING PACKAGE. AT A MINIMUM: FOR TRUSS HEEL DEPTHS LESS THAN 8", ONLY BLOCKING BY THE TRUSS ENGINEERING IS REQUIRED; FOR TRUSS HEEL DEPTHS BETWEEN 8" AND 12", USE FULL HEIGHT SAWN-LUMBER BLOCKING; FOR TRUSS HEEL DEPTHS GREATER THAN 12", USE TRUSS BLOCKING OR A SHEATHED KNEEWALL. SEE THE STRUCTURAL DRAWINGS FOR ANY ADDITIONAL BLOCK REQUIREMENTS FOR THE LATERAL-FORCE RESISTING SYSTEM.

ENGINEERED SHOP DRAWINGS SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A REGISTERED PROFESSIONAL ENGINEER, LICENSED IN THE STATE WHERE THE STRUCTURE WILL BE ERECTED, AS THE DELEGATED (SPECIALTY) ENGINEER. PLUMBING, ELECTRICAL, AND MECHANICAL DRAWINGS SHALL BE COORDINATED WITH THE TRUSS LAYOUT TO ENSURE THAT THERE ARE NO CONFLICTS WITH DUCTS, RECESSED FIXTURES, PLUMBING PIPES, TRAPS, HOODS, CEILING STEPS/SLOPES, ETC. TRUSS LAYOUT SHALL BE MODIFIED AND/OR TRUSS CHASES SHALL BE ADDED TO AVOID CONFLICTS. TRUSS SPACING SHALL NOT EXCEED

MAXIMUM NOTED IN PLAN NOTES, U.N.O. THE FOLLOWING LOAD DURATION FACTORS SHALL BE USED:

DEAD LOAD	0.90
DEAD LOAD + FLOOR LIVE LOAD	1.00
DEAD LOAD + ROOF LIVE LOAD	1.25
DEAD LOAD + WIND LOAD	1.60

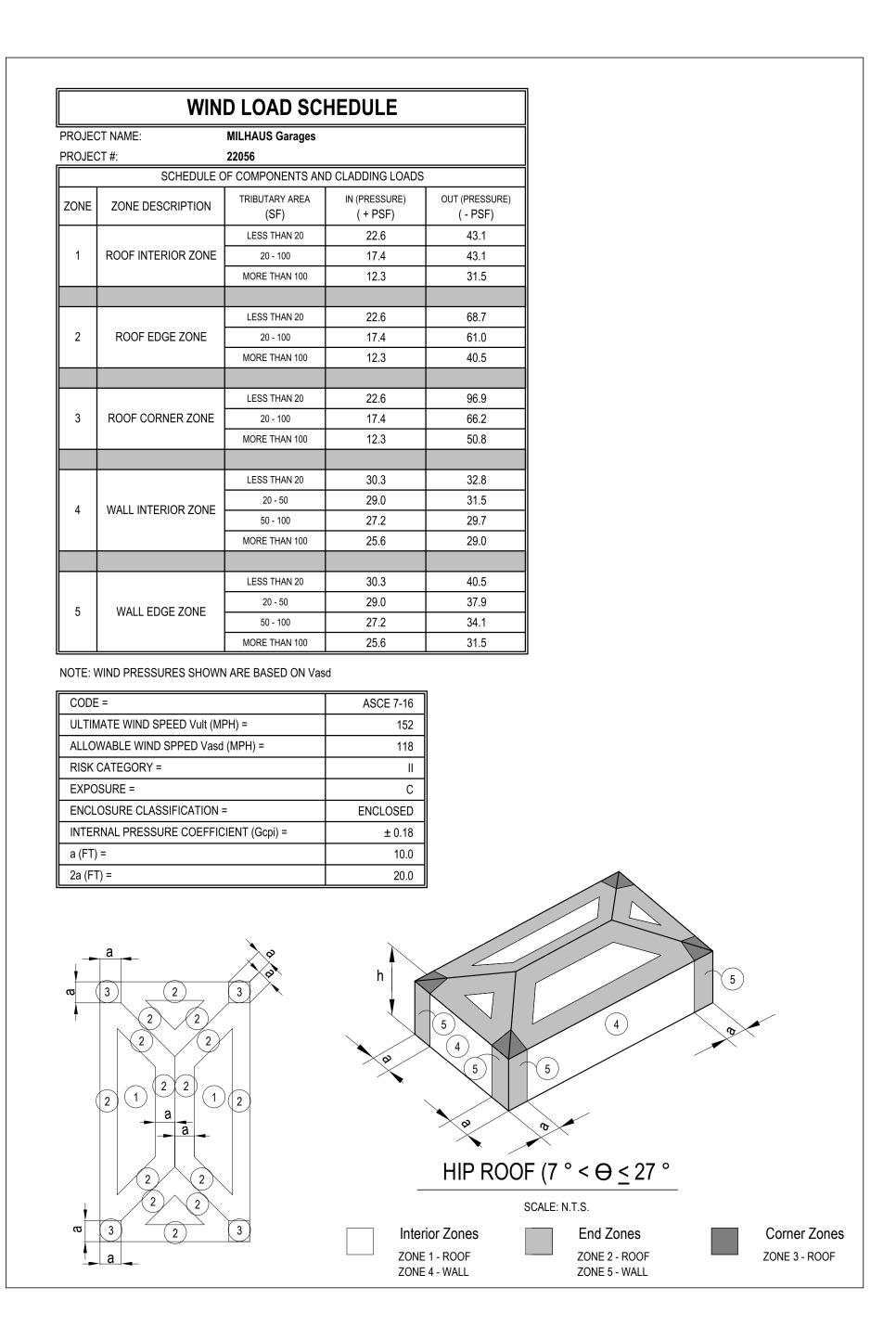
ALL TRUSSES SHALL BE DESIGNED FOR A MAXIMUM DEFLECTION OF L/360 FOR LIVE LOAD AND L/240, NOT TO EXCEED 1", FOR TOTAL LOAD; THE MAXIMUM DEFLECTION DUE TO TOTAL LOAD OF 1" IS INCLUSIVE OF ALL DEAD LOAD, SELF-WEIGHT, SUPERIMPOSED DEAD LOAD, AND LIVE LOAD, INCLUDING CREEP.

PLYWOOD ROOF, FLOOR, AND WALL SHEATHING ARE DESIGNED AS DIAPHRAGMS/SHEAR WALLS AND SHALL COMPLY WITH APPLICABLE PROVISIONS OF CHAPTER 23 OF THE BUILDING CODE AND SHALL BE FASTENED IN ACCORDANCE WITH THE MINIMUM REQUIREMENTS OF BUILDING CODE TABLES, UNLESS SHOWN OTHERWISE. PLYWOOD SHALL BE INSTALLED WITH THE STRENGTH AXIS OF EACH PANEL PERPENDICULAR TO THE SUPPORTS IN ALL CASES. PLYWOOD ROOF PANELS SHALL BE INSTALLED AS SHOWN IN CASES 1 THROUGH 4 IN TABLE 2306.2.1 (CONT.). BLOCKING SHALL BE PROVIDED BETWEEN ALL WOOD ROOF FRAMING MEMBERS AT ALL RIDGES AND VALLEYS FOR FULL PLYWOOD EDGE SUPPORT. AT ROOF VENT LOCATIONS, PROVIDE 2X4 BLOCKING, ON THE FLAT, ON ALTERNATING SIDES OF THE VENT BETWEEN ROOF FRAMING MEMBERS.

ALL WOOD SHEAR WALLS SHALL HAVE ALL PLYWOOD EDGES FULLY BLOCKED WITH THE SAME STUD SIZE AS THE WALLS, WITH THE BLOCKING INSTALLED SO THAT THE PLYWOOD IS NAILED INTO THE NARROW STUD FACE. FASTEN THE SHEATHING WITH THE SAME NAIL SIZE AND SPACING TO EACH PLY OF DOUBLE TOP AND BOTTOM WALL PLATES, AS APPLICABLE. FASTEN THE SHEATHING WITH THE SAME NAIL SIZE AND SPACING TO MULTI-PLY COLUMNS AT ENDS OF WALLS; WHERE SOLID COLUMNS ARE USED AT ENDS OF WALLS, FASTEN THE SHEATHING WITH THE SAME NAIL SIZE AND SPACING IN VERTICAL ROWS WITH 1-1/2" ROW SPACING FOR FULL HEIGHT OF COLUMN. WHERE SHEAR WALL PANEL EDGE NAILING IS 3" OR LESS, THE BLOCKING AT PANEL EDGES SHALL BE 3" NOMINAL OR GREATER AND THE NAILING SHALL BE STAGGERED. MULTIPLE PLY STUDS MAY BE USED AS THE PANEL EDGE BLOCKING IN LIEU OF 3" NOMINAL BLOCKING; FASTEN THE PLIES WITH NAILS HAVING THE SAME LENGTH AS THE TOTAL BLOCKING THICKNESS WITH SPACING TO MATCH THE PLYWOOD PANEL EDGE NAILING, STAGGERED. PANELS SHALL NOT BE LESS THAN 4FTX8FT, EXCEPT AT BOUNDARIES AND CHANGES IN FRAMING.

WOOD FRAMING CONNECTORS:

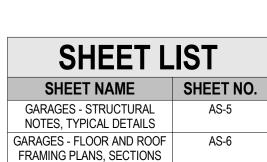
ALL CONNECTORS SHALL BE GALVANIZED. CONNECTOR MODEL NUMBERS SHOWN ARE STRONG-TIE CONNECTORS AS MANUFACTURED BY SIMPSON STRONG-TIE CO., 5956 W. LAS POSITAS BLVD., P.O. BOX 10789, PLEASANTON, CA 94588, 800-999-5099, WWW.STRONGTIE.COM. SUBSTITUTIONS ARE ACCEPTABLE WITH THE APPROVAL OF THE SEOR. UNLESS SHOWN OTHERWISE, INSTALL THE *LARGEST* FASTENER SIZE AND *MAXIMUM* NUMBER OF FASTENERS SHOWN IN LATEST SIMPSON CATALOG. WHERE SDS SCREWS ARE SPECIFIED IN THE SIMPSON CATALOG, SDS SCREWS MUST BE USED UNLESS EXPRESSLY SHOWN IN THE DRAWINGS OTHERWISE. ALL ROOF AND UPPER-LEVEL UPLIFT CONNECTORS SHALL BE LOCATED ON THE SAME SIDE OF THE WALL AS THE EXTERIOR SHEATHING

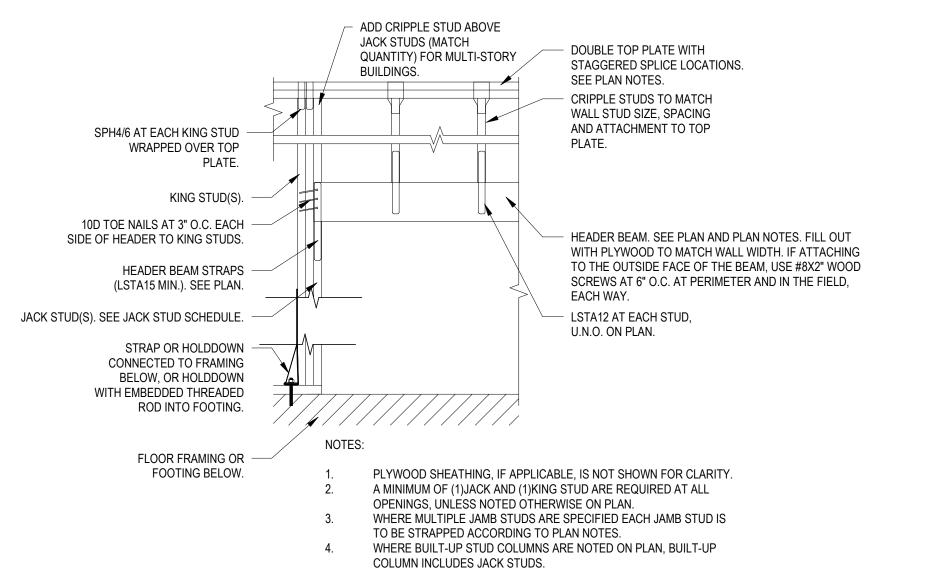




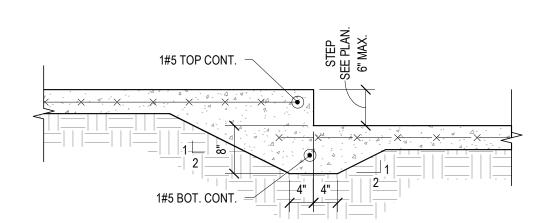
ABBREVIATIONS

DESCRIPTION

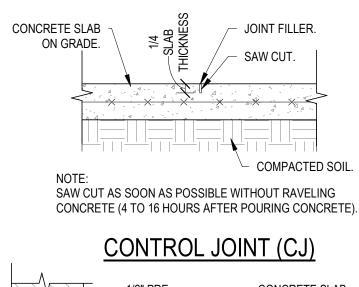


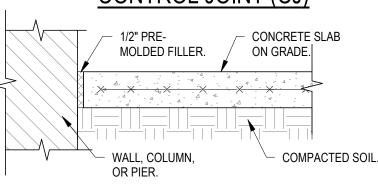


TYPICAL WOOD WALL OPENING DETAIL SCALE: NTS

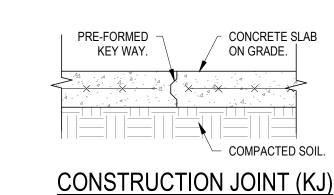


SLAB ON GRADE STEP - 6" OR LESS





ISOLATION JOINT (IJ)



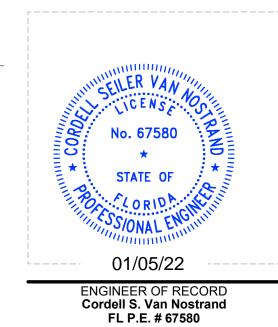
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ORLANDO, FLORIDA 32801 407 926 3000 INFO@BAKERBARRIOS.COM BAKERBARRIOS.COM AA0002981 | LC26000427

189 S. ORANGE AVE., SUITE 1700



PERMIT SET

SUBMISSION

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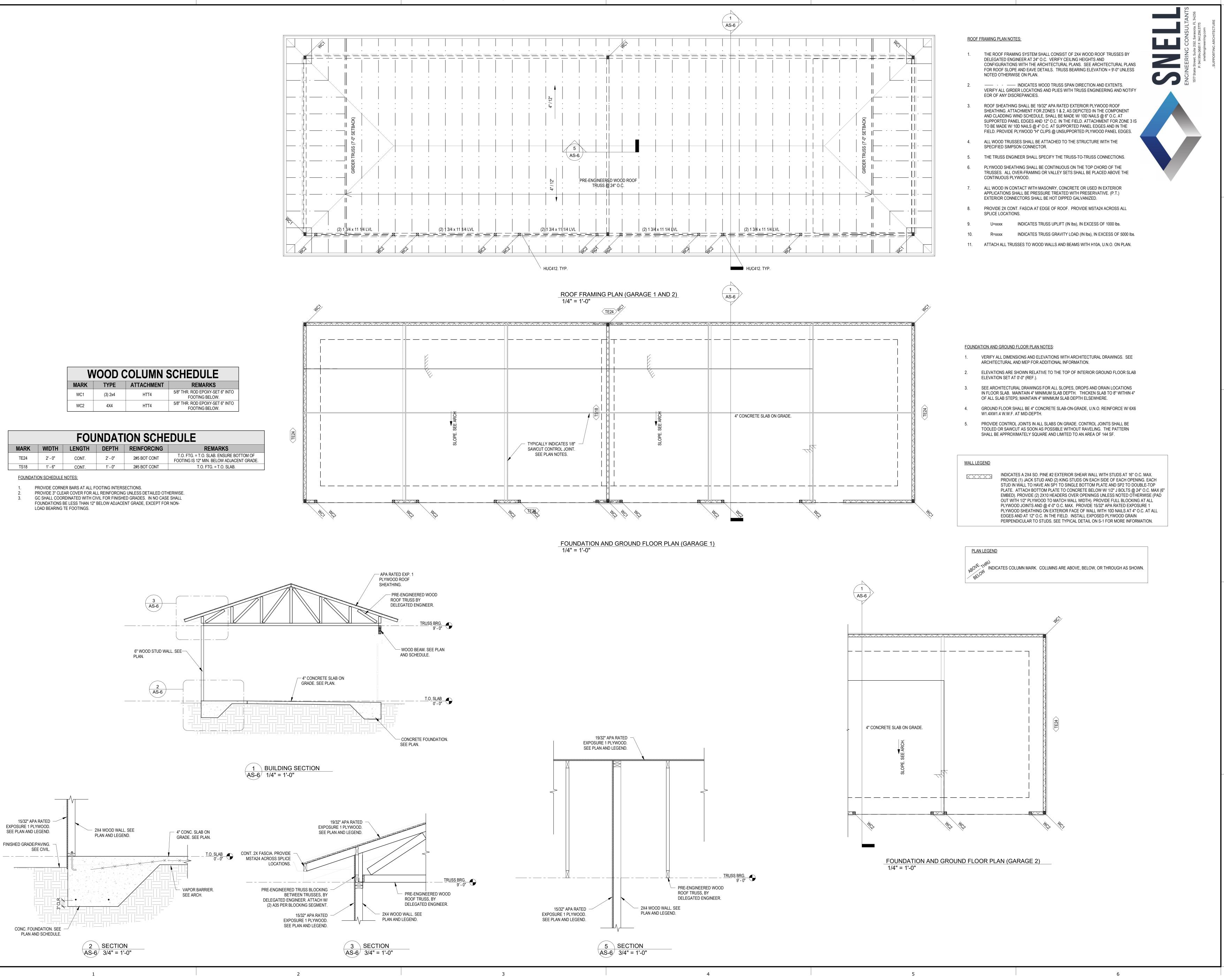
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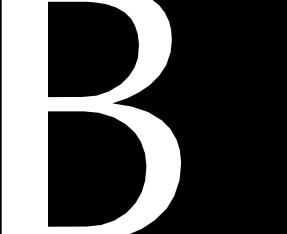
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PROJECT NO:
220035.00

GARAGES -STRUCTURAL NOTES, TYPICAL DETAILS





Baker Barrios

ORLANDO 189 S. ORANGE AVE., SUITE 1700 ORLANDO, FLORIDA 32801 407 926 3000

INFO@BAKERBARRIOS.COM BAKERBARRIOS.COM AA0002981 | LC26000427

No. 67580

*
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ENGINEER OF RECORD Cordell S. Van Nostrand FL P.E. # 67580

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GARAGES - FLOOR AND ROOF FRAMING PLANS, SECTIONS

STRUCTURAL NOTES

GENERAL NOTES:

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

ALL DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK. DO NOT SCALE DRAWINGS.

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS.

ELECTRONIC VERSIONS OF THE STRUCTURAL DRAWINGS ARE THE SOLE, COPYRIGHTED PROPERTY OF SNELL ENGINEERING AND ARE NOT TO BE USED OR TRANSFERRED WITHOUT THE EXPRESS, WRITTEN PERMISSION OF SNELL ENGINEERING.

THE STRUCTURAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE -BUILDING 7TH EDITION (2020). THE FOLLOWING SUPERIMPOSED LOADINGS HAVE BEEN UTILIZED:

ASCE 7-16 ULTIMATE WIND SPEED 152 MPH - 118 MPH ALLOWABLE WIND SPEED EXPOSURE C OPEN STRUCTURE INTERNAL PRESSURE COEFFICIENT - +/- 0.18 RISK FACTOR II SEE WIND SCHEDULE FOR PRESSURES

FEMA A/V ZONE FLOOD DESIGN CLASS BFE ELEVATION 11.00 NAVD FREEBOARD PROPOSED LOWEST FLOOR ELEVATION 12.00 NAVD BOT. OF LOWEST HORIZ. STRUCT. MEMBER 12.00 NAVD REQ'D MIN. DRY FLOOD-PROOF ELEVATION 12.00 NAVD

STRUCTURAL SYSTEMS OF BUILDINGS AND STRUCTURES ARE DESIGNED, CONNECTED AND ANCHORED TO RESIST FLOTATION, COLLAPSE OR PERMANENT LATERAL MOVEMENT DUE TO STRUCTURAL LOADS AND STRESSES FROM FLOODING EQUAL TO THE DESIGN FLOOD ELEVATION IN CONFORMANCE WITH ASCE 24-14 AND FBC-R 322.1.2/FBC 1612.1.

RISK CATEGORY SEISMIC IMPORTANCE FACTOR le SITE CLASS D SEISMIC DESIGN CATEGORY D Ss - 1.094 g Sds - 0.775 g Sd1 - 0.395 g

IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO CONFIRM ALL FINAL WEIGHTS OF EQUIPMENT (MECHANICAL, CHILLED WATER, KITCHEN, SPA/POOL, ETC.), CLADDING, FINISHES, ETC. PRIOR TO ERECTING STRUCTURE SUPPORTING THESE ITEMS. ALL WEIGHTS ACCOUNTED FOR IN THE STRUCTURAL DESIGN ARE LISTED ABOVE OR ON THE PLANS.

SHOP DRAWING REVIEW: SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC.

ANY COMPONENT NOTED AS "DELEGATED" SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF FLORIDA AND NOT BY THE SER. SIGNED AND SEALED DRAWINGS SHALL BE PROVIDED TO THE ARCHITECT AND SEOR FOR REVIEW AS A SHOP DRAWING; CALCULATIONS WILL BE PROVIDED IF REQUESTED.

ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW NOTATION WILL BE RETURNED UNCHECKED. EVERY EFFORT WILL BE MADE TO RETURN THE SHOP DRAWINGS WITHIN TEN BUSINESS DAYS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO SUBMIT THE SHOP DRAWINGS ALLOWING FOR AN ADEQUATE REVIEW PERIOD.

ONE SET OF PRINTS WILL BE RETAINED BY THE ENGINEER AND ONE BY THE ARCHITECT, THE CONTRACTOR SHALL RECEIVE THE REMAINING PRINTS FOR SUBMITTAL TO THE BUILDING DEPARTMENT AND AS REQUIRED FOR DISTRIBUTION.

IN ALL INSTANCES THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN A REQUEST FOR INFORMATION (RFI) OR SIMILAR DOCUMENTATION BY THE ENGINEER. EVERY EFFORT WILL BE MADE TO RETURN THE RFIS WITHIN TWO BUSINESS DAYS. SUBMITTALS SHALL CLEARLY IDENTIFY THE SPECIFIC PROJECT, APPLICABLE CODES AND DESIGN CRITERIA, AND DETAILING OF ALL COMPONENTS NECESSARY TO ENSURE PROPER INSTALLATION OF THE COMPONENTS

SHOP DRAWINGS SHOULD BE SUBMITTED FOR ALL COMPONENTS OF THE STRUCTURAL FRAMING SYSTEM, AS REQUIRED BY THE ARCHITECT, AND AS NOTED ELSEWHERE IN THESE NOTES, INCLUDING, BUT NOT LIMITED TO: CONCRETE MIX DESIGNS CONCRETE REINFORCEMENT

MASONRY BLOCK MASONRY BLOCK ACCESSORIES

MASONRY REINFORCING ANY PROPOSED MANUFACTURER CHANGE FROM THE BASIS OF DESIGN

IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE ALL TRADES AND CONSULTANTS, CROSS REFERENCE THEIR DRAWINGS WITH THE OVERALL DESIGN, AND PROVIDE TO EACH A COMPLETE SET OF DRAWINGS AND SUBMITTALS TO ENSURE COMPATIBILITY OF CONSTRUCTION PER DESIGN INTENT.

ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2,000 PSF ON COMPACTED FILL OR COMPACTED NATIVE SOIL. BEFORE CONSTRUCTION COMMENCES AND IF REQUIRED BY THE JURISDICTION, SOIL BEARING CAPACITY SHALL BE VERIFIED BY A SUBSURFACE INVESTIGATION, AS WELL AS FIELD AND LABORATORY TESTS PERFORMED BY A CERTIFIED TESTING LABORATORY, WHOSE REPORT SHALL INCLUDE ANALYSIS AND RECOMMENDATIONS FOR SITE PREPARATION IN ORDER TO BEAR THE FOUNDATION LOADS. ABOVE REPORT SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW BEFORE FOUNDATION CONSTRUCTION BEGINS.

PENETRATIONS:

NO PENETRATIONS SHALL BE MADE IN ANY STRUCTURAL MEMBERS WITHOUT PREVIOUS APPROVAL OF THE EOR. EXCEPT THOSE PENETRATIONS SHOWN ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHOULD SUBMIT DRAWINGS SHOWING: ANY LOCATION WHERE OPENINGS, PENETRATIONS, OR ANY PLACE MORE THAN 3 PIPES OR CONDUITS ARE LOCATED IN A SLAB SYSTEM; ANY LOCATION IN A BEAM OR COLUMN WHERE MORE THAN 4% OF THE MEMBER SECTION IS REMOVED IN ANY

PLUMBING SLEEVES

MINIMUM SLEEVE SPACING SHALL BE THREE DIAMETERS CENTER TO CENTER OF THE LARGER SLEEVE OR 6" CLEAR BETWEEN SLEEVES, WHICHEVER IS GREATER. PRIOR TO CONSTRUCTION SLEEVE LOCATIONS AND SIZES SHALL BE APPROVED BY THE ENGINEER. CONDUITS, PIPES AND SLEEVES SHALL BE PLACED AND SPACED IN ACCORDANCE WITH ACI 318 6.3.

REINFORCING STEEL:

SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. SECURE APPROVAL OF SHOP DRAWINGS PRIOR TO COMMENCING FABRICATION.

REINFORCING BARS SHOWN IN SECTIONS DEPICT TYPICAL CONFIGURATION AND ARE NOT SPECIFIC TO THE CONCRETE MEMBER CUT ON PLAN; SEE PLAN AND SCHEDULES FOR ALL BAR SIZE AND QUANTITIES. TAKE-OFFS AND QUANTITIES SHALL BE OBTAINED FROM THE SCHEDULES AND PLANS, NOT FROM SECTIONS. WHERE HOOKS, LAP LENGTHS, ETC. ARE SHOWN IN SECTIONS, ALL LENGTHS AND DETAILS SHALL MEET ACI REQUIREMENTS FOR REINFORCING DETAILS. HOOKS SHALL BE ORIENTED AS REQUIRED TO FIT WITHIN THE CONCRETE MEMBER.

WELDED WIRE FABRIC

TO CONFORM TO ASTM A-185, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. MINIMUM LAP SHALL BE ONE SPACE PLUS TWO INCHES.

MASONRY WALLS:

MASONRY UNITS SHALL MEET ASTM C-90 FOR HOLLOW LOAD BEARING TYPE MASONRY WITH UNIT STRENGTH OF 2000 PSI ON THE NET AREA (F'M = 2000 PSI). MORTAR SHALL BE TYPE "M" OR "S" AND MEET ASTM C-270. GROUT SHALL BE 3000 PSI MINIMUM COMPRESSIVE STRENGTH AND MEET ASTM C-476. PROVIDE HOOKED DOWELS IN FOOTINGS FOR ALL VERTICAL REINFORCING ABOVE. LAP SPLICES 48 BAR DIAMETERS.

MASONRY CONSTRUCTION SHALL BE LAID IN RUNNING BOND CONFIGURATION UNLESS OTHERWISE NOTED.

BLOCK CELLS AS SHOWN ON PLANS SHALL BE GROUT FILLED WITH VERTICAL REINFORCING BARS. SEE PLAN NOTES FOR BAR SIZE AND SPACING. DOWELS SHALL BE USED TO PROVIDE CONTINUITY INTO THE STRUCTURE ABOVE AND/OR BELOW, UNLESS NOTED OTHERWISE. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFINE CONCRETE AND GROUT TO AREA REQUIRED. CELLS TO BE GROUT FILLED SHALL HAVE VERTICAL ALIGNMENT SUFFICIENT TO MAINTAIN A CLEAR, UNOBSTRUCTED, CONTINUOUS VERTICAL GROUT SPACE

PROVIDE 9 GAUGE GALVANIZED HORIZONTAL JOINT REINFORCING (DUR-O-WALL OR SEOR APPROVED SUBSTITUTION) AT ALTERNATE BLOCK COURSES, BEGINNING 8" ABOVE FOOTINGS AND FLOOR LEVELS. MASONRY WALLS ABOVE OPENINGS SHALL BE REINFORCED AT THE SAME SPACING AS THE WALL WITH DOWELS HOOKED INTO THE BEAM OR LINTEL ABOVE THE OPENING

GROUT LIFT: AN INCREMENT OF GROUT HEIGHT WITHIN A TOTAL GROUT POUR. GROUT POUR: THE TOTAL HEIGHT OF MASONRY TO BE GROUTED PRIOR TO ERECTION OF ADDITIONAL MASONRY. A GROUT POUR CONSISTS OF ONE OR MORE GROUT LIFTS. GROUT POURS SHALL SET FOR A MINIMUM OF 4 HOURS BEFORE ANY ADDITIONAL GROUT PLACEMENT.

GROUT SHALL HAVE A SLUMP BETWEEN 8 AND 11 INCHES, EXCEPT SELF-CONSOLIDATING GROUT. JOB-SITE PROPORTIONING OF SELF-CONSOLIDATING GROUT IS NOT PERMITTED.

MASONRY GROUTING REQUIREMENTS:

FIELD-MIXED GROUT SHALL BE PLACED WITHIN 1-1/2 HOURS FROM INTRODUCING WATER INTO THE MIXTURE AND BEFORE INITIAL SET.

GROUT SLUMP REQUIREMENTS: FOR GROUT SLUMP BETWEEN 8 AND 10 INCHES. THE MAXIMUM GROUT LIFT HEIGHT IS 5 FEET.

FOR GROUT SLUMP BETWEEN 10 AND 11 INCHES, THE MAXIMUM GROUT LIFT HEIGHT IS 12.67 FEET. FOR SELF-CONSOLIDATING GROUT, THE GROUT LIFT HEIGHT SHALL NOT EXCEED THE GROUT POUR HEIGHT (24

GROUT LIFT HEIGHTS EXCEEDING 5 FEET SHALL MEET THE FOLLOWING REQUIREMENTS: MASONRY MORTAR HAS CURED FOR AT LEAST 4 HOURS.

GROUT SLUMP IS BETWEEN 10 AND 11 INCHES. NO INTERMEDIATE BOND BEAMS ARE PLACED BETWEEN THE TOP AND BOTTOM OF THE GROUT LIFT HEIGHT.

EACH GROUT LIFT SHALL BE CONSOLIDATED BY MECHANICAL VIBRATION AT THE TIME OF PLACEMENT. CONSOLIDATION IS NOT REQUIRED FOR SELF-CONSOLIDATING GROUT. EACH GROUT LIFT SHALL BE RECONSOLIDATED BY MECHANICAL VIBRATION AFTER INITIAL WATER LOSS AND SETTLEMENT HAS OCCURRED, AND BEFORE ADDING THE SUBSEQUENT GROUT LIFT. RECONSOLIDATION IS NOT REQUIRED FOR SELF-CONSOLIDATING GROUT.

THE TIME BETWEEN PLACING GROUT LIFTS SHALL NOT EXCEED 1 HOUR. THE MAXIMUM POUR HEIGHT IS 24 FEET.

A GROUT KEY SHALL BE PROVIDED AT THE TOP OF EACH GROUT LIFT AND GROUT POUR. GROUT KEYS SHOULD BE FORMED BY TERMINATING THE GROUT 1-1/2 INCHES BELOW A MORTAR JOINT.

AT SILLS OF MASONRY OPENINGS IN LOAD-BEARING MASONRY WALLS, PROVIDE AN 8" KNOCKOUT COURSE, GROUTED SOLID AND REINFORCED WITH 1 #5 CONTINUOUS HORIZONTAL BAR, TYPICAL UNLESS NOTED OTHERWISE ON PLAN OR DETAILS.

TIE BEAMS:

CONCRETE:

BEAMS WITH THE PREFIX "TB" SHALL BE OF CONCRETE POURED AFTER THE BLOCK WALLS BELOW ARE IN PLACE. REINFORCING SHALL BE CONTINUOUS THROUGH TIE BEAMS WITH MINIMUM LAP SPLICES OF 48 BAR DIAMETERS AND BENT BARS AT CORNERS. USE METAL LATH, MORTAR, OR SPECIAL UNITS TO CONFINE CONCRETE TO AREA REQUIRED, IN ACCORDANCE WITH ACI 530.1, SECTION 4.3.3.3 (SOLID METAL OR FELT CAVITY CAPS ARE PROHIBITED). WHERE TIE BEAMS ARE NOTED AS KNOCK-OUT (K.O.) BLOCKS, KNOCK-OUT BLOCKS SHALL BE GROUT-FILLED AS SPECIFIED IN THE MASONRY SECTION ABOVE.

UNLESS NOTED ON PLAN OR SCHEDULED, AN 8"X16" CONCRETE TIE BEAM SHALL BE PROVIDED AT THE TOP OF ALL MASONRY WALLS. TOP OF TIE BEAM SHALL BE TOP OF WALL ELEVATION AND REINFORCED WITH (2)#5 BARS TOP AND BOTTOM, WITH #3 STIRRUPS AT 24" O.C. MAX.

ALL CONCRETE SHALL MEET ACI 318 'BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE' AND ACI 301 'SPECIFICATIONS FOR STRUCTURAL CONCRETE', INCLUDING, BUT NOT LIMITED TO, MIX DESIGN, STRENGTH, COVER, PLACEMENT, CURING, TESTING, FORMS, FLATNESS, ETC. SEE THE ARCHITECTURAL SPECIFICATIONS AND PLANS FOR ANY ADDITIONAL REQUIREMENTS. ALL CONCRETE SHALL BE AN APPROVED MIX DESIGN PROPORTIONED TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW

3000 PSI FOR FOUNDATIONS AND SLABS ON GRADE

4000 PSI FOR ALL OTHER STRUCTURAL CONCRETE

WITH A PLASTIC AND WORKABLE MIX:

SUBMIT PROPOSED MIX DESIGN FOR EACH PORTION OF WORK PRIOR TO USE. ALL MIX DESIGNS SHALL BE ACCOMPANIED BY A MINIMUM OF 15 FIELD STRENGTH TEST RECORDS. AS NOTED IN ACI 301 4.2.3.2(A): NO OTHER DOCUMENTATION OF AVERAGE COMPRESSIVE STRENGTH WILL BE ACCEPTED WITHOUT PRIOR WRITTEN APPROVAL BY THE SER. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. MIX SHALL MEET THE REQUIREMENTS OF ASTM C33 FOR COARSE AGGREGATE. FOR ALL FLATWORK, AT LEAST 75% OF LARGE AGGREGATE SHALL CONSIST OF #57 STONE. CONCRETE SHALL COMPLY WITH ALL THE REQUIREMENTS OF ASTM STANDARD C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED.

ALL CONCRETE MIX DESIGNS SHALL INCLUDE A WRITTEN DESCRIPTION INDICATING WHERE EACH PARTICULAR MIX IS TO BE PLACED WITHIN THE STRUCTURE.

CONCRETE SHALL BE PLACED AND CURED ACCORDING TO ALL STANDARDS AND SPECIFICATIONS, INCLUDING ALL ACI REQUIREMENTS.

THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1-1/2) HOURS. IF FOR ANY REASON THERE IS A LONGER DELAY THAN THAT STATED ABOVE. THE ONCRETE SHALL BE DISCARDED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LAB TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE. ALL SLABS SHALL BE CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1-D AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE. ALL SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY. CALCIUM CHLORIDES SHALL NOT BE UTILIZED; OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER.

THE GENERAL CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD A MINIMUM OF 48 HOURS PRIOR PLACEMENT OF ANY STRUCTURAL CONCRETE.

UNLESS NOTED OTHERWISE ON PLANS, THE FOLLOWING CONCRETE CLEAR COVER SHALL BE PROVIDED FOR ALL NON-

PRESTRESSED CONCRETE REINFORCEMENT PER ACI 318:			
CONCRETE CAST AGAINST EARTH:	ALL BARS	-	3"
CONCRETE EXPOSED TO EARTH (FORMED FACE):	ALL BARS	-	2"
CONCRETE EXPOSED TO WEATHER:	#6 BARS AND GREATER	-	2"
	#5 BARS AND SMALLER	-	1 1/2"
WHERE NOT EXPOSED TO EARTH OR WEATHER:			
SLABS, WALLS, AND JOISTS:	#14 & #18 BARS	-	1 1/2"
	#11 BARS AND SMALLER	-	3/4"
BEAMS AND COLUMNS:	ALL BARS	-	1 1/2"

A) ASTM C143 - "STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE." MAXIMUM SLUMP SHALL BE 4-6 INCHES, PRIOR TO ADDING A SUPER PLASTICIZER. B) ASTM C39 - "STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS." A SEPARATE TEST SHALL BE CONDUCTED FOR EACH CLASS, FOR EVERY 50 CUBIC YARDS (OR FRACTION THEREOF), PLACED PER DAY. REQUIRED CYLINDER(S) QUANTITIES AND TEST AGE AS FOLLOWS: 1 AT 7 DAYS 2 AT 28 DAYS

ONE ADDITIONAL RESERVE CYLINDER TO BE TESTED UNDER THE DIRECTION OF THE ENGINEER, IF REQUIRED. IF 28 DAY STRENGTH IS ACHIEVED, THE ADDITIONAL CYLINDER(S) MAY BE DISCARDED.

POUR STRUCTURAL CONCRETE WITHIN THE FOLLOWING TOLEF	RANCES:
VARIATION FROM PLUMB:	1/4" IN 10'-0"
VARIATION FROM LEVEL IN TOPS OF PILASTERS:	1/8" IN 10'-0"
VARIATION FOOTINGS:	
PLAN DIMENSIONS:	+2", - 1/2"
THICKNESS:	- 0"

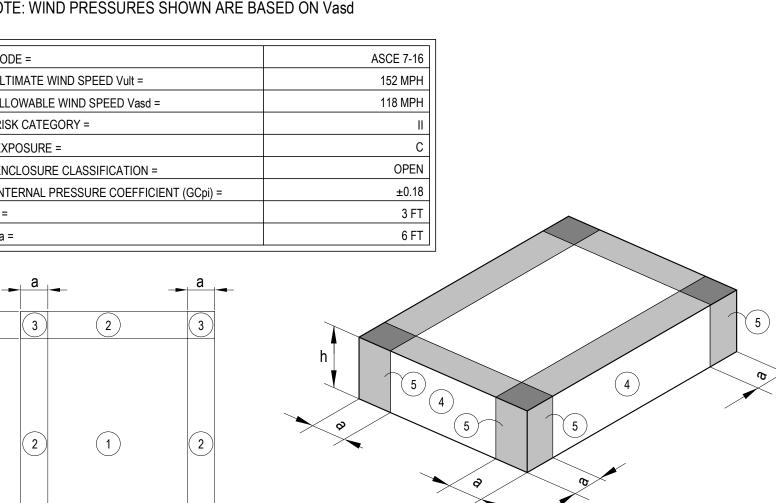
CHEMICAL ANCHORS:

SHALL BE AN EQUAL TWO-PART EPOXY POLYMER INJECTION SYSTEM, SUCH AS SIMPSON SET-XP "STRUCTURAL ANCHORING ADHESIVE", HILTI HIT-HY 200 OR ENGINEER APPROVED SUBSTITUTION, INSTALLED IN ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS. INSTALLERS SHALL BE TRAINED BY THE MANUFACTURER'S REPRESENTATIVE. BRUSH AND BLOW OUT ALL HOLES.

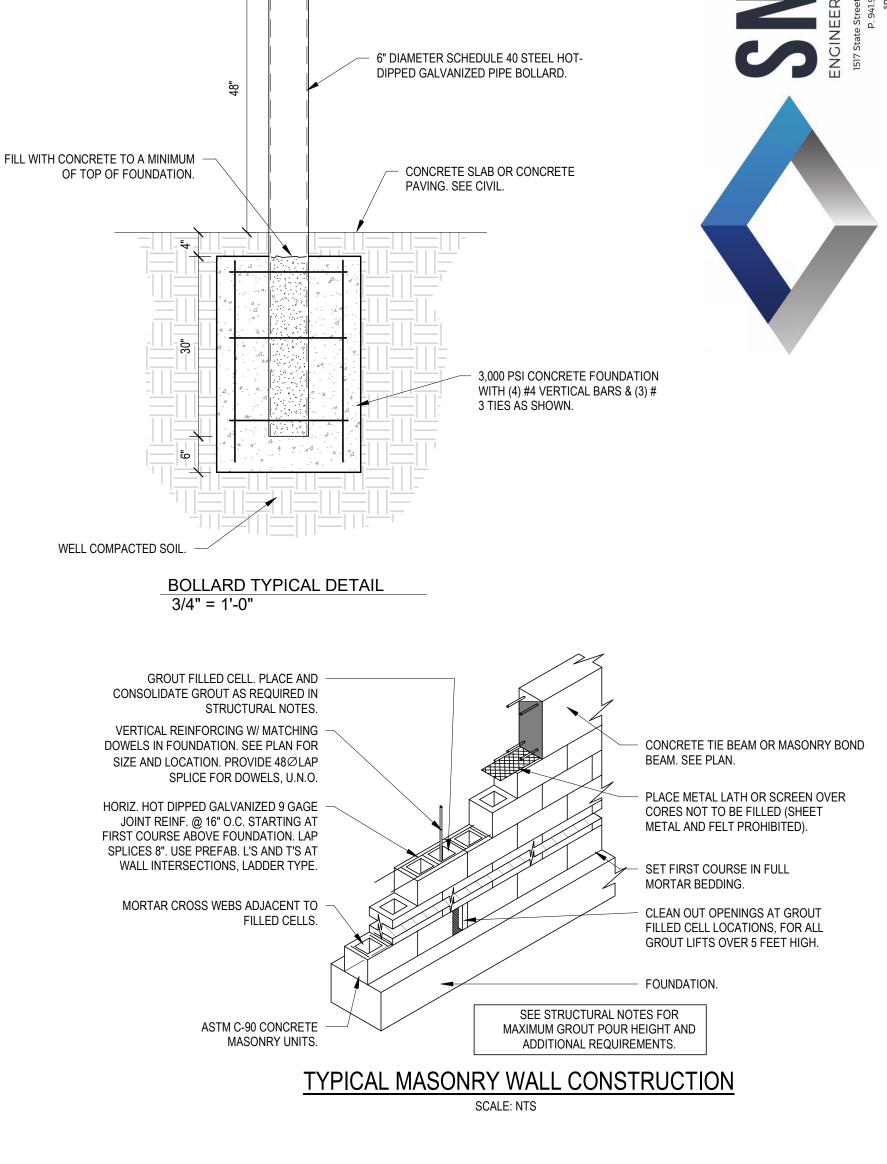
POUND(S) ARCHITECT B OR BOT BOTTOM B/ OR B.O. BOTTOM OF BASE PLATE CANTILEVER CONTROL JOIN CLEAR CONTINUOUS DIAMETER **EACH** EACH END EACH FACE **EXPANSION J** ELEVATION **EMBEDMENT** EDGE OF SLAE **EQUAL** FACH WAY **EXISTING** 28 DAY COMPF FLOOR DRAIN FINISHED FLOO FIRE RETARDA FOOTING GAUGE GALVANIZED GENERAL COI H OR HORIZ HORIZONTAL HOLLOW STRU CONSTRUCTIO LONG LEG HOR

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28 DAY COMPRESSIVE STRENGTH FLOOR DRAIN FINISHED FLOOR ELEVATION FIRE RETARDANT TREATED FOOTING GAUGE GALVANIZED GENERAL CONTRACTOR HORIZONTAL HOLLOW STRUCTURAL STEEL CONSTRUCTION JOINT SW SHORT WAY/ SHEAR WALL T TOP T.O. OR T/ TOP OF TEMP TEMPERATURE THRU THROUGH TRANS TRANSVERSE TYP TYPICAL V OR VERT W/ WITH WOOD	EACH WAY	SS	STAINLESS STEEL
FLOOR DRAIN FINISHED FLOOR ELEVATION FIRE RETARDANT TREATED FOOTING GAUGE GALVANIZED GENERAL CONTRACTOR HORIZONTAL HOLLOW STRUCTURAL STEEL CONSTRUCTION JOINT TOP T.O. OR T/ TOP TANPERATURE THRU THROUGH TRANS TRANSVERSE TYP TYPICAL V OR VERT VERTICAL W/ WITH WOOD	EXISTING	STD	STANDARD
FINISHED FLOOR ELEVATION FIRE RETARDANT TREATED FOOTING GAUGE GALVANIZED GENERAL CONTRACTOR HORIZONTAL HOLLOW STRUCTURAL STEEL CONSTRUCTION JOINT T.O. OR T/ TOP OF TEMP TEMPERATURE THRU THROUGH TRANS TRANSVERSE TYP TYPICAL V OR VERT W/ WITH WOOD	28 DAY COMPRESSIVE STRENGTH	SW	SHORT WAY/ SHEAR WALL
FIRE RETARDANT TREATED FOOTING GAUGE GALVANIZED GENERAL CONTRACTOR HORIZONTAL HOLLOW STRUCTURAL STEEL CONSTRUCTION JOINT TEMP TEMPERATURE THR THREADED THRU THROUGH TRANS TRANSVERSE TYP TYPICAL V OR VERT WITH WOOD	FLOOR DRAIN	Т	TOP
FOOTING GAUGE GALVANIZED GENERAL CONTRACTOR HORIZONTAL HOLLOW STRUCTURAL STEEL CONSTRUCTION JOINT THR THREADED THRU THROUGH TRANS TRANSVERSE TYP TYPICAL V OR VERT WITH WOOD	FINISHED FLOOR ELEVATION	T.O. OR T/	TOP OF
GAUGE GALVANIZED GENERAL CONTRACTOR HORIZONTAL HOLLOW STRUCTURAL STEEL CONSTRUCTION JOINT THRU THROUGH TRANS TRANSVERSE TYP TYPICAL VOR VERT VERTICAL W/ WITH WOOD	FIRE RETARDANT TREATED	TEMP	TEMPERATURE
GALVANIZED GENERAL CONTRACTOR HORIZONTAL HOLLOW STRUCTURAL STEEL CONSTRUCTION JOINT TRANS TRANSVERSE TYP TYPICAL V OR VERT VERTICAL W/ WITH WOOD	FOOTING	THR	THREADED
GENERAL CONTRACTOR HORIZONTAL HOLLOW STRUCTURAL STEEL CONSTRUCTION JOINT TYP TYPICAL V OR VERT VERTICAL W/ WITH WOOD	GAUGE	THRU	THROUGH
HORIZONTAL HOLLOW STRUCTURAL STEEL CONSTRUCTION JOINT V OR VERT W// WITH WOOD	GALVANIZED	TRANS	TRANSVERSE
HOLLOW STRUCTURAL STEEL W/ WITH CONSTRUCTION JOINT WD WOOD	GENERAL CONTRACTOR	TYP	TYPICAL
CONSTRUCTION JOINT WD WOOD	HORIZONTAL	V OR VERT	VERTICAL
	HOLLOW STRUCTURAL STEEL	W/	WITH
LONG LECTIODIZONTAL MODIZING DOINT	CONSTRUCTION JOINT	WD	WOOD
	LONG LEG HORIZONTAL	WP	WORKING POINT
LONG LEG VERTICAL WWF WELDED WIRE FABRIC	LONG LEG VERTICAL	WWF	WELDED WIRE FABRIC
LONG WAY	LONG WAY		

WIND LOAD SCHEDULE SCHEDULE OF COMPONENTS AND CLADDING LOADS TRIBUTARY AREA IN (PRESSURE) OUT (PRESSURE) ZONE ZONE DESCRIPTION **ROOF INTERIOR ZONE** MORE THAN 100 **ROOF EDGE ZONE** MORE THAN 100 LESS THAN 20 **ROOF CORNER ZONE** MORE THAN 100 LESS THAN 20 WALL INTERIOR ZONE MORE THAN 100 WALL EDGE ZONE MORE THAN 100 NOTE: WIND PRESSURES SHOWN ARE BASED ON Vasd

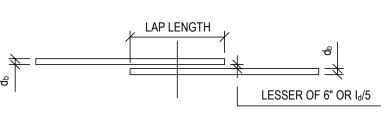


CODE =	ASCE 7-16
ULTIMATE WIND SPEED Vult =	152 MPH
ALLOWABLE WIND SPEED Vasd =	118 MPH
RISK CATEGORY =	ш
EXPOSURE =	С
ENCLOSURE CLASSIFICATION =	OPEN
INTERNAL PRESSURE COEFFICIENT (GCpi) =	±0.18
a =	3 FT
2a =	6 FT
2 1 2	h 5 4 5 5 5
80 3 2 3	FLAT ROOF (Θ = 0 °) SCALE: N.T.S.
	Interior Zones ZONE 1 - ROOF ZONE 4 - WALL End Zones ZONE 2 - ROOF ZONE 5 - WALL Corner Zones ZONE 3 - ROOF



DECORATIVE BOLLARD COVER. SEE ARCH.

	TEI	NSION (CLASS 'B	') LAP SPLICE LE	COMPRESSION LAP / SPLICE LENGTH	
BAR SIZE	TOP BARS		OTHER BARS		ALL BARS
	3000 PSI	4000 PSI	3000 PSI	4000 PSI	ALL CONCRETE WITH f ² 33000 psi
#3	28	24	22	19	12
#4	37	32	29	25	15
#5	47	40	36	31	19
#6	56	48	43	37	23
#7	81	70	63	54	27
#8	93	80	72	62	30
#9	105	91	81	70	34
#10	118	102	91	79	38
#11	131	113	101	87	43
#14	121	105	93	81	-
#18	161	139	124 107		-



NOTES: REFER TO "HOOKED REINFORCEMENT TENSION DEVELOPMENT LENGTH SCHEDULE" WHEN THE STRAIGHT DEVELOPMENT LENGTH IN TENSION CANNOT BE ACCOMMODATED IN THE CONCRETE SECTION. ALWAYS USE TENSION LAP SPLICE LENGTH VALUES, UNLESS THE PLANS OR DETAILS NOTE OTHERWISE. TABULATED DEVELOPMENT AND LAP SPLICE LENGTHS ARE BASED ON REINFORCING STEEL YIELD STRENGTH Fy=60 KSI, NORMAL WEIGHT CONCRETE, AND CLASS B LAPS. TOP BARS ARE DEFINED AS HORIZONTAL BARS WITH MORE THAN 12 INCHES OF FRESH CONCRETE CAST IN THE

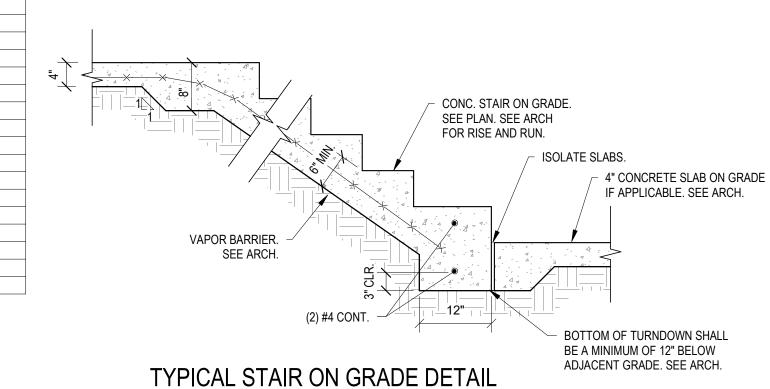
MEMBER BELOW THE DEVELOPMENT LENGTH OR SPLICE. TOP BAR FACTOR DOES NOT APPLY TO BARS IN WALLS. WHEN DIFFERENT BAR DIAMETERS ARE SPLICED, USE SMALLER BAR LAP SPLICE LENGTH. ALL TABULATED VALUES ARE MINIMUM LENGTH, IN CASE OF CONFLICT WITH PLANS, SECTIONS, OR DETAILS USE LONGER LENGTH. d/b = BAR DIAMETER I/d = DEVELOPMENT, LAP OR SPLICE LENGTH.

ADJUST TABULATED LENGTH BY THE FOLLOWING FACTORS WHERE APPLICABLE. NOTE THAT FACTORS ARE CUMULATIVE (E.G. 1.30x1.50 = 1.95) LIGHT WEIGHT CONCRETE 3 OR LESS BUNDLED BARS: 4 OR MORE BUNDLED BARS CLEAR SPACING LESS THAN 2d/b AND CLEAR COVER LESS THAN d/b

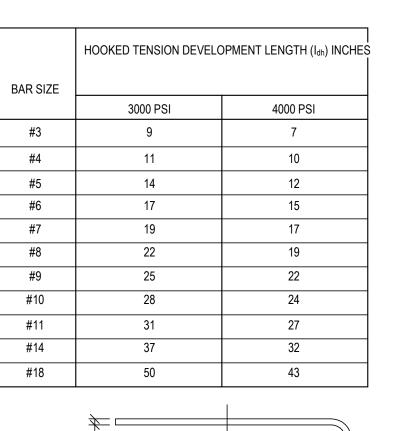
CLASS A LAP SPLICE EPOXY COATED BARS WELDED AND/OR MECHANICAL SPLICES MAY BE USED AT THE GENERAL CONTRACTOR'S OPTION PROVIDED THAT THE SPLICE IS CAPABLE OF DEVELOPING AT LEAST 125% OF THE YIELD STRENGTH OF THE LARGER BAR IN TENSION. WHERE WELDED AND/OR MECHANICAL SPLICES ARE TO BE USED, THE GENERAL CONTRACTOR SHALL SUBMIT FULL DATA OF THE PROPOSED MATERIAL, PROCEDURES, AND INSTALLATION INSTRUCTIONS TO THE ENGINEER FOR REVIEW AS A SHOP DRAWING SUBMISSION.

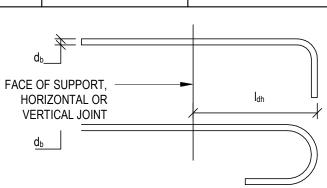
USE MECHANICAL COUPLERS FOR #14 AND LARGER BARS. LAP SPLICES IN CONCRETE MASONRY SHALL BE AS SPECIFIED IN STRUCTURAL NOTES.

TYP. STRAIGHT REINFORCEMENT DEVELOPMENT AND SPLICE LENGTH SCHEDULE



SCALE: N.T.S.



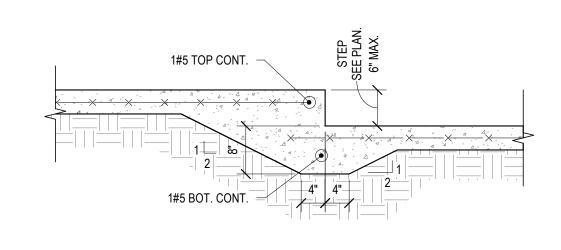


SEE TYPICAL TIE AND STIRRUP HOOKS DETAIL FOR ADDITIONAL

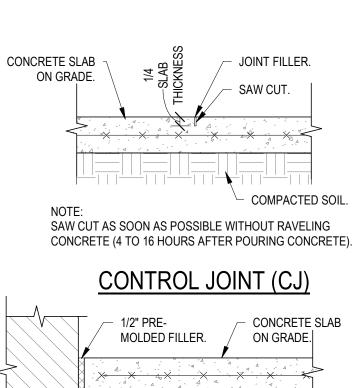
INFORMATION TABULATED DEVELOPMENT LENGTH ARE BASED ON REINFORCING STEEL YIELD STRENGTH F/y = 60 KSI AND NORMAL WEIGHT CONCRETE. ALL TABULATED VALUES ARE MINIMUM LENGTHS. IN CASE OF CONFLICT WITH THE PLANS, SECTIONS, OR DETAILS, USE THE LONGER LENGTH. $d_b = BAR DIAMETER$

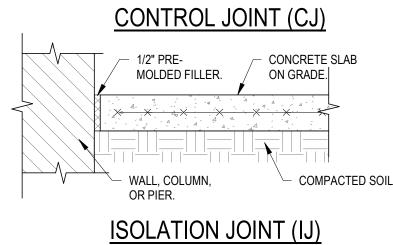
I_{dh} = DEVELOPMENT LENGTH ADJUST TABULATED LENGTHS BY THE FOLLOWING FACTORS WHERE APPLICABLE. NOTE THAT THE FACTORS ARE CUMULATIVE. REINFORCING BAR STRENGTH OTHER THAN 60 KSI: (F_V/60 KSI) LIGHT WEIGHT CONCRETE: 1.30 EPOXY COATED BARS 1.20

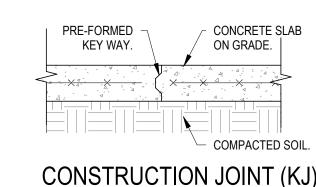
TYP. HOOKED REINFORCEMENT TENSION DEVELOPMENT LENGTH SCHEDULE



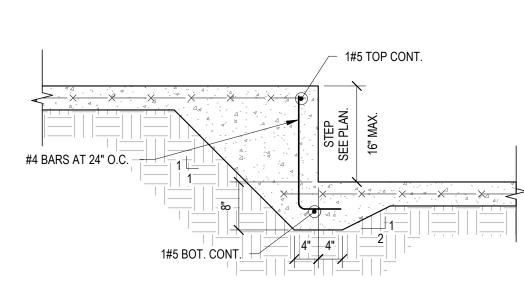
STEPS 6" OR LESS







SHEET LIST					
SHEET NAME	SHEET NO.				
DUMPSTER ENCLOSURE - STRUCT. NOTES, TYPICAL DETAILS	AS-7				
DUMPSTER ENCLOSURE - PLAN, SCHEDULES, SECTIONS	AS-8				



STEPS GREATER THAN 6"

SLAB ON GRADE STEP TYPICAL DETAILS



Baker Barrios ORLANDO

189 S. ORANGE AVE., SUITE 1700

INFO@BAKERBARRIOS.COM

AA0002981 | LC26000427

ORLANDO, FLORIDA 32801

BAKERBARRIOS.COM

407 926 3000

NGINEER OF RECORD Cordell S. Van Nostrand FL P.E. # 67580

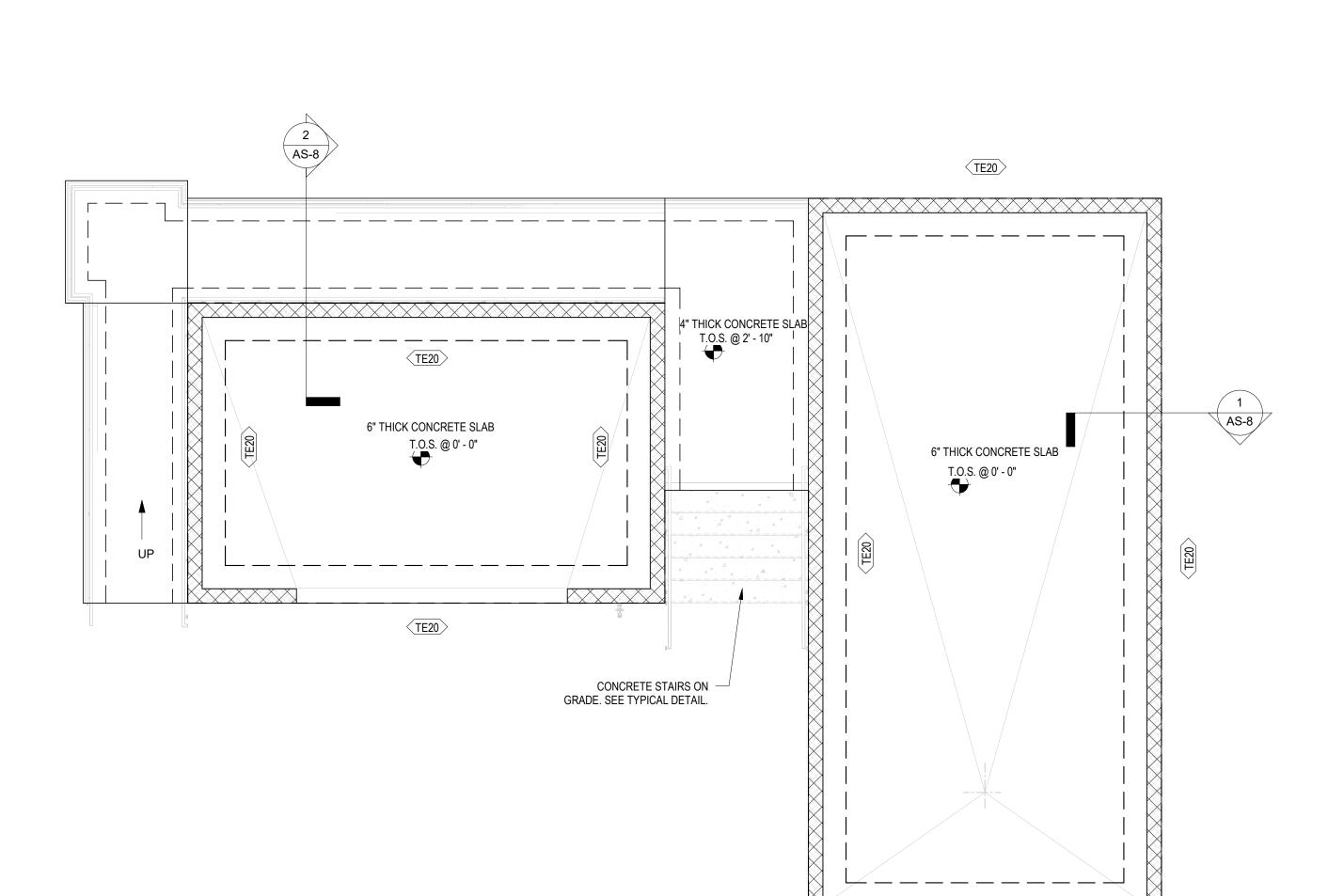
SUBMISSION CATED OR REPRESENTED BY THIS DRAWING AR WNED BY AND THE PROPERTY OF <u>BAKER BARRIOS</u> C<u>CHITECTS, INC</u>. AND WERE CREATED, EVOLVED, AND EVELOPED FOR USE ON AND IN CONNECTION WITH TH CIFIED PROJECT. NONE OF THE IDEAS, DESIGN ANGEMENTS OR PLANS SHALL BE USED BY OR LOSED TO ANY PERSON, FIRM, OR CORPORA ISSION OF <u>BAKER BARRIOS ARCHITECTS, INC</u> VING: REPRODUCTION HEREOF IS A CRIMINAL FENSE UNDER 18 U.S.C. SEC. 506 UNAUTHORIZ LOSURE MAY CONSTITUTE TRADE SECRET SAPPROPRIATION IN VIOLATION OF 1.C.24-2-31-1 Q. AND OTHER LAWS. THE IDEAS, ARRANGEMENTS NS DISCLOSED HEREIN MAY BE PATENTED OR B THE BEST OF THE ARCHITECT'S OR ENGINEER'S WLEDGE AND ABILITY, THE PLANS AND CIFICATIONS COMPLY WITH THE APPLICABLE NIMUM BUILDING CODES.

MILHAUS

ISR-82

7780 LIGHTARD KNOTT LN FORT MYERS, FL 33905

220035.00 **DUMPSTER ENCLOSURE -**STRUCT. NOTES, TYPICAL DETAILS



FOUNDATION & GROUND FLOOR PLAN

(TE20)

(2) 8" K.O. BLOCKS AT TOP OF WALL, GROUTED SOLID W/ 1#5 BAR IN EACH COURSE. PROVIDE CORNER BARS FOR ALL BARS.	
FINISHED GRADE. SEE CIVIL. TO. SLAB VAPOR BARRIER. SEE ARCH. THICKENED EDGE FOOTING. SEE PLAN. WELL COMPACTED SOIL.	8" CONCRETE WALL REINFORCE W. #5 VERT AND HORIZ BARS @ 12" O.C. MAX, EACH WAY (SINGLE MAT). FINISHED GRADE. SEE CIVIL T.O. SLAB O'-O" VAPOR BARRIER. SEE ARCH. THICKENED EDGE FOOTING. SEE PLAN. WELL COMPACTED SOIL.
1 SECTION AS-8 3/4" = 1'-0"	2 SECTION AS-8 3/4" = 1'-0"

FOUNDATION SCHEDULE						
MARK	WIDTH	LENGTH	DEPTH	REINFORCING	REMARKS	
TE20	1' - 8"	CONT.	1' - 4"	2#5 BOT CONT	TOP OF FTG. = TOP OF SLAB. ENSURE BOTTOM OF FOOTING IS A MINIMUM OF 12" BELOW ADJACENT GRADE.	

FOUNDATION SCHEDULE NOTES:

- 1. PROVIDE CORNER BARS AT ALL FOOTING INTERSECTIONS.
- PROVIDE 3" CLEAR COVER FOR ALL REINFORCING UNLESS DETAILED OTHERWISE. BOTTOM OF ALL FOUNDATIONS SHALL BE BELOW THE FROST LINE BASED ON FINISHED GRADE. GC SHALL COORDINATED WITH CIVIL FOR FINISHED GRADES. IN NO CASE SHALL FOUNDATIONS BE LESS THAN 12" BELOW ADJACENT GRADE, EXCEPT FOR NON-LOAD BEARING TE FOOTINGS.

WALL LEGEND

INDICATES AN 8" LOAD BEARING MASONRY WALL. PROVIDE 1#6 BAR IN GROUTED CELLS AT CORNERS, ENDS, INTERSECTIONS OF WALLS, EACH SIDE OF OPENINGS, UNDER POINT LOADS, AND AT 24" O.C. MAX. ALL CELLS BELOW GRADE, OR IN CONTACT WITH SOIL, SHALL BE GROUTED SOLID. WHERE 12" MASONRY IS SHOWN, GROUT AND REINFORCE PER NOTE ABOVE. PROVIDE (2) FULLY GROUTED 8" K.O. COURSES AT TOP OF WALLS W/ 1#5 CONT. IN EACH COURSE

FOUNDATION & GROUND FLOOR PLAN NOTES:

- 1. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. SEE ARCHITECTURAL AND MEP FOR ADDITIONAL INFORMATION.
- 2. SEE ARCHITECTURAL DRAWINGS FOR ALL SLOPES, DROPS AND DRAIN LOCATIONS IN FLOOR SLAB. MAINTAIN 4" MINIMUM SLAB DEPTH. THICKEN SLAB TO 8" WITHIN 4" OF ALL SLAB STEPS; MAINTAIN 4" MINIMUM SLAB DEPTH ELSEWHERE.
- 3. PROVIDE CONTROL JOINTS IN ALL SLABS ON GRADE. CONTROL JOINTS SHALL BE TOOLED OR SAWCUT AS SOON AS POSSIBLE WITHOUT RAVELING. THE PATTERN SHALL BE APPROXIMATELY SQUARE AND LIMITED TO AN AREA OF 144 SF.
- 4. GROUND FLOOR SHALL BE 6" CONCRETE SLAB-ON-GRADE, U.N.O. REINFORCE W/ 6X6 W2.9XW2.9 W.W.F. AT MID-DEPTH.

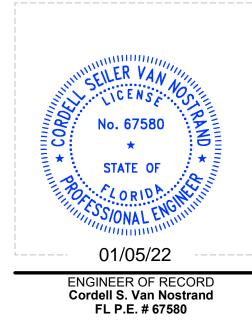
Baker Barrios

ORLANDO

407 926 3000
INFO@BAKERBARRIOS.COM
BAKERBARRIOS.COM
AA0002981 | LC26000427

189 S. ORANGE AVE., SUITE 1700

ORLANDO, FLORIDA 32801



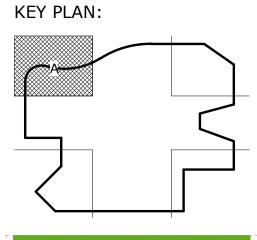
PERMIT SET

DATE SUBMISSION

ALL IDEAS, DESIGNS, ARRANGEMENTS AND PLANS INDICATED OR REPRESENTED BY THIS DRAWING ARE OWNED BY AND THE PROPERTY OF BAKER BARRIOS ARCHITECTS, INC. AND WERE CREATED, EVOLVED, AND DEVELOPED FOR USE ON AND IN CONNECTION WITH THE SPECIFIED PROJECT. NONE OF THE IDEAS, DESIGNS, ARRANGEMENTS OR PLANS SHALL BE USED BY OR DISCLOSED TO ANY PERSON, FIRM, OR CORPORATION FOR ANY PURPOSE WHATSOEVER WITHOUT THE WRITTEN PERMISSION OF BAKER BARRIOS ARCHITECTS, INC. WARNING: REPRODUCTION HEREOF IS A CRIMINAL OFFENSE UNDER 18 U.S.C. SEC. 506 UNAUTHORIZED DISCLOSURE MAY CONSTITUTE TRADE SECRET MISAPROPRIATION IN VIOLATION OF 1.C.24-2-31-1 ET. SEQ. AND OTHER LAWS. THE IDEAS, ARRANGEMENTS AND DESIGNS DISCLOSED HEREIN MAY BE PATENTED OR BE THE SUBJECT OF PENDING PATENT APPLICATION.

TO THE BEST OF THE ARCHITECT'S OR ENGINEER'S KNOWLEDGE AND ABILITY, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES.

Y PLAN:





MILHAUS

SR-82

7780 LIGHTARD KNOTT LN FORT MYERS, FL 33905
PROJECT NO:
220035.00

DUMPSTER
ENCLOSURE PLAN, SCHEDULES,
SECTIONS

GENERAL NOTES:

STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.

ALL DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK. DO NOT SCALE DRAWINGS.

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS.

ELECTRONIC VERSIONS OF THE STRUCTURAL DRAWINGS ARE THE SOLE, COPYRIGHTED PROPERTY OF SNELL ENGINEERING AND ARE NOT TO BE USED OR TRANSFERRED WITHOUT THE EXPRESS, WRITTEN PERMISSION OF SNELL ENGINEERING.

DESIGN LOADS:

THE STRUCTURAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE FLORIDA BUILDING CODE - BUILDING 7TH EDITION (2020). THE FOLLOWING SUPERIMPOSED LOADINGS HAVE BEEN UTILIZED:

ROOF:			
	LIVE LOAD	-	20 PSF.
	LIVE LOAD (CONCENTRATED)	-	300 LBS.
	DEAD LOAD	-	25 PSF.
	DEAD LOAD (AVAILABLE TO RESIST UPLIFT)	-	5 PSF.

FLOORS AND OTHER SIMILAR SURFACES SHALL BE DESIGNED TO SUPPORT THE UNIFORMLY DISTRIBUTED LIVE LOADS OR CONCENTRATED LIVE LOADS SHOWN ABOVE, WHICHEVER PRODUCES THE GREATER LOAD EFFECT. DISTRIBUTED LOADS AND CONCENTRATED LOADS ARE NON-CONCURRENT UNLESS NOTED OTHERWISE.

WIND:							
	ASCE 7-	16					
		TE WIND SPEED				_	152 MPH
						-	
		ABLE WIND SPEED				-	118 MPH
	EXPOSL	JRE C					
	ENCLOS	SED STRUCTURE					
	INTERN	AL PRESSURE COE	FFICIENT	•		_	+/- 0.18
	RISK FA						,
	_		DDECCUI	250			
	SEE WIN	ND SCHEDULE FOR	PRESSU	KES			
FLOOD:							
	FEMA X	ZONE					
	FLOOD I	DESIGN CLASS				_	II
	BFE ELE					_	11.00 NAVD
						_	
	FREEBC	JAKU			,	•	1 FT
SEISMIC	; :						
	RISK CA	TEGORY					II
	SEISMIC	IMPORTANCE FAC	CTOR le			_	1.0
	SITE CL						1.0
			7 \/				
		DESIGN CATEGOR					
	Ss	0.048 g	S1	0.024 g			
	Sds	0.051 g	Sd1	0.038 g			
		•		•			

IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO CONFIRM ALL FINAL WEIGHTS OF EQUIPMENT (MECHANICAL, CHILLED WATER, KITCHEN, SPA/POOL, ETC.), CLADDING, FINISHES, ETC. PRIOR TO ERECTING STRUCTURE SUPPORTING THESE ITEMS. ALL WEIGHTS ACCOUNTED FOR IN THE STRUCTURAL DESIGN ARE LISTED ABOVE OR ON THE PLANS.

SHOP DRAWING REVIEW:

ADEQUATE REVIEW PERIOD.

SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY.

LENGTH, ELEVATIONS, DIMENSIONS, ETC. ANY COMPONENT NOTED AS "DELEGATED" SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF FLORIDA AND NOT BY THE SER. SIGNED AND SEALED DRAWINGS SHALL BE PROVIDED TO THE ARCHITECT AND SEOR FOR REVIEW AS A SHOP DRAWING;

CALCULATIONS WILL BE PROVIDED IF REQUESTED. ALL SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. DRAWINGS SUBMITTED WITHOUT REVIEW NOTATION WILL BE RETURNED UNCHECKED. EVERY EFFORT WILL BE MADE TO RETURN THE SHOP DRAWINGS WITHIN TEN BUSINESS DAYS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO SUBMIT THE SHOP DRAWINGS ALLOWING FOR AN

ONE SET OF PRINTS WILL BE RETAINED BY THE ENGINEER AND ONE BY THE ARCHITECT, THE CONTRACTOR SHALL RECEIVE THE REMAINING PRINTS FOR SUBMITTAL TO THE BUILDING DEPARTMENT AND AS REQUIRED FOR DISTRIBUTION.

IN ALL INSTANCES THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN A REQUEST FOR INFORMATION (RFI) OR SIMILAR DOCUMENTATION BY THE ENGINEER. EVERY EFFORT WILL BE MADE TO RETURN THE RFIS WITHIN TWO BUSINESS DAYS. SUBMITTALS SHALL CLEARLY IDENTIFY THE SPECIFIC PROJECT, APPLICABLE CODES AND DESIGN CRITERIA, AND DETAILING OF ALL COMPONENTS NECESSARY TO ENSURE PROPER INSTALLATION OF THE COMPONENTS AND SYSTEM.

SHOP DRAWINGS SHOULD BE SUBMITTED FOR ALL COMPONENTS OF THE STRUCTURAL FRAMING SYSTEM, AS REQUIRED BY THE ARCHITECT, AND AS NOTED ELSEWHERE IN THESE NOTES, INCLUDING, BUT NOT LIMITED TO: CONCRETE MIX DESIGNS CONCRETE REINFORCEMENT

PRE-ENGINEERED WOOD TRUSSES ANY PROPOSED MANUFACTURER CHANGE FROM THE BASIS OF DESIGN

IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE ALL TRADES AND CONSULTANTS, CROSS REFERENCE THEIR DRAWINGS WITH THE OVERALL DESIGN, AND PROVIDE TO EACH A COMPLETE SET OF DRAWINGS AND SUBMITTALS TO ENSURE COMPATIBILITY OF CONSTRUCTION PER DESIGN INTENT. FOUNDATIONS:

ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2,000 PSF ON COMPACTED FILL OR COMPACTED NATIVE SOIL. BEFORE CONSTRUCTION COMMENCES AND IF REQUIRED BY THE JURISDICTION, SOIL BEARING CAPACITY SHALL BE VERIFIED BY A SUBSURFACE INVESTIGATION, AS WELL AS FIELD AND LABORATORY TESTS PERFORMED BY A CERTIFIED TESTING LABORATORY, WHOSE REPORT SHALL INCLUDE ANALYSIS AND RECOMMENDATIONS FOR SITE PREPARATION IN ORDER TO BEAR THE FOUNDATION LOADS. ABOVE REPORT SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW BEFORE FOUNDATION CONSTRUCTION BEGINS.

SOIL COMPACTION:

FOUNDATIONS ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE AS SPECIFIED ABOVE. SOIL COMPACTION SHALL MEET THE MORE STRINGENT OF THE CRITERIA LISTED BELOW OR AS SPECIFIED IN THE GEOTECHNICAL REPORT. REMOVE TOP-SOIL TO A MINIMUM DEPTH OF ONE FOOT OVER THE ENTIRE BUILDING AREA AND FIVE FEET BEYOND BUILDING LINES. COMPACT SUB-GRADE USING A VIBRATORY COMPACTER SUCH AS "VIBRATOW II" OR EQUIVALENT WITH A MINIMUM OF FOUR PASSES. PLACE AND COMPACT FILL MATERIAL TO FINISHED GRADE LEVEL IN LIFTS NOT EXCEEDING 12" THICK. SUB-GRADE AND EACH LIFT SHALL BE COMPACTED TO MINIMUM 95% MODIFIED PROCTOR DENSITY DETERMINED IN ACCORDANCE WITH ASTM. D-1557. VERIFICATION THAT THE COMPACTION REQUIREMENTS HAVE BEEN MET SHALL BE MADE BY AN INDEPENDENT GEOTECHNICAL CONSULTANT EMPLOYED BY THE OWNER AND APPROVED BY THE ENGINEER. LOCATIONS FAILING TO MEET THE REQUIREMENTS SHALL BE RECOMPACTED AND RETESTED AT THE CONTRACTORS EXPENSE AND AS DIRECTED BY THE INDEPENDENT GEOTECHNICAL CONSULTANT.

PENETRATIONS:

NO PENETRATIONS SHALL BE MADE IN ANY STRUCTURAL MEMBERS WITHOUT PREVIOUS APPROVAL OF THE EOR, EXCEPT THOSE PENETRATIONS SHOWN ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHOULD SUBMIT DRAWINGS SHOWING: ANY LOCATION WHERE OPENINGS, PENETRATIONS, OR ANY PLACE MORE THAN 3 PIPES OR CONDUITS ARE LOCATED IN A SLAB SYSTEM; ANY LOCATION IN A BEAM OR COLUMN WHERE MORE THAN 4% OF THE MEMBER SECTION IS REMOVED IN ANY ORIENTATION.

PLUMBING SLEEVES:

MINIMUM SLEEVE SPACING SHALL BE THREE DIAMETERS CENTER TO CENTER OF THE LARGER SLEEVE OR 6" CLEAR BETWEEN SLEEVES. WHICHEVER IS GREATER. PRIOR TO CONSTRUCTION SLEEVE LOCATIONS AND SIZES SHALL BE APPROVED BY THE ENGINEER. CONDUITS PIPES AND SLEEVES SHALL BE PLACED AND SPACED IN ACCORDANCE WITH ACI 318 6.3.

REINFORCING STEEL:

SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. SECURE APPROVAL OF SHOP DRAWINGS PRIOR TO

REINFORCING BARS SHOWN IN SECTIONS DEPICT TYPICAL CONFIGURATION AND ARE NOT SPECIFIC TO THE CONCRETE MEMBER CUT ON PLAN; SEE PLAN AND SCHEDULES FOR ALL BAR SIZE AND QUANTITIES. TAKE-OFFS AND QUANTITIES SHALL BE OBTAINED FROM THE SCHEDULES AND PLANS, NOT FROM SECTIONS. WHERE HOOKS, LAP LENGTHS, ETC. ARE SHOWN IN SECTIONS, ALL LENGTHS AND DETAILS SHALL MEET ACI REQUIREMENTS FOR REINFORCING DETAILS. HOOKS SHALL BE ORIENTED AS REQUIRED TO FIT WITHIN THE CONCRETE

WELDED WIRE FABRIC:

TO CONFORM TO ASTM A-185, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS. MINIMUM LAP SHALL BE ONE SPACE PLUS TWO INCHES.

ALL CONCRETE SHALL MEET ACI 318 'BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE' AND ACI 301 'SPECIFICATIONS FOR STRUCTURAL CONCRETE', INCLUDING, BUT NOT LIMITED TO, MIX DESIGN, STRENGTH, COVER, PLACEMENT, CURING, TESTING, FORMS. FLATNESS, ETC. SEE THE ARCHITECTURAL SPECIFICATIONS AND PLANS FOR ANY ADDITIONAL REQUIREMENTS.

ALL CONCRETE SHALL BE AN APPROVED MIX DESIGN PROPORTIONED TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW WITH A PLASTIC AND WORKABLE MIX:

3000 PSI FOR FOUNDATIONS AND SLABS ON GRADE. 4000 PSI FOR ALL OTHER STRUCTURAL CONCRETE.

SUBMIT PROPOSED MIX DESIGN FOR EACH PORTION OF WORK PRIOR TO USE. ALL MIX DESIGNS SHALL BE ACCOMPANIED BY A MINIMUM OF 15 FIELD STRENGTH TEST RECORDS, AS NOTED IN ACI 301 4.2.3.2(A); NO OTHER DOCUMENTATION OF AVERAGE COMPRESSIVE STRENGTH WILL BE ACCEPTED WITHOUT PRIOR WRITTEN APPROVAL BY THE SER. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. MIX SHALL MEET THE REQUIREMENTS OF ASTM C33 FOR COARSE AGGREGATE. FOR ALL FLATWORK, AT LEAST 75% OF LARGE AGGREGATE SHALL CONSIST OF #57 STONE. CONCRETE SHALL COMPLY WITH ALL THE REQUIREMENTS OF ASTM STANDARD C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED WHEN CONCRETE IS

ALL CONCRETE MIX DESIGNS SHALL INCLUDE A WRITTEN DESCRIPTION INDICATING WHERE EACH PARTICULAR MIX IS TO BE PLACED WITHIN TRUSSES AT ALL BEARING LOCATIONS SHALL BE STABILIZED. THE GENERAL CONTRACTOR SHALL FOLLOW ALL REQUIREMENTS BY THE THE STRUCTURE.

CONCRETE SHALL BE PLACED AND CURED ACCORDING TO ALL STANDARDS AND SPECIFICATIONS, INCLUDING ALL ACI REQUIREMENTS.

THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1-1/2) HOURS. IF FOR ANY REASON THERE IS A LONGER DELAY THAN THAT STATED ABOVE. THE CONCRETE SHALL BE DISCARDED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LAB TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE. ALL SLABS SHALL BE CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1-D AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE. ALL SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY. CALCIUM CHLORIDES SHALL NOT BE UTILIZED; OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER.

THE GENERAL CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD A MINIMUM OF 48 HOURS PRIOR PLACEMENT OF ANY STRUCTURAL

UNLESS NOTED OTHERWISE ON PLANS, THE FOLLOWING CONCRETE CLEAR COVER SHALL BE PROVIDED FOR ALL NON-PRESTRESSED CONCRETE REINFORCEMENT PER ACI 318:

CONCRETE CAST AGAINST EARTH:	ALL BARS	-
CONCRETE EXPOSED TO EARTH (FORMED FACE):	ALL BARS	-
CONCRETE EXPOSED TO WEATHER:	#6 BARS AND GREATER	-
	#5 BARS AND SMALLER	-
WHERE NOT EXPOSED TO EARTH OR WEATHER:		
SLABS, WALLS, AND JOISTS:	#14 & #18 BARS	-
	#11 BARS AND SMALLER	-
BEAMS AND COLUMNS:	ALL BARS	-

A) ASTM C143 - "STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE." MAXIMUM SLUMP SHALL BE 4-6 INCHES. PRIOR TO ADDING A SUPER PLASTICIZER. B) ASTM C39 - "STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS." A SEPARATE TEST SHALL BE CONDUCTED FOR EACH CLASS, FOR EVERY 50 CUBIC YARDS (OR FRACTION THEREOF), PLACED PER DAY. REQUIRED CYLINDER(S) QUANTITIES AND TEST AGE AS FOLLOWS: 1 AT 7 DAYS

1 1/2"

ONE ADDITIONAL RESERVE CYLINDER TO BE TESTED UNDER THE DIRECTION OF THE ENGINEER, IF REQUIRED. IF 28 DAY STRENGTH IS ACHIEVED, THE ADDITIONAL CYLINDER(S) MAY BE DISCARDED.

POUR STRUCTURAL CONCRETE WITHIN THE FOLLOWING TOL	ERANCES:
VARIATION FROM PLUMB:	1/4" IN 10'-0"
VARIATION FROM LEVEL IN TOPS OF PILASTERS:	1/8" IN 10'-0"
VARIATION FOOTINGS:	
PLAN DIMENSIONS:	+2", - 1/2"
THICKNESS:	- 0"

CHEMICAL ANCHORS:

SUPPLEMENT, AS FOLLOWS:

2 AT 28 DAYS

SHALL BE AN EQUAL TWO-PART EPOXY POLYMER INJECTION SYSTEM, SUCH AS SIMPSON SET-XP "STRUCTURAL ANCHORING ADHESIVE" HILTI HIT-HY 200 OR ENGINEER APPROVED SUBSTITUTION, INSTALLED IN ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS. INSTALLERS SHALL BE TRAINED BY THE MANUFACTURER'S REPRESENTATIVE. BRUSH AND BLOW OUT ALL HOLES.

STRUCTURAL WOOD COMPONENTS (BEAMS, JOISTS, RAFTERS, ETC.) SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE FIBER STRESSES FOR NO. 2 SOUTHERN PINE CONFORMING TO NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION WITH 2015 NDS

SHEAR	FV	= 175 PSI.
BENDING 2X6	FB	= 1,000 PSI.
BENDING 2X8	FB	= 925 PSI.
BENDING 2X10	FB	= 800 PSI.
BENDING 2X12	FB	= 750 PSI.

WOOD IN CONTACT WITH CONCRETE OR MASONRY, AND AT OTHER LOCATIONS AS SHOWN ON STRUCTURAL DRAWINGS, SHALL BE PROTECTED OR PRESSURE TREATED IN ACCORDANCE WITH AITC-109. MEMBER SIZES SHOWN ARE NOMINAL UNLESS NOTED OTHERWISE.

ALL NAILS SHOWN ON PLANS ASSUME COMMON WIRE NAILS UNLESS SPECIFICALLY NOTED ON DRAWINGS. BOLTS FOR WOOD CONSTRUCTION AT INTERIOR LOCATIONS SHALL CONFORM TO ASTM A307. THREADED RODS AT INTERIOR LOCATIONS SHALL CONFORM TO ASTM A36. EXTERIOR BOLTS AND THREADED RODS PROTECTED FROM MOISTURE AND WEATHER SHALL BE HOT-DIP GALVANIZED. EXPOSED EXTERIOR BOLTS AND THREADED RODS SHALL BE AISI 316 STAINLESS STEEL. ALL NAILS LARGER THAN 10D AND SCREWS LARGER THAN 7 GAUGE SHALL BE PREDRILLED AS NEEDED TO PREVENT SPLITTING OF THE WOOD. BOLT HOLES IN WOOD MEMBERS SHALL BE A MINIMUM OF 1/32" LARGER THAN THE BOLT DIAMETER, BUT NO GREATER THAN 1/16" LARGER. A METAL PLATE OR WASHER NO SMALLER THAN A STANDARD CUT WASHER SHALL BE LOCATED BETWEEN THE BOLT HEAD AND THE WOOD AND BETWEEN THE NUT AND THE

FOR WOOD WALLS, PROVIDE A SINGLE CONTINUOUS BOTTOM PLATE AND DOUBLE CONTINUOUS TOP PLATE. THE TOP PLATE PLIES SHALL HAVE STAGGERED LAPS OF 24" WITH (8) 10D NAILS IN EACH SPLICE. AT WALL CORNERS AND INTERSECTIONS, TOP PLATES SHALL BE STAGGERED AND OVERLAPPED WITH (4) 10D NAILS IN THE JOINT.

ENGINEERED WOOD TRUSS SYSTEMS SHALL BE DESIGNED BY SUPPLIER'S SPECIALTY ENGINEER TO CONFIGURATION AND LOAD-CARRYING CAPACITY SHOWN ON DRAWINGS AND SPECIFICATIONS. ALL INDIVIDUAL TRUSS MEMBERS, TRUSS PLATE CONNECTIONS, TRUSS-TO-TRUSS CONNECTIONS, COMMON TRUSSES, AND GIRDER TRUSSES SHALL BE DESIGNED FOR COMPONENT AND CLADDING WIND LOADING, EXCEPT THOSE TRUSSES EXCEEDING 700 SQUARE FEET IN TRIBUTARY AREA. ALTERNATE TRUSS LAYOUTS ARE ACCEPTABLE ONLY AS A CHANGE ORDER WHICH WILL INCLUDE ENGINEERING CHARGES FOR REDESIGN OF THE STRUCTURE BY THE ENGINEER OF RECORD. SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS SHALL SHOW AND SPECIFY ALL CONNECTOR TYPES UTILIZED WITHIN TRUSSES. AS WELL AS CONNECTORS UTILIZED IN ALL OTHER CONNECTIONS AND ATTACHMENTS BETWEEN TRUSSES OR COMPONENTS SUPPLIED AS PART OF THE ENGINEERED TRUSS SYSTEM. AN ERECTION DRAWING SHALL BE INCLUDED, IDENTIFYING ALL TRUSS SYSTEM COMPONENTS, AS WELL AS ALL PERMANENT BRACING REQUIRED FOR TRUSS DESIGN.

DELEGATED TRUSS ENGINEERING PACKAGE. AT A MINIMUM: FOR TRUSS HEEL DEPTHS LESS THAN 8", ONLY BLOCKING BY THE TRUSS ENGINEERING IS REQUIRED; FOR TRUSS HEEL DEPTHS BETWEEN 8" AND 12", USE FULL HEIGHT SAWN-LUMBER BLOCKING; FOR TRUSS HEEL DEPTHS GREATER THAN 12", USE TRUSS BLOCKING OR A SHEATHED KNEEWALL. SEE THE STRUCTURAL DRAWINGS FOR ANY ADDITIONAL BLOCK REQUIREMENTS FOR THE LATERAL-FORCE RESISTING SYSTEM.

"FLOOR TRUSSES" AND "4X2 TRUSSES" REFER TO TRUSSES WITH THE TOP AND BOTTOM CHORDS ORIENTED WITH THE STRONG AXIS HORIZONTAL (I.E. ON THE FLAT). "ROOF TRUSSES" AND "2X4 TRUSSES" REFER TO TRUSSES WITH THE TOP AND BOTTOM CHORDS ORIENTED WITH THE STONG AXIS VERTICAL.

ENGINEERED SHOP DRAWINGS SHALL BEAR THE SIGNATURE AND IMPRESSED SEAL OF A REGISTERED PROFESSIONAL ENGINEER, LICENSED IN THE STATE WHERE THE STRUCTURE WILL BE ERECTED. AS THE DELEGATED (SPECIALTY) ENGINEER. PLUMBING, ELECTRICAL. AND MECHANICAL DRAWINGS SHALL BE COORDINATED WITH THE TRUSS LAYOUT TO ENSURE THAT THERE ARE NO CONFLICTS WITH DUCTS, RECESSED FIXTURES, PLUMBING PIPES, TRAPS, HOODS, CEILING STEPS/SLOPES, ETC. TRUSS LAYOUT SHALL BE MODIFIED AND/OR TRUSS CHASES SHALL BE ADDED TO AVOID CONFLICTS. TRUSS SPACING SHALL NOT EXCEED MAXIMUM NOTED IN PLAN NOTES, U.N.O.

THE FOLLOWING LOAD DURATION FACTORS SHALL BE USED:

THE SUPERIMPOSED DEAD LOAD, AS SPECIFIED IN THE DESIGN LOADS SECTION ABOVE, INCLUDES THE OVERALL WEIGHT OF THE FIRE SPRINKLER SYSTEM PIPES. THE GENERAL CONTRACTOR SHALL PROVIDE THE TRUSS MANUFACTURER WITH THE LOCATIONS OF THE PIPE SUPPORTS AND THE LOADS FROM ALL SPRINKLER LINES GREATER THAN 2" DIAMETER. THE GENERAL CONTRACTOR SHALL PROVIDE THE TRUSS MANUFACTURER AND THE SEOR WITH THE FINAL WEIGHTS FOR ALL MECHANICAL EQUIPMENT, INCLUDE SECONDARY FRAMING AND CURBS, PRIOR TO FABRICATION OF TRUSSES AND STRUCTURAL MEMBERS SUPPORTING SAID TRUSSES

ALL TRUSSES SHALL BE DESIGNED FOR A MAXIMUM DEFLECTION OF L/360 FOR LIVE LOAD AND L/240, NOT TO EXCEED 1", FOR TOTAL LOAD; THE MAXIMUM DEFLECTION DUE TO TOTAL LOAD OF 1" IS INCLUSIVE OF ALL DEAD LOAD, SELF-WEIGHT, SUPERIMPOSED DEAD LOAD, AND LIVE LOAD. INCLUDING CREEP.

PLYWOOD ROOF, FLOOR, AND WALL SHEATHING ARE DESIGNED AS DIAPHRAGMS/SHEAR WALLS AND SHALL COMPLY WITH APPLICABLE PROVISIONS OF CHAPTER 23 OF THE BUILDING CODE AND SHALL BE FASTENED IN ACCORDANCE WITH THE MINIMUM REQUIREMENTS OF BUILDING CODE TABLES. UNLESS SHOWN OTHERWISE. PLYWOOD SHALL BE INSTALLED WITH THE STRENGTH AXIS OF EACH PANEL PERPENDICULAR TO THE SUPPORTS IN ALL CASES. PLYWOOD ROOF PANELS SHALL BE INSTALLED AS SHOWN IN CASES 1 THROUGH 4 IN TABLE 2306.2.1 (CONT.). BLOCKING SHALL BE PROVIDED BETWEEN ALL WOOD ROOF FRAMING MEMBERS AT ALL RIDGES AND VALLEYS FOR FULL PLYWOOD EDGE SUPPORT. AT ROOF VENT LOCATIONS, PROVIDE 2X4 BLOCKING, ON THE FLAT, ON ALTERNATING SIDES OF THE VENT BETWEEN ROOF FRAMING MEMBERS.

ALL WOOD SHEAR WALLS SHALL HAVE ALL PLYWOOD EDGES FULLY BLOCKED WITH THE SAME STUD SIZE AS THE WALLS, WITH THE BLOCKING INSTALLED SO THAT THE PLYWOOD IS NAILED INTO THE NARROW STUD FACE. FASTEN THE SHEATHING WITH THE SAME NAIL SIZE AND SPACING TO EACH PLY OF DOUBLE TOP AND BOTTOM WALL PLATES, AS APPLICABLE. FASTEN THE SHEATHING WITH THE SAME NAIL SIZE AND SPACING TO MULTI-PLY COLUMNS AT ENDS OF WALLS; WHERE SOLID COLUMNS ARE USED AT ENDS OF WALLS, FASTEN THE SHEATHING WITH THE SAME NAIL SIZE AND SPACING IN VERTICAL ROWS WITH 1-1/2" ROW SPACING FOR FULL HEIGHT OF COLUMN. WHERE SHEAR WALL PANEL EDGE NAILING IS 3" OR LESS, THE BLOCKING AT PANEL EDGES SHALL BE 3" NOMINAL OR GREATER AND THE NAILING SHALL BE STAGGERED. MULTIPLE PLY STUDS MAY BE USED AS THE PANEL EDGE BLOCKING IN LIEU OF 3" NOMINAL BLOCKING; FASTEN THE PLIES WITH NAILS HAVING THE SAME LENGTH AS THE TOTAL BLOCKING THICKNESS WITH SPACING TO MATCH THE PLYWOOD PANEL EDGE NAILING, STAGGERED. PANELS SHALL NOT BE LESS THAN 4FTX8FT, EXCEPT AT BOUNDARIES AND CHANGES IN FRAMING.

MANUFACTURED WOOD STRUCTURAL COMPONENTS:

MEMBERS DESIGNATED 'LVL' (E.G., 1-3/4 X 14 LVL) SHALL BE LAMINATED VENEER LUMBER AS MANUFACTURED BY TRUS JOIST CORPORATION (MICRO=LAM), ALPINE ENGINEERED PRODUCTS (ASI TIMBERMAX-LVL), OR ENGINEER APPROVED SUBSTITUTION. FB = 2600 PSI, E = 2,000,000 PSI, FV = 285 PSI.

MEMBERS DESIGNATED 'PSL' (E.G., 31/2"X111/4" PSL) SHALL BE PARALLAM PARALLEL STRAND LUMBER AS MANUFACTURED BY TRUS JOIST CORPORATION (MICRO=LAM), OR ENGINEER APPROVED SUBSTITUTION. FB = 2900 PSI, E = 2,000,000 PSI, FV = 290 PSI.

WOOD FRAMING CONNECTORS: ALL CONNECTORS SHALL BE GALVANIZED. CONNECTOR MODEL NUMBERS SHOWN ARE STRONG-TIE CONNECTORS AS MANUFACTURED BY SIMPSON STRONG-TIE CO., 5956 W. LAS POSITAS BLVD., P.O. BOX 10789, PLEASANTON, CA 94588, 800-999-5099, WWW.STRONGTIE.COM. SUBSTITUTIONS ARE ACCEPTABLE WITH THE APPROVAL OF THE STRUCTURAL ENGINEER. UNLESS SHOWN OTHERWISE, INSTALL THE *LARGEST* FASTENER SIZE AND *MAXIMUM* NUMBER OF FASTENERS SHOWN IN LATEST SIMPSON CATALOG. WHERE SDS SCREWS ARE SPECIFIED IN THE SIMPSON CATALOG. SDS SCREWS MUST BE USED UNLESS EXPRESSLY SHOWN IN THE DRAWINGS OTHERWISE. ALL

ALL COLUMN BASE AND HOLD-DOWN CONNECTORS (HDU, HTT, LTT, ETC.) FOR STRUCTURAL COMPOSITE LUMBER (PSL, LVL, LSL, ETC.) SHALL BE INSTALLED IN THE WIDE FACE OF THE COLUMN, AND NOT THE NARROW FACE. THE NARROW FACE IS THE SURFACE THAT SHOWS THE VERTICAL THIN EDGES OF THE STRUCTURAL COMPOSITE LUMBER LAYERS.

STRUCTURAL NOTES

FOUNDATION & GROUND

FLOOR PLAN

ROOF FRAMING PLAN

SCHEDULES

BUILDING SECTIONS

COMPRESSION

LAP / SPLICE LENGTH

ALL BARS

ALL CONCRETE WITH f'c3 3000 psi

TENSION (CLASS 'B') LAP SPLICE LENGTH

OTHER BARS

4000 PSI

62

3000 PSI

43

93

LESSER OF 6" OR Id/5

REFER TO "HOOKED REINFORCEMENT TENSION DEVELOPMENT LENGTH SCHEDULE" WHEN THE STRAIGHT

TABULATED DEVELOPMENT AND LAP SPLICE LENGTHS ARE BASED ON REINFORCING STEEL YIELD STRENGTH Fy=60

TOP BARS ARE DEFINED AS HORIZONTAL BARS WITH MORE THAN 12 INCHES OF FRESH CONCRETE CAST IN THE

MEMBER BELOW THE DEVELOPMENT LENGTH OR SPLICE. TOP BAR FACTOR DOES NOT APPLY TO BARS IN WALLS.

ALL TABULATED VALUES ARE MINIMUM LENGTH, IN CASE OF CONFLICT WITH PLANS, SECTIONS, OR DETAILS USE

WELDED AND/OR MECHANICAL SPLICES MAY BE USED AT THE GENERAL CONTRACTOR'S OPTION PROVIDED THAT THE

WHERE WELDED AND/OR MECHANICAL SPLICES ARE TO BE USED, THE GENERAL CONTRACTOR SHALL SUBMIT FULL

DATA OF THE PROPOSED MATERIAL, PROCEDURES, AND INSTALLATION INSTRUCTIONS TO THE ENGINEER FOR REVIEW

SPLICE IS CAPABLE OF DEVELOPING AT LEAST 125% OF THE YIELD STRENGTH OF THE LARGER BAR IN TENSION.

ADJUST TABULATED LENGTH BY THE FOLLOWING FACTORS WHERE APPLICABLE. NOTE THAT FACTORS ARE

DEVELOPMENT LENGTH IN TENSION CANNOT BE ACCOMMODATED IN THE CONCRETE SECTION. ALWAYS USE TENSION LAP SPLICE LENGTH VALUES. UNLESS THE PLANS OR DETAILS NOTE OTHERWISE.

WHEN DIFFERENT BAR DIAMETERS ARE SPLICED. USE SMALLER BAR LAP SPLICE LENGTH.

A. CLEAR SPACING LESS THAN 2d/b AND CLEAR COVER LESS THAN d/b

LAP SPLICES IN CONCRETE MASONRY SHALL BE AS SPECIFIED IN STRUCTURAL NOTES.

TYP. STRAIGHT REINFORCEMENT DEVELOPMENT

TOP BARS

4000 PSI

113

105

LAP LENGTH

KSI, NORMAL WEIGHT CONCRETE, AND CLASS B LAPS.

I/d = DEVELOPMENT, LAP OR SPLICE LENGTH.

LIGHT WEIGHT CONCRETE:

CUMULATIVE (E.G. 1.30x1.50 = 1.95)

B. 3 OR LESS BUNDLED BARS:

AS A SHOP DRAWING SUBMISSION.

USE MECHANICAL COUPLERS FOR #14 AND LARGER BARS.

AND SPLICE LENGTH SCHEDULE

4 OR MORE BUNDLED BARS:

3000 PSI

28

56

93

105

118

131

121

d/b = BAR DIAMETER

CLASS A LAP SPLICE

EPOXY COATED BARS

BAR SIZE

#8

#10

#14

NOTES:

CS-100

CS-101

CS-200

CS-300

BAR SIZE

#5

#10

#11

#14

#18

FACE OF SUPPORT.

HORIZONTAL OR

VERTICAL JOINT

 $d_b = BAR DIAMETER$

ROOF AND UPPER-LEVEL UPLIFT CONNECTORS SHALL BE LOCATED ON THE SAME SIDE OF THE WALL AS THE EXTERIOR SHEATHING.

FOR ALL CONNECTORS REQUIRING A THREADED ROD ATTACHMENT TO CONCRETE OR GROUT-FILLED MASONRY, PROVIDE A THREADED ROD DIAMETER AS SPECIFIED IN THE SIMPSON CATALOG WITH A DOUBLE-NUT AND OVERSIZED WASHER EMBEDDED A MINIMUM OF 9" (UNLESS NOTED OTHERWISE ON THE PLANS) INTO THE MEMBER PRIOR TO THE POUR.

ABBREVIATIONS		ABBREVIATIONS		
MARK	DESCRIPTION	MARK	DESCRIPTION	
	POUND(S)	MAX	MAXIMUM	
	AND	MIN	MINIMUM	
RCH	ARCHITECT	N.A.V.D.	NORTH AMERICAN VERTICAL DATUM	
OR BOT	BOTTOM	N.G.V.D.	NATIONAL GEODETIC VERTICAL DATUM	
OR B.O.	BOTTOM OF	NIC	NOT IN CONTRACT	
)	BASE PLATE	O.C.	ON CENTER	
ANT.	CANTILEVER	OH OR OPP HAND	OPPOSITE HAND	
	CONTROL JOINT	PLF POUNDS PRE LINE		
 .R	CLEAR	PROJ	PROJECTION	
ONT.	CONTINUOUS	PSF	POUNDS PER SQUARE FOOT	
A/Ø	DIAMETER	PSI	POUNDS PER SQUARE INCH	
١.	EACH	PT	PRESSURE TREATED/ POST TENSIONED	
	EACH END	RD	ROOF DRAIN	
:	EACH FACE	REF	REFERENCE	
	EXPANSION JOINT	REINF	REINFORCING	
=	ELEVATION	REQ'D	REQUIRED	
/IBED	EMBEDMENT	RTF	RIPPED TO FIT	
OS	EDGE OF SLAB	SEOR	STRUCTURAL ENGINEER OF RECORD	
Q .	EQUAL	SIM SIMILAR		
V	EACH WAY	SS	STAINLESS STEEL	
(IST	EXISTING	STD	STANDARD	
2	28 DAY COMPRESSIVE STRENGTH	SW	SHORT WAY/ SHEAR WALL	
)	FLOOR DRAIN	T	TOP	
E	FINISHED FLOOR ELEVATION	T.O. OR T/	TOP OF	
RT	FIRE RETARDANT TREATED	TEMP	TEMPERATURE	
G.	FOOTING	THR	THREADED	
٨.	GAUGE	THRU	THROUGH	
ALV.	GALVANIZED	TRANS	TRANSVERSE	
)	GENERAL CONTRACTOR	TYP	TYPICAL	
OR HORIZ	HORIZONTAL	V OR VERT	VERTICAL	
SS	HOLLOW STRUCTURAL STEEL	W/	WITH	
	CONSTRUCTION JOINT	WD	WOOD	
Н	LONG LEG HORIZONTAL	WP	WORKING POINT	
V	LONG LEG VERTICAL	WWF	WELDED WIRE FABRIC	
V	LONG WAY			

2x OUTLOOKERS

SEE PLAN

2x FASCIA, SEE ARCH

FOR DETAILS. -

EL.VARIES

GABLE TRUSS STIFFENERS AS

DROPPED GABLE TRUSS.

AT 24" O.C. MAX.

WOOD WALL. SEE PLAN.

PROVIDE VERTICAL MEMBERS

¹⁵/₃₂" APA RATED PLYWOOD WALL

ALL EDGES AND 12" O.C. IN THE

SHEATHING W/ 8d NAILS @ 6" O.C. AT

SAW CUT.

SAW CUT AS SOON AS POSSIBLE WITHOUT RAVELING

CONTROL JOINT (CJ)

4000 PSI

7

10

12

15

19

22

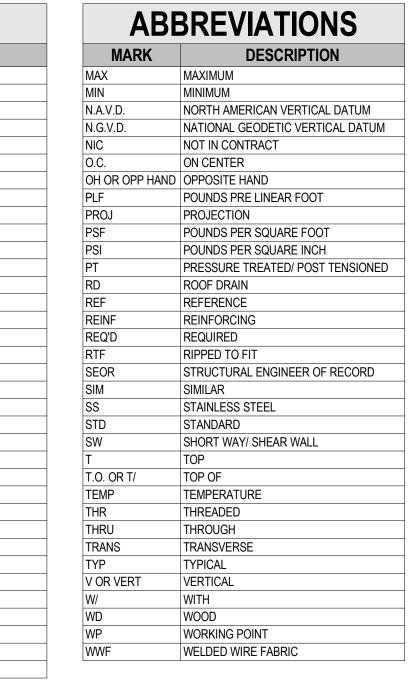
24

43

CONCRETE (4 TO 16 HOURS AFTER POURING CONCRETE).

COMPACTED SOIL.

REQUIRED BY TRUSS ENGINEER.



SHEATHING.

LUS24 @ EA

OUTLOOKER

GABLE END X-BRACING DETAIL

CONCRETE SLAB

COMPACTED SOIL.

ON GRADE.

SEE PLAN NOTES.

(2) GBC GABLE BRACE

BRACE. (1) EA. SIDE.

CONN. AT EACH

MOLDED FILLER.

WALL, COLUMN,

OR PIER.

ISOLATION JOINT (IJ)

OUTLOOKER.



2X BLOCKING TO MATCH

(6) 10D NAILS

FROM BRACE

TO BLOCKING.

DIAGONAL

PRE-FORMED

KEY WAY.

2x6 'T' BRACE CONT. TO

DIAGONAL BRACE END.

CONSTRUCTION JOINT (KJ

CONCRETE SLAB

COMPACTED SOI

ON GRADE.

WITHIN 12" OF EACH

PRE-ENGINEERED

WOOD TRUSSES.

TRUSS TOP CHORD.

(3) 10d NAILS

PT 2x6 DIAGONAL 'T'

BRACING @ 4'-0" O.C

EXTEND BRACING TO

ROOF SHEATHING.



ORLANDO

189 S. ORANGE AVE., SUITE 1700

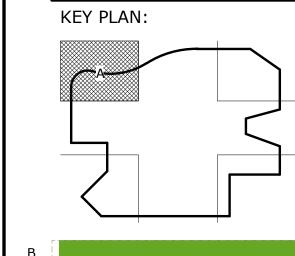
NGINEER OF RECORI

Cordell S. Van Nostrand

FL P.E. # 67580

SUBMISSION CATED OR REPRESENTED BY THIS DRAWING AR WNED BY AND THE PROPERTY OF <u>BAKER BARRIOS</u> C<u>CHITECTS, INC</u>. AND WERE CREATED, EVOLVED, AND EVELOPED FOR USE ON AND IN CONNECTION WITH TH CIFIED PROJECT. NONE OF THE IDEAS, DESIGNS ANGEMENTS OR PLANS SHALL BE USED BY OR LOSED TO ANY PERSON, FIRM, OR CORPORATI MISSION OF <u>BAKER BARRIOS ARCHITECTS, INC</u> RNING: REPRODUCTION HEREOF IS A CRIMINAL FENSE UNDER 18 U.S.C. SEC. 506 UNAUTHORIZ LOSURE MAY CONSTITUTE TRADE SECRET SAPPROPRIATION IN VIOLATION OF 1.C.24-2-31-1 Q. AND OTHER LAWS. THE IDEAS, ARRANGEMENTS NS DISCLOSED HEREIN MAY BE PATENTED OR BE THE BEST OF THE ARCHITECT'S OR ENGINEER'S WLEDGE AND ABILITY, THE PLANS AND CIFICATIONS COMPLY WITH THE APPLICABLE

NIMUM BUILDING CODES.





MILHAUS

ISR-82

7780 LIGHTARD KNOTT LN FORT MYERS, FL 33905

22063

STRUCTURAL

NOTES **CS-001**

Idh = DEVELOPMENT LENGTH ADJUST TABULATED LENGTHS BY THE FOLLOWING FACTORS WHERE APPLICABLE. NOTE THAT THE FACTORS ARE CUMULATIVE. REINFORCING BAR STRENGTH OTHER THAN 60 KSI: (F_v/60 KSI) LIGHT WEIGHT CONCRETE: 1.30 EPOXY COATED BARS 1.20 DEVELOPMENT LENGTH SCHEDULE

SEE TYPICAL TIE AND STIRRUP HOOKS DETAIL FOR ADDITIONAL

TABULATED DEVELOPMENT LENGTH ARE BASED ON REINFORCING STEEL

ALL TABULATED VALUES ARE MINIMUM LENGTHS. IN CASE OF CONFLICT

WITH THE PLANS, SECTIONS, OR DETAILS, USE THE LONGER LENGTH.

YIELD STRENGTH F/v = 60 KSI AND NORMAL WEIGHT CONCRETE.

CONCRETE SLAB

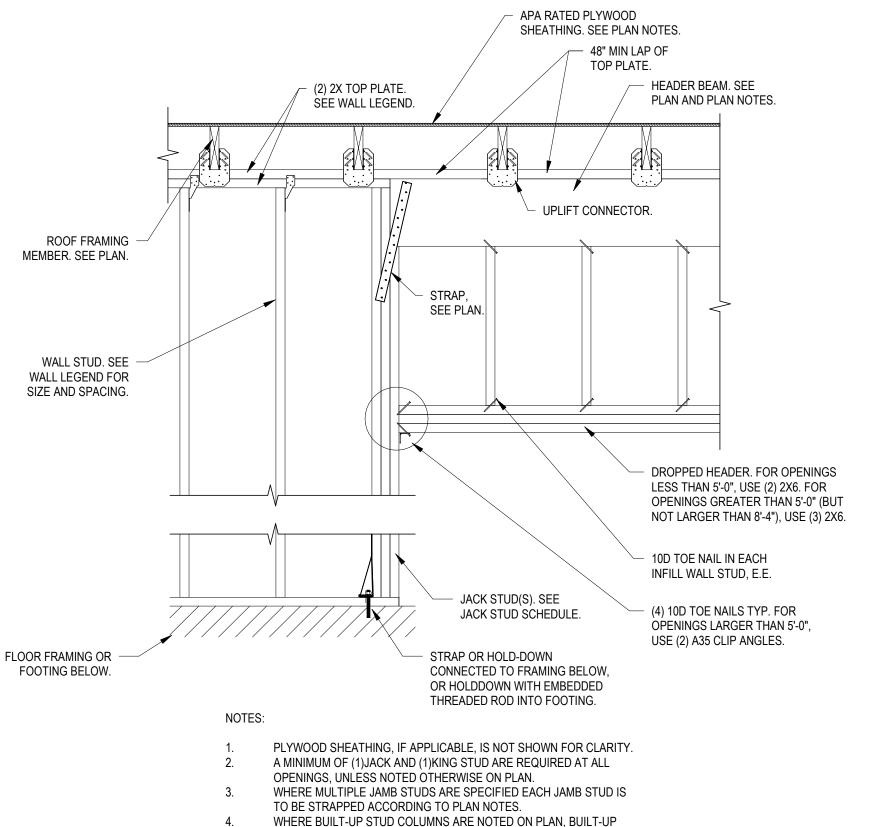
ON GRADE.

HOOKED TENSION DEVELOPMENT LENGTH (Idh) INCHES

3000 PSI

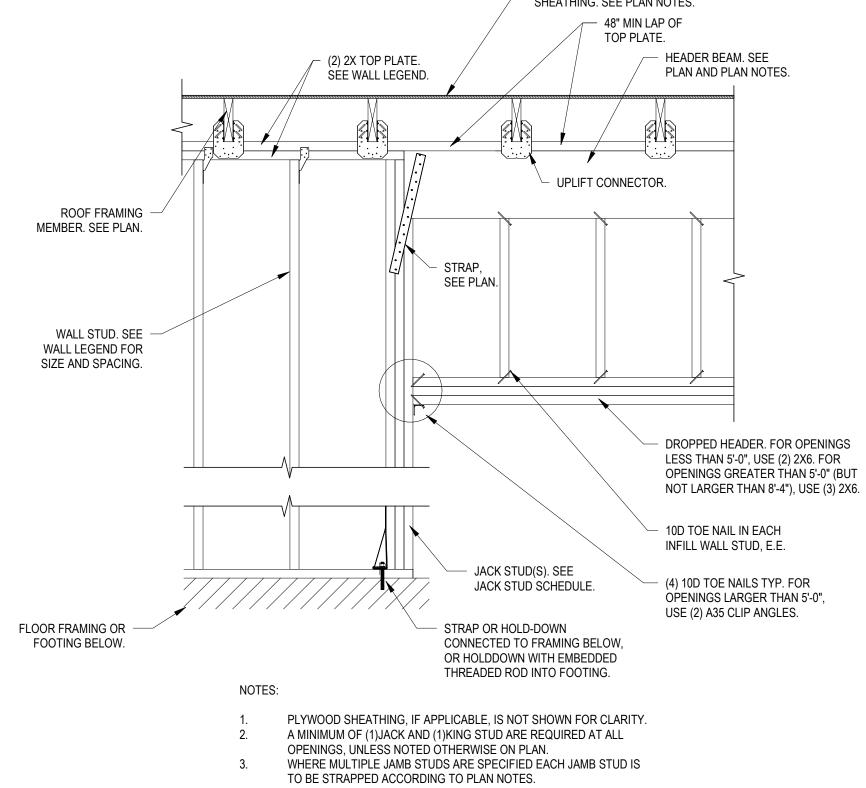
14

19

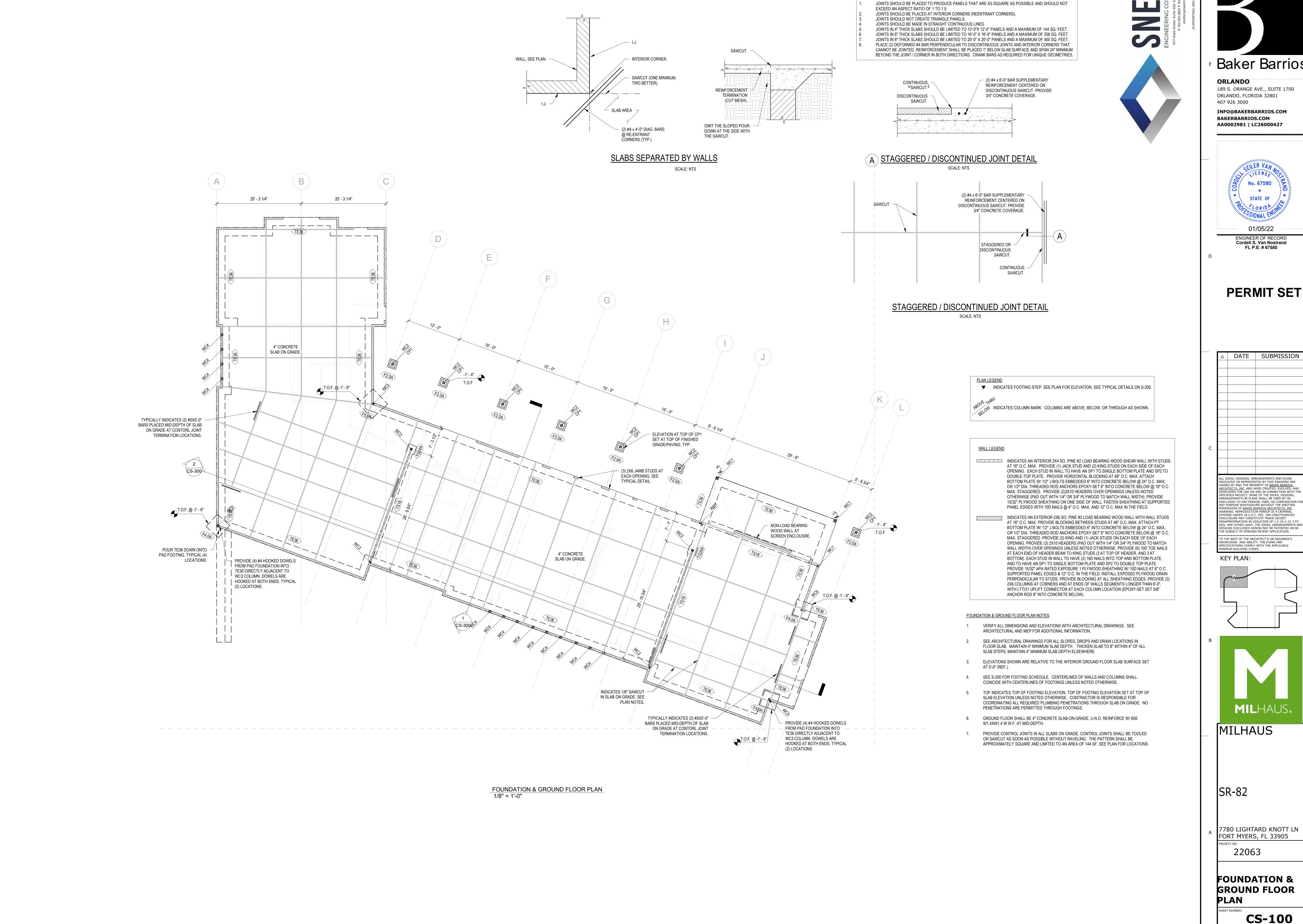


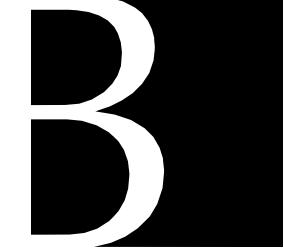
SCALE: NTS

TYP. HOOKED REINFORCEMENT TENSION



COLUMN INCLUDES JACK STUDS. TYPICAL WOOD WALL OPENING DETAIL

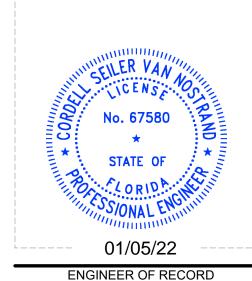




GUIDELINES FOR CONTROL JOINTS (CJ) IN SLABS-ON-GRADE:

Baker Barrios

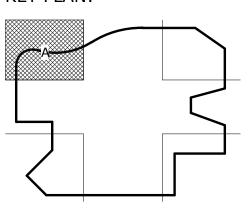
189 S. ORANGE AVE., SUITE 1700 ORLANDO, FLORIDA 32801 407 926 3000 INFO@BAKERBARRIOS.COM **BAKERBARRIOS.COM**



PERMIT SET

SUBMISSION

INDIC OWNE ARCH: DEVEI SPECI ARRAI DISCL ANY P PERMI WARN OFFEN DISCL	ATED OR REPRÉSEN ED BY AND THE PROI ITIECTS, INC. AND W LOPED FOR USE ON IFIED PROJECT. NON NGEMENTS OR PLAN OSED TO ANY PERS URPOSE WHATSOEV ISSION OF BAKER B, IIING: REPRODUCTIC NSE UNDER 18 U.S.C. OSURE MAY CONST	RANGEMENTS AND PLANS TED BY THIS DRAWING ARE PERTY OF BAKER BARRIOS IERE CREATED, EVOLVED, AND AND IN CONNECTION WITH THE E OF THE IDEAS, DESIGNS, S SHALL BE USED BY OR ON, FIRM, OR CORPORATION FOR IER WITHOUT THE WRITTEN ARRIOS ARCHITECTS, INC. IN HEREOF IS A CRIMINAL I. SEC. 506 UNAUTHORIZED ITUTE TRADE SECRET OLATION OF 1.C.24-2-31-1 ET.





MILHAUS

7780 LIGHTARD KNOTT LN FORT MYERS, FL 33905

22063

FOUNDATION & GROUND FLOOR

CS-100





ENGINEER OF RECORD Cordell S. Van Nostrand

Baker Barrios

189 S. ORANGE AVE., SUITE 1700

ORLANDO, FLORIDA 32801

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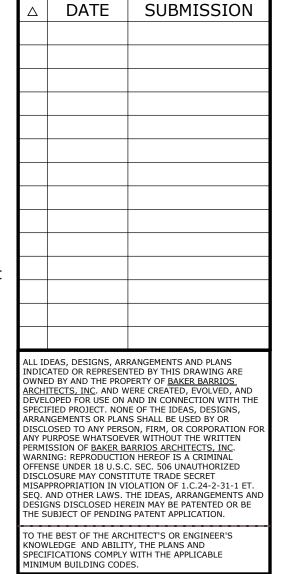
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ORLANDO

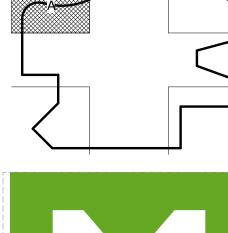
407 926 3000

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FL P.E. # 67580



KEY PLAN:





MILHAUS

INDICATES TRUSS UPLIFT (IN lbs), IN EXCESS OF 1000 lbs.

INDICATES TRUSS GRAVITY LOAD (IN lbs), IN EXCESS OF 5000 lbs.

☐ INDICATES PRE-ENGINEERED WOOD OVERFRAMING BY TRUSS

MANUFACTURER. OVERFRAMING ATTACHMENT TO PRE-ENGINEERED WOOD TRUSSES

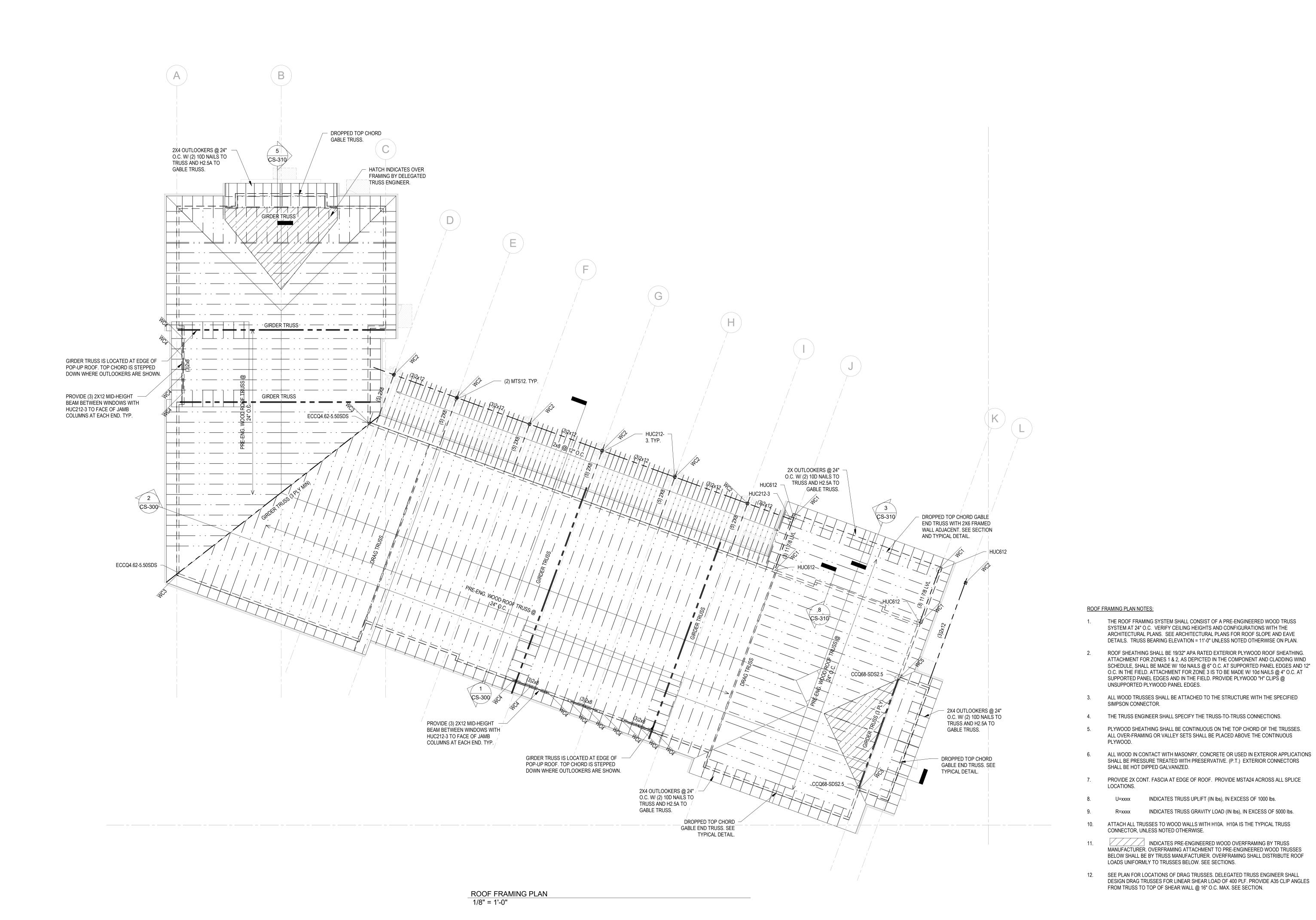
SR-82

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22063

ROOF FRAMING PLAN

CS-101







REMARKS

T.O. FTG. = T.O. SLAB. ENSURE B.O.

FOOTING IS A MIN. OF 12" BELOW

ADJACENT GRADE.

T.O. FTG. = T.O. SLAB.

T.O. FTG. = T.O. SLAB.

- DECORATIVE MASONRY COLUMN, BY OTHERS. MASONRY COLUMN IS FRAMED WITH (4) GROUTED AND REINFORCED MASONRY CELLS (16"X16" MAX

SIZE). DETAILING OF MASONRY, REINFORCEMENT,

GROUTING REQUIREMENTS, AND CONCRETE CAP

ELEMENT AT TOP OF COLUMN, IS BY OTHERS.

- 2'-0"X2'-0" X 16" DEEP CONCRETE FOOTING.

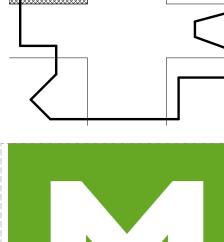
PROVIDE 3#5 E.W.

BOTTOM BARS.

	MILEN ER VAN
	No. 67580 * STATE OF O1/05/22
 	01/05/22

PERMIT SET

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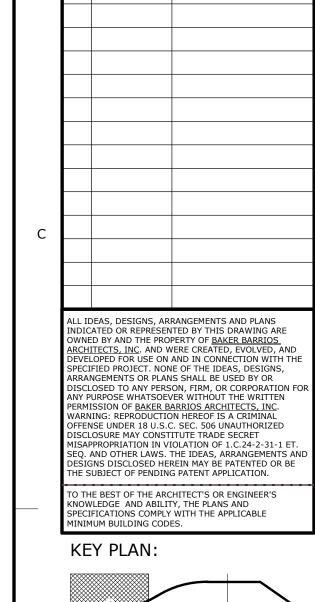
SR-82

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CS-200

Baker Barrios ORLANDO 189 S. ORANGE AVE., SUITE 1700 ORLANDO, FLORIDA 32801 407 926 3000 INFO@BAKERBARRIOS.COM **BAKERBARRIOS.COM** AA0002981 | LC26000427

No. 67580 * STATE OF O1/05/22
ENGINEER OF RECORD Cordell S. Van Nostrand FL P.E. # 67580

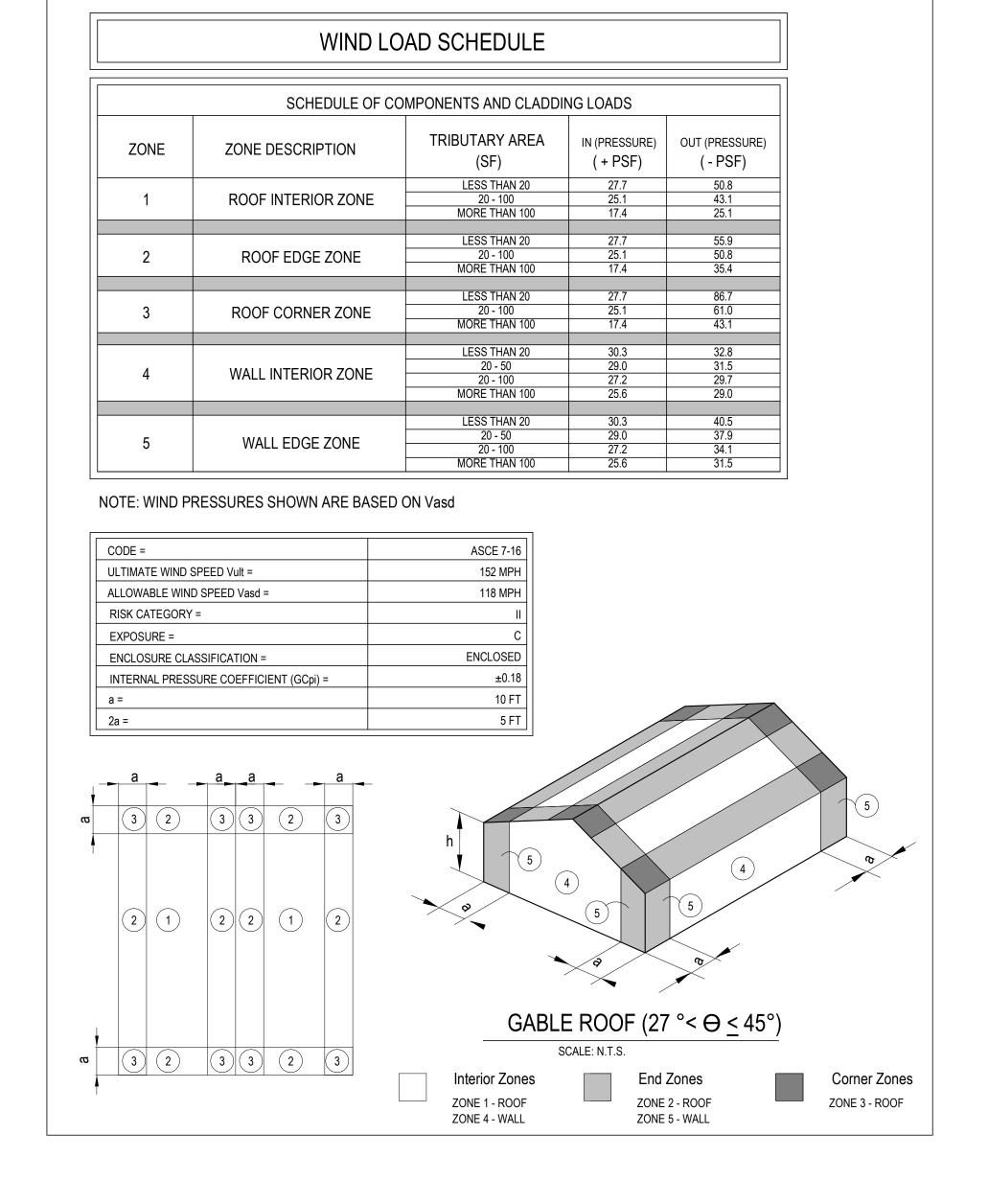


MILHAUS

22063

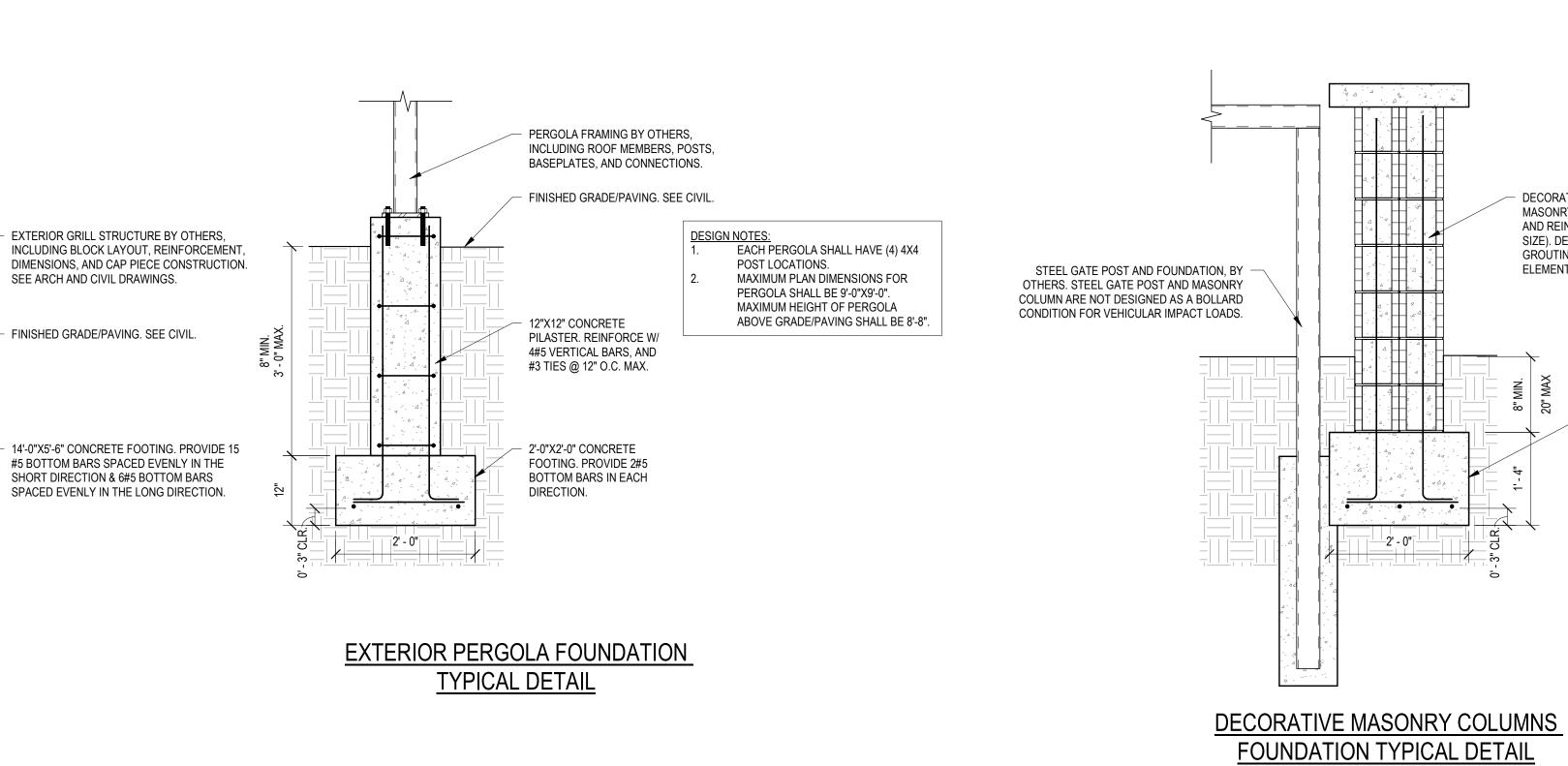
SCHEDULES

FOUNDATION SCHEDULE WOOD COLUMN SCHEDULE 2' - 0" 1' - 0" 4' - 0" 1' - 0" 2#5 BOT. E.W. 4' - 0" 4#5 B.E.W. 5' - 0" 5' - 0" 1' - 1" 6#5 B.E.W. SEE PLAN FOR TOP ATTACHMENT. SEE PLAN FOR TOP ATTACHMENT. 2' - 0" CONT. 1' - 6" TE36 3#5 CONT. 5 1/4x5 1/4 PSL HDU8-SDS2.5 SEE PLAN FOR TOP ATTACHMENT. ATTACH TO TOP PLATE W/ SP6. (3) 2X8 HIGH BEAMS ATTACH TO EACH COLUMN 2' - 0" W/ HUC28-3. TYP. 2' - 0" 1' - 0" 2#5 BOT. E.W. SEE PLAN FOR TOP ATTACHMENT. FOUNDATION SCHEDULE NOTES: PROVIDE CORNER BARS AT ALL FOOTING INTERSECTIONS. PROVIDE 3" CLEAR COVER FOR ALL REINFORCING UNLESS DETAILED OTHERWISE. GC SHALL COORDINATED WITH CIVIL FOR FINISHED GRADES. IN NO CASE SHALL FOUNDATIONS BE LESS THAN 12" BELOW ADJACENT GRADE, EXCEPT FOR NON-LOAD BEARING TE FOOTINGS. CONCRETE COLUMN SCHEDULE



EXTERIOR ELECTRIC GRILL

FOUNDATION TYPICAL DETAIL



6X6 PT

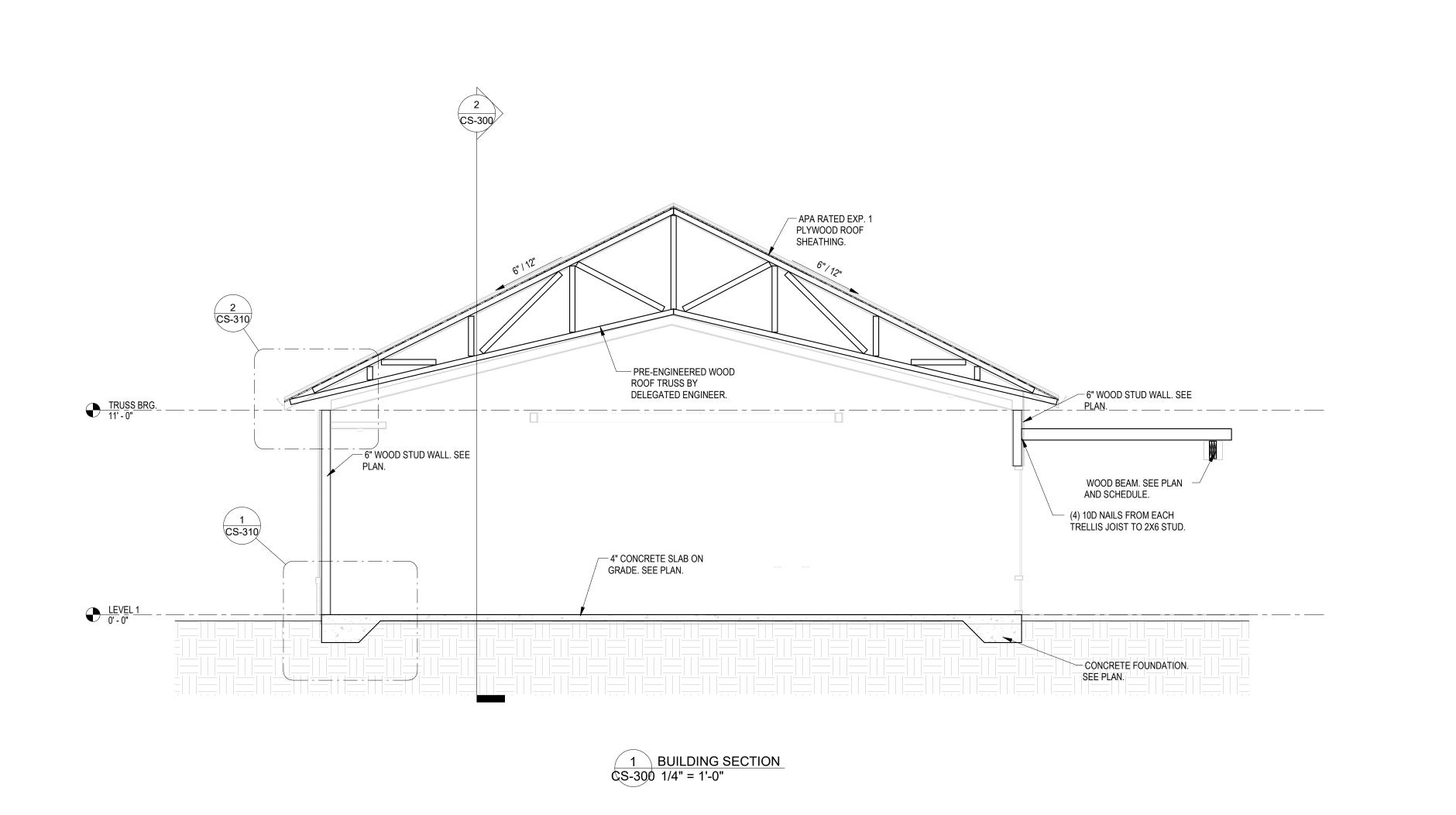
SIZE (BxH) REINF. SPACING TIE CONFIG. REMARKS

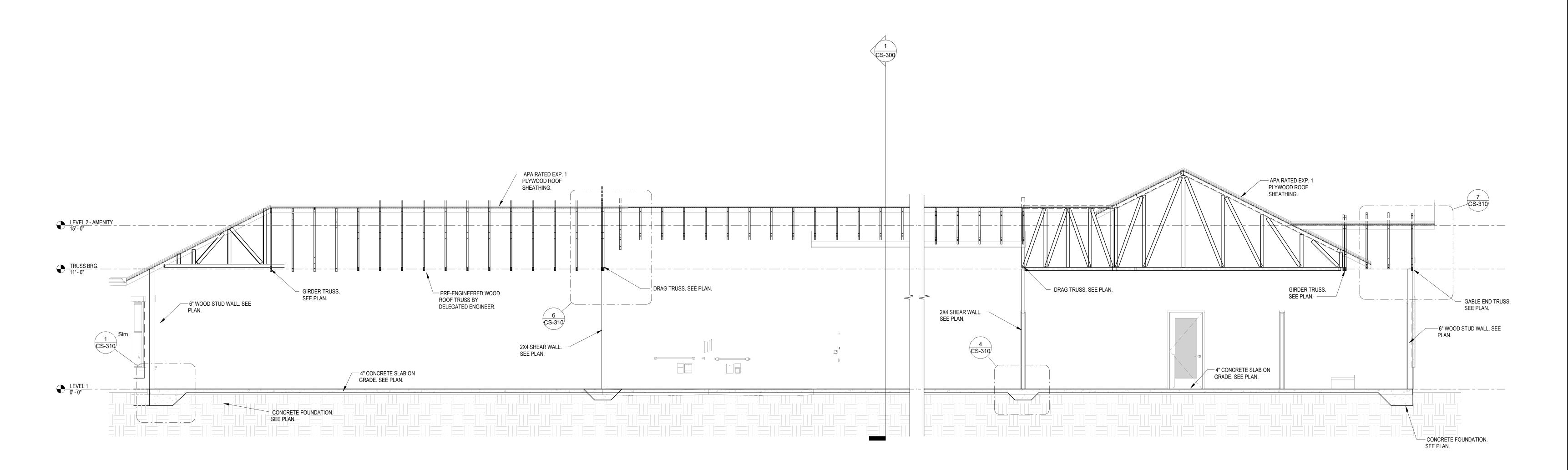
CONCRETE COLUMN TIE CONFIGURATIONS

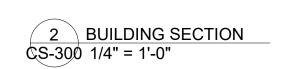
CP1 16 X 16 8#6 #3@12" O.C. TYPE A

WC4

WC5







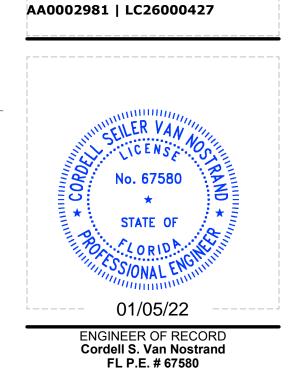


ORLANDO, FLORIDA 32801

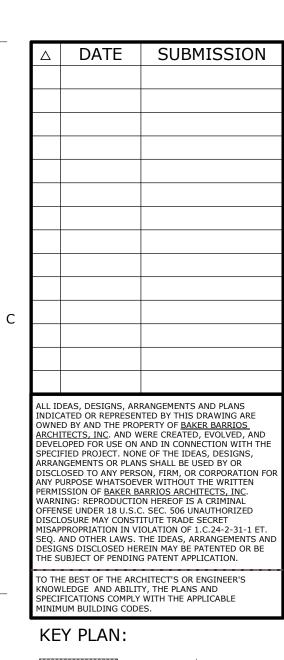
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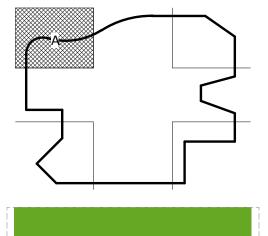
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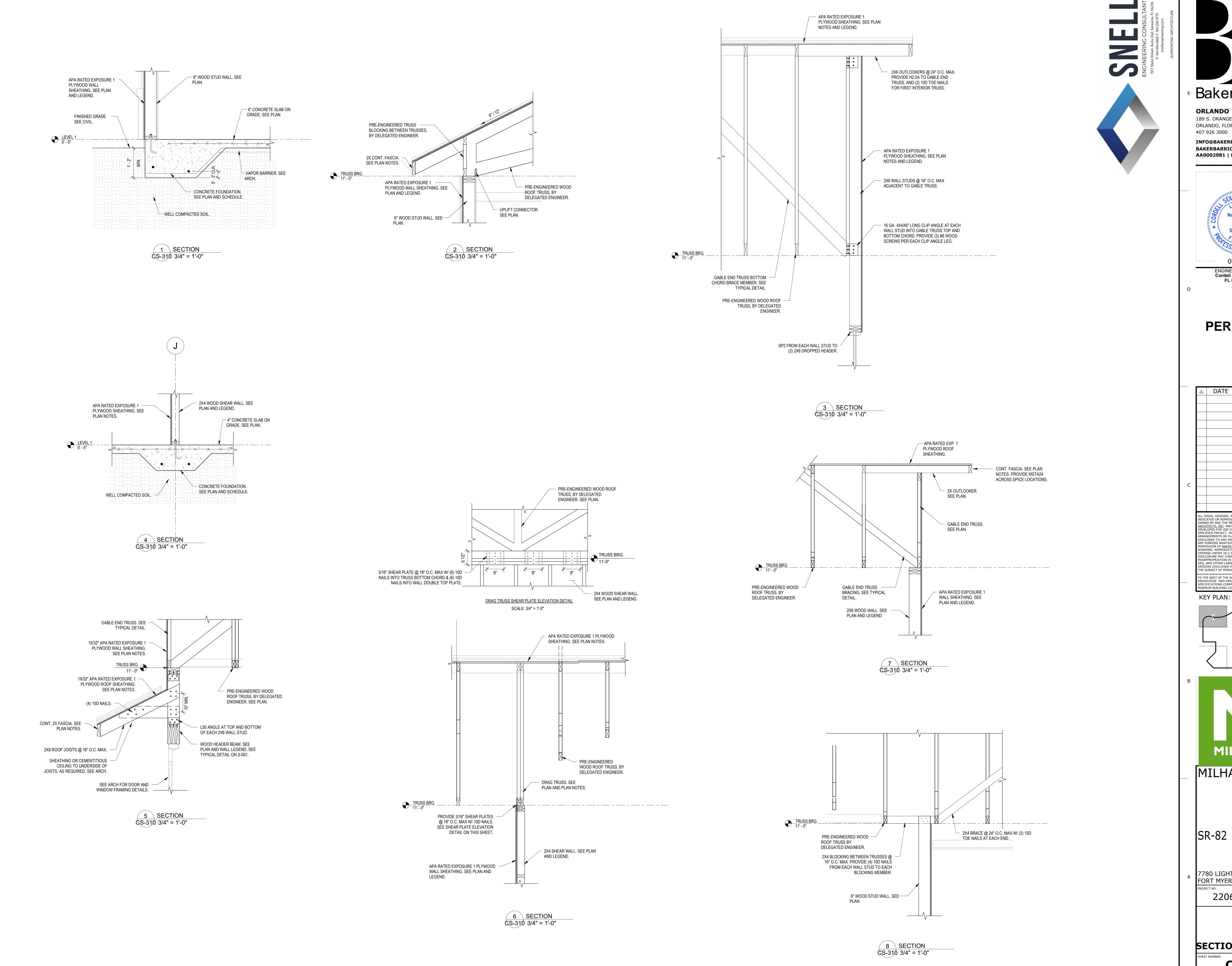
MILHAUS

SR-82

7780 LIGHTARD KNOTT LN FORT MYERS, FL 33905
PROJECT NO:
22063

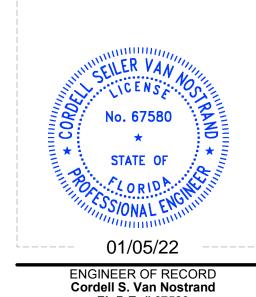
BUILDING SECTIONS

CS-300



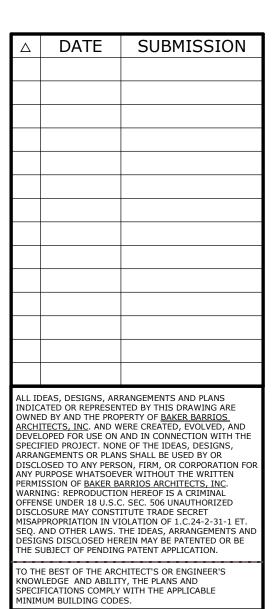
Baker Barrios

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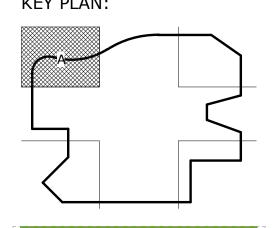


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FL P.E. # 67580



KEY PLAN:





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SECTIONS

CS-310