### Oasis at Surfside - Phase II 1.0 CODES AND STANDARDS Cape Coral, Florida 1.1 "2020 Florida Building Code" and "International Building Code", 2018. 1.2 "Minimum Design Loads for Buildings and other Structures" SEI/ASCE 7-16.

- 1.4 "Manual of Standard Practice", Concrete Reinforcing Steel Institute, latest edition.
- 1.5 "Building Code Requirements for Masonry Structures", ACI 530-13, ASCE 5-13, TMS 402-16.
- 1.6 "National Design Specification for Wood Construction," AF&PA NDS-2018.
- 1.7 To the best of my knowledge the plans and specifications comply with the minimum building codes Adam Sisk, PE
- 2.0 DESIGN LOADS: Project Located in: City of Cape Coral, County of Lee, State of Florida.
- 2.1 Gravity Loads: (Reduced where allowed)

GRAVITY LOADS				
Location	Location Uniform (psf)			
Roof Loads:				
Dead Load	20			
Live Load	20	300		
Floor Loads:				
Dead Load	35 (includes partition and gypcrete)			
Floor Live Loads:				
Public Rooms and Corridors Serving them	100			
Private Rooms and Corridors Serving them	40			
Mechanical & Electrical Rooms	150			
Storage	125			

2.2 Drifting Snow Loads per (IBC 2018).

$$\begin{array}{rrrr} Pg &=& 0 \ psf \\ I &=& 1.0 \\ Ce &=& 1.0 \\ Ct &=& 1.0 \end{array}$$

2.3 Risk Category = II

2.4 Wind Loads per (IBC 2018) & ASCE 7-16 (3-second gust)

Main Wind Force Resisting System: V 157 mph

Exposure Category "C"

Building is enclosed & Internal Pressure coefficient (GCpi) = +0.18 & -0.18Topographic Factor Kzt = 1.0 Wind Directionality Factor, Kd = 0.85

Calculated Wind Base Shear (For MWFRS)

Main Building = Vx = 529k Vy = 658k

Components and Cladding: V 157 mph

Exposure	Category	"С"	

			Compon	ents and C	laddin
Walls	Area =	= 10ft <sup>2</sup>	Area =	$Area = 20ft^2$	
Zone 4	69.3	-74.9	66.1	-72.0	62.
Zone 5	69.3	-92.6	66.1	-86.3	62.
	Area = $10ft^2$				
Roof	Area =	= 10ft <sup>2</sup>	Area =	= 20ft <sup>2</sup>	A
Roof Zone 1	Area = 28.1	= 10ft <sup>2</sup> -110.3	Area = 26.3	= 20ft <sup>2</sup> -103.0	A 24.
Zone 1	28.1	-110.3	26.3	-103.0	24.

Notes:

Areas noted are effective wind areas as per ASCE 7-16, 26.2 definitions. See figures below for Zone locations.

- Plus and minus signs signify pressures acting toward and away from surfaces, respectively.
- Design pressures shown in table are strength design wind pressures. Allowable stress design wind pressures may be calculated by factoring the pressures by 0.6.
- 5. Design pressures for effective wind areas between those noted in schedule may be
- interpolated. Tributary area = greater of LxW or LxL/3. Deflections may be calculated based on 42% of these loads.

2.5 2.5 Seismic Loads per (IBC 2018) & ASCE 7-16 (3-second gust) Risk Category = II

Site class = "D" (Per Geotechnical Report) Spectral Response Coefficients:

$$SDS = 0.057g$$
  
 $SD1 = 0.04g$ 

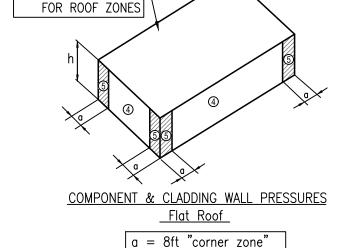
- Cs = 0.0088Seismic Design Category = A Seismic Importance Factor = 1.0 Basic Seismic - Force - Resisting System
- Bearing Wall System Wood Framed Walls Sheathed with Wood Structural Panels RX=RY=6.5,  $\Omega X=\Omega Y=3.0$ , CD X=CD Y=4.0Design Base Shear
- Main Building = Vx = Vy = 35k

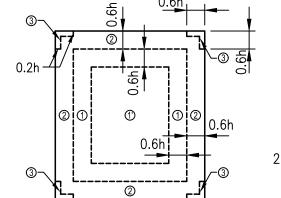
Building Height Limit = NL Analysis Procedure - 12.8.1 ASCE 7-16 Equivalent Lateral Force Procedure

2.6 Guardrail designed per International Building Code, Section 1607.8

Guardrail: Uniform load = 50 plf, any direction - per 1607.8.1 Concentrated load = 200 lbs, any direction - per 1607.8.1.1Intermediate Rail: (all those expect handrail) per 1607.8.1.2

2.7 Flood Loads: Project is located in AE7 flood zone.





COMPONENT & CLADDING ROOF PRESSURES Flat Roof h = 49ft "mean roof height"

# SEE FIGURE TO RIGHT

2. Contractor is to notify architect immediately of conditions or items varying from depicted information.

1. All drawings are to be coordinated with all site information by owner and contractor, and applicable codes. 3. Planworx Architecture, P.A. is not responsible for constructed variations from the information depicted

- 1.3 "Building Code Requirements for Structural Concrete (ACI 318-14)" American Concrete Institute 2014.



- ng Wind Pressure (psf)  $Area = 50ft^2$  $Area = 100 ft^2$  $Area = 500 ft^2$ -67.9 58.9 -64.7 58.9 -64.7 58.9 -72.0 2.0 -78.1 58.9 -72.0  $Area = 50ft^2$  $Area = 100ft^2$  $Area = 500ft^2$ -93.4 22.4 -86.1 22.4 -86.1 -63.4 22.4 -63.4 22.4 -63.4 22.4 -114.4 22.4 -114.4 -123.6 1 -198.1 26.3 -179.5 24.1 -154.8 22.4 -136.0 22.4 -136.0

- 3.0 FOUNDATIONS:
- 3.1 Foundation design is based on geotechnical report# 18-225 by Velocity Engineering Services of Fort Meyers, FL dated September 11, 2018. This report is available for inspection at the office of the architect or owner. The recommendations contained in this report are herein made part of the requirements of these contract documents.
- 3.2 Top of footing (T/FTG) elevations are shown on the drawings or are to be determined by the Contractor in the field in accordance with the guidelines set forth in the drawings.
- 3.3 Bottom of exterior footings, grade beams and walls shall bear at a minimum depth of 1'-6" below final grade per geotechnical report.
- 3.4 Testing and Inspection:
  - a. All areas to have slabs on grade shall be proof rolled in accordance with and under observation fo the Geotechnical Engineer and approved prior to preparation for concrete placement.
  - b. All foundation bearing strata shall be inspected and approved by the Geotechnical Engineer prior to any concrete placement.
  - c. Geotechnical Engineer shall be the sole judge as to suitability of all foundation and/or slab bearing strata.
  - d. Footing bearing elevations shall be adjusted in the field as required to meet the design bearing pressures by additional excavation or compaction and/or backfilling or by other means acceptable to the Geotechnical Engineer.
- 3.5 Undercutting to remove existing fill beneath footings and slab shall be performed at the direction of the Geotechnical Engineer.
- 3.6 Footings shall bear on strata capable of sustaining a minimum bearing pressure of 2,500 psf.
- 3.7 Engineered Fill: All fill material shall be selected in accordance with the Geotechnical Report Material shall be a clean, low plastic soil with a plasticity index less than 30 (less than 15 is preferred), liquid limit less than 50, and unit weight of 120 pcf (+ 5 pcf)
- 3.8 Compaction: All fill shall be placed in loose lifts not exceeding 8 inches in thickness and compacted to a minimum of 95 percent Standard Proctor (ASTM D-698) except that the top 12 inches shall be compacted to a minimum of 96 percent Standard Proctor. Moisture shall be controlled to within 3 percent above or below optimum content.
- 3.9 Contractor shall review all construction considerations as outlined in the Geotechnical report and bid accordingly.
- 4.0 CONCRETE:
- 4.1 Concrete Strength: All concrete shall be in accordance with the American Concrete Institute (ACI) 301 and 318.
- 4.2 Concrete shall have a 28 day compressive strength and density as follows:

a.	Footings and Interior Slab-on-grade		F
b.	Elevated Slab on Decks	4,000psi, pea gravel mix, Density = ±145pcf	
с.	Exterior Slab on Grade	4,000psi, Density = $\pm 145$ pcf	
d.	CMU Grout Fill		F
		Slump 8"-11" or grout per Structural Masonry	,
		Notes, this sheet.	6
	b. c.	<ul><li>b. Elevated Slab on Decks</li><li>c. Exterior Slab on Grade</li></ul>	

- 4.3 Concrete Mix Designs: a. Submittals: Submit written reports of each proposed concrete mix not less than 15 days prior to the start of work.
  - b. Mix designs, including water, cement ratios and slumps, shall be prepared in accordance with ACI 301-05, Section 4, Cement shall conform to ASTM C 150 Type 1 or at contractor's option, ASTM C 595 Type IP where fly ash is permitted. Normal weight aggregate shall conform to ASTM C 33 and light weight aggregate shall conform to ASTM C 330. No admixtures containing calcium chloride shall be permitted in any concrete.
  - c. Aggregate size shall be #67 stone for supported slabs or other formed concrete elements; #57 stone for slabs on arade and footinas or other concrete elements formed from and poured against earth;
  - #89 stone for masonry grout. d. Water reducing admixture shall be used in all concrete.
  - e. Air entraining admixture in accordance with ACI 301 shall be used in all concrete exposed freezing and thawing during construction or service conditions.
  - f. Concrete subjected to freezing/thawing shall have a maximum water/cement ratio of 0.45 and shall contain the amount of air entraining agent specified in ACI 301-05 Section 4.
- 4.4 Curing:
- See specifications for curing method options and apply within two (2) hours after completion of finishing to all concrete flatwork and walls, U.N.O., other than footings and grade beams.
- 4.5 Use a non-corrosive, non-chloride accelerating admixture in concrete exposed to temperatures below 40 degrees. Uniformly heat the water and aggregates to a temperature of not less than 50 degrees. Place and cure concrete in accordance with ACI 306.
- 4.6 When hot weather conditions exist, place and cure concrete in accordance with ACI 301. Cool ingredients before mixing to maintain concrete temp. at time of placement below 90 degrees.
- 4.7 Reinforcing in all abutting concrete, including footings shall be continuous through or around all corners or intersections. Dowels or splices shall be equal in size and spacing to the reinforcing in the abutting members.
- 4.8 Refer to architectural drawings for door and window openings, drips, reglets, washes, masonry anchors, brick ledge elevations, slab depressions and miscellaneous embedded plates, bolts, anchors, angles, etc.
- 4.9 Refer to plumbing, mechanical and electrical drawings for underfloor, perimeter and other drains and for sleeves, outlet boxes, conduit, anchors, etc. The various trades are responsible for their items.
- 4.10 Base plates, anchor rods, support angles and other steel exposed to earth or granular fill shall be covered with a minimum of 3" of concrete.
- 4.11 Fill slabs, not shown on the structural drawings, shall be reinforced with a minimum of 6 x 6 x W2.0 x W2.0 WWM unless noted otherwise on other drawings.
- 4.12 Finish surfaces to the following tolerances, according to ASTM E 1155, for a randomly trafficked floor surface: a. Specified overall values of flatness, F(F) 25; and of levelness, F(L) 20; with minimum local values equal to  $\frac{3}{4}$  of the overall flatness and levelness values.
- b. The composite F(F) and F(L) numbers shall be measured and reported within 72 hours after completion of slab concrete finishing operations and before removal of any supporting shores.
- 4.13 Non-shrink grout shall be pre-mixed, non-corrosive, non-metallic, non-staining containing silica sands, Portland cement, shrinkage compensating and water reducing agents. Product shall only require the addition of water. Minimum compressive strength shall be 2500 psi after one day and 7000 psi after 28 days. Grout shall be free of gas producing or air releasing and oxidizing agents and contain no corrosive iron, aluminum or gypsum.
- 4.14 Provide concrete grout not mortar for reinforced masonry lintel and bond beams where indicated on drawing or as scheduled.
- 4.15 Tolerance for anchor rods and other embedded items shall be per the AISC Code of Standard Practice Section 7.5.
- 4.16 Unless otherwise shown in the architectural drawings, provide 3/4-inch chamfers at all column, wall, slab or beam edges that are exposed to view in the finished structure.

6.2 Concrete Masonry Units (CMU): a. Units shall be lightweight cellular units conforming to ASTM C 90, Grade N-2. Concrete masonry net area exceeding 95 pcf. b. Design compressive strength of CMU (fm) = 2,000psi. 6.3 Mortar shall conform to ASTM C 270. Mortar shall be type "S" and shall conform to the ASTM C270 proportion requirements. 6.4 Neither type "N" mortar nor masonry cement shall be used as part of the lateral force resisting system. 6.5 Grouting: grout with pea gravel concrete masonry fill, see note 4.2 this sheet. b. All bond beams shall be filled with grout and reinforced as indicated on the drawings (details or schedules). Mortar fill is not permitted. unless specifically noted otherwise on the drawings. Unreinforced walls indicated as grouted shall be grouted full height, unless specifically noted otherwise. Mortar fill is not permitted. drawings. Mortar fill is not permitted. e. Vertical grouting shall be low lift or high lift as follows: the Contractor. Lifts shall not exceed 4'-0" in height. (2) High lift grouting is permissible only for filling of cellular masonry units and shall not exceed 12'-8" in height. Clean out holes shall be provided at the base of each grouted cell. Grouting shall be stopped 1-1/2" below the top of a course to form a key at the joint. a. Grouting of masonry beams or lintels shall be done in one continuous operation.

5. Planworx Architecture, P.A. retains ownership of all of designs depicted and implied herein.

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4.17 Concrete cover for cast-in-place concrete reinforcement: Concrete cast against & permanently exposed to earth: Concrete exposed to earth or weather:	3 Inches
No. 6 through No. 18 Bars:	
No. 5 Bar and smaller:	1½" Inches
Concrete not exposed to weather or in contact with ground:	
Slabs, Walls, Joists:	
No. 11 Bar and smaller:	¾" Inches
Beams, Columns:	
Primary Reinforcement, Ties, Stirrups:	1½" Inches

#### 5.0 REINFORCING STEEL:

5.1 Reinforcing shall be domestic new billet steel conforming to ASTM A615, Grade 60 or 60S including stirrups and ties, except that reinforcing which is required to be welded shall conform to ASTM A706.

5.2 Field bending of concrete reinforcing steel is not permitted.

5.3 Welded wire mat and fabric shall conform to ASTM A184 and A185 respectively and shall be provided in flat sheets. Welded wire mat/fabric shall be lapped 0'-6" at all splices.

#### 5.4 Bar Splices:

	f'c = 3,000psi			f'c = 4,000psi	f'c = 5,000psi		
Bar Size	ar Size Ld (in) Class "B" Lap Splice (in)		Ld (in) Class "B" Lap Splice (in)		Ld (in)	Class "B" Lap Splice (in)	
#3	17	22	15	19	13	17	
#4	22	29	19	25	17	23	
<b>#5</b>	28	36	24	31	22	28	
#6	33	43	29	37	26	34	
#7	48	63	42	54	38	49	
#8	55	72	48	62	43	56	

Values are based on normal weight concrete.

Ld = minimum embed of rebar Class "B" lap splice refers to minimum distance bars

must be lapped for a full tension splice.

6.0 STRUCTURAL MASONRY:

6.1 All structural masonry shall conform to ACI 530 standards as appropriate to the material.

- unit strength shall be no less than 2,000psi in accordance with ASTM C 140, with a unit weight not
- a. Grout shall conform to ASTM C476 as specified by proportion. Masonry grout shall conform to the ASTM proportion requirements for coarse grout with a slump of 8 to 11 inches. Contractor may substitute
- All masonry wall cells or cavities indicated as reinforced shall be grouted for the full height of the wall,
- d. All masonry cells or cavities below grade shall be grouted solid unless specifically noted otherwise on the
- (1) Low lift grouting shall be used for all cavity walls and may be used for all walls at the option of

- h. Consolidate pours with mechanical vibrator and reconsolidate by mechanical vibration after initial water loss and settlement has occured.
- i. Mechanical vibrator shall be a low velocity vibrator with a  $\frac{34}{4}$ " head.

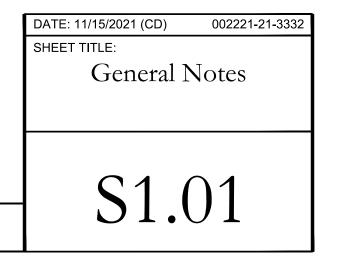
#### 6.6 Masonry Reinforcing:

- a. Foundation dowels may slope a maximum of 1:6 to align with wall cavities or vertical CMU cores. Greater slopes will require replacement of the foundation dowels.
- b. Spliced reinforcing shall be lapped a length calculated per IBC 2107.5 OR 15" OR as shown on drawings, whichever is greatest. All splices shall be wired together.
- c. Vertical reinforcing bars shall have a minimum clearance of 3/4" from masonry and shall be held in position top and bottom and at intervals not exceeding 4'-0". Accessories for such support shall be used. Provide "AA Wire Products Company" (or approved equal) Rebar Positioner AA225 or AA239 for vertical bars and AA238 for horizontal bars or approved equal products from other suppliers.
- d. Horizontal joint reinforcing shall be lapped no less than 6" all splices, including corners and tees where no control joint is used.
- e. All horizontal joint reinforcing shall stop at control joints. f. Horizontal reinforcing in bond beams shall be continuous through control joints.
- q. All CMU walls shall have joint reinforcing @ 16"o.c. All joint reinforcing shall have (2) 9 gauge
- (0.148"ø or W1.7) side rods & cross rods @ 16"o.c.

6.7 Masonry contractor shall provide for and coordinate with other trades for placement of all items to be embedded or built into the masonry.

MINIMUM SPLICING LENGTH (Ld) FOR MASONRY				
BAR SIZE SPLICE LENGTH				
#3	16"			
#4	22"			
#5 26"				
#6	43"			
#7	60"			





- 7.0 GENERAL FRAMING NOTES:
- 7.1 -All exterior and interior load bearing walls shall be as noted or approved equals: 2x4 SPF #2, 2x6 SPF #2
  - -All interior non-load bearing wall, shall be SPF #2, or approved equal. -All top plates shall be SYP #2 or better. All sill plates shall be SYP#3 or better. Pressure treat all sill plates in contact with concrete or masonry.
  - Standard U1 to the requirements of Use Category 3B (UC3B) or Use Category 4A (UC4A). -All pressure treated Glulam (GL) members shall be Rosboro Treated X-Beam 2400 $F_{h}$ -1.9E or approved
  - equal. -All Glulams shall be Rosboro 24F-V4 or better.
- review prior to fabrication. The shop drawings shall show the following: — Layout plan
  - Bearing locations
  - Truss elevations
  - Mechanical openings Structural calculations
  - North Carolina professional engineer seal to certify design - Hurricane clips and tie downs
- 7.3 Floor deck/diaphragm
  - Place long direction perpendicular to framing
  - Stagger end joints
  - Glue and nail panels down with 10d common
  - Provide the following nail pattern:
  - @ 6" O.C. @ panel edge @ 12" O.C. @ interior of panel.
- 7.4 Roof Deck/Diaphragm
  - Roof sheathing shall be 5/8-inch exterior grade plywood - Place long direction perpendicular to framing
- Stagger end joints
- 7.5 Wall Sheathing - Exterior wall Sheathing shall be 15/32 exterior grade plywood or OSB.

  - Nail panels with 8d or 10d common OSB as noted in schedules.
  - All horizontal edges of exterior wall and shear wall sheathing shall be blocked -
  - see details on S5.0 series sheets Shear walls
  - @ 3" O.C. @ panel edge
  - @ 12" O.C. @ interior of panel.
  - Exterior walls @ 3" O.C. @ panel edge
  - @ 12" O.C. @ interior of panel.
- 7.6 See plan for location of Shear Walls and S5.0X Sheet Series for framing requirements.
- practices shall be used in all cases.
- shall be hot-dipped galvanized.
- 7.9 Do not bend coil straps.
- 2018.

-See plans and load bearing wall schedule for locations, spacing, and load bearing studs species.

-All pressure treated 2x material shall be SYP #2 or better and shall be treated in accordance with AWPA -All pressure treated Parallam shall be Truss Joist MacMillan, Wolmanized Parallam PSL, or approved equal.

-All Laminated Veneer Lumber (LVL) shall be Louisiana Pacific, Gang-Lam 3100F<sub>b</sub>-2.0E or approved equal.

7.2 All roof and floor trusses shall be Builders First Source or approved equal. Truss supplier shall construct trusses to provide full bearing on all walls and girders. The truss supplier shall also submit drawings for

- Floor deck shall be 3/4-inch exterior grade tongue and groove Advantech

- Provide roof sheathing clips, Simpson PSCL/PSCA or approved equal at all unsupported edges. - Nail to supporting members with 10d @ 6" o.c. edges and 12" o.c. field..

- Interior shear wall sheathing shall be  $\frac{1}{16}$  or  $\frac{15}{32}$  plywood or OSB as noted in schedules. - Shear wall sheathing may be placed either horizontal or vertical and stagger end joints

7.7 [X] number in box notes the required number of bundled studs in that location. Bundled studs shall rest on framing member below or provide solid blocking from sub-floor to plate or girder below. Good framing

7.8 All strap and tie connections shall have z-max (g185) triple zinc coating (or hot-dipped galvanized). All nails

7.10 Unless noted otherwise, connect all building components per table 2304.9.1 - fastening schedule, per IBC

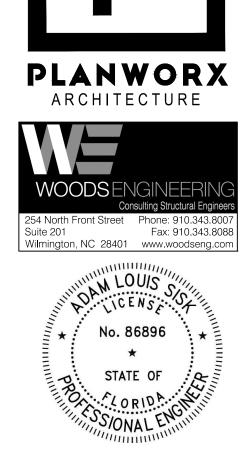
- 8.0 POST-INSTALLED ANCHORS:
- 8.1 Except where indicated on the drawings, post-installed anchors shall consist of the following anchor types as provided by HILTI, Inc. Contact HILTI at (800) 879-8000 for product related questions.
  - Anchorage to Concrete a. Adhesive anchors for cracked and uncracked concrete use:
    - 1. HILTI HIT-HY 200 Safe Set System with HILTI HIT-Z Rod per ICC ESR-3187 (pending) 2. HILTI HIT-HY 200 Safe Set System with HILTI Hollow Drill Bit System with HAS-E threaded rod
    - per ICC ESR-3187.
    - 3. HILTI HIT-RE 500-SD Epoxy Adhesive Anchoring system with HAS-E threaded rod per ICC ESR-2322 for slow cure applications.
  - b. Medium duty mechanical anchors for cracked and uncracked concrete use: 1. HILTI KWIK HUS EZ and KWIK HUS EZ-I Screw Anchors per ICC ESR-3027
    - 2. HILTI KWIK BOLT-TZ Expansion Anchors per ICC ESR-1917
    - 3. HILTI KWIK Bolt 3 Expansion Anchors (uncracked concrete only) per ICC ESR-2302
  - c. Heavy duty mechanical anchors for cracked and uncracked concrete use:
  - I. HILTI HDA Undercut Anchors per ICC ESR 1546 2. HILTI HSL-3 Expansion Anchors per ICC ESR 1545
  - Rebar Doweling into Concrete
  - a. Adhesive anchors for cracked and uncracked concrete use:
    - 1. HILTI HIT-HY 200 Safe Set System with HILTI Hollow Drill Bit System with continuously
    - deformed rebar per ICC ESR-3187. 2. HILTI HIT-RE 500-SD Epoxy Adhesive Anchoring System with continuously deformed rebar per ICC ESR-2322.
  - Anchorage to Solid Grouted Masonry
  - a. Adhesive Anchors use: 1. HILTI HIT-HY 70 Masonry Adhesive Anchoring System (ICC pending).
    - 2. Steel anchor element shall be HILTI HAS-E continuously threaded rod or continuously deformed
  - steel rebar. b. Mechanical Anchors use:
    - 1. HILTI KWIK BOLT-3 Expansion Anchors per ICC ESR 1385.
  - Anchorage to Hollow/Multi-wythe Masonry
  - a. Adhesive Anchors use: 1. HILTI HIT-HY 70 Masonry Adhesive Anchoring System per ICC ESR-3342.
    - 2. Steel anchor element shall be HILTI HAS-E continuously threaded rod or continuously deformed steel rebar.
    - 3. The appropriate size screen tube shall be used per adhesive manufacturer's recommendation.
- 8.2 Anchor capacity used in design shall be based on the technical data published by HILTI or such other method as approved by the Structural Engineer of record. Substitution requests for alternate products must be approved in writing by the Structural Engineer of record prior to use. Contractor shall provide calculations demonstrating that the substituted product is capable of achieving the performance values of the specified product. Substitutions will be evaluated by their having and ICC ESR showing compliance with the relevant building code for seismic uses, load resistance, installation category, and availability of comprehensive installation instructions. Adhesive anchor evaluation will also consider creep, in-service temperature and installation temperature.
- 8.3 Install anchors per the manufacturer instructions, as included in the anchor packaging.
- 8.4 Overhead adhesive anchors must be installed using the HILTI PROFI System.
- 8.5 The contractor shall arrange an anchor manufacturer's representative to provide onsite installation training for all of their anchoring products specified. The Structural Engineer of record must receive documented confirmation that all of the contractor's personnel who install anchors are trained prior to the commencement of installing anchors.
- 8.6 Anchor capacity is dependant upon spacing between adjacent anchors and proximity of anchors to edge of concrete. Install anchors in accordance with spacing and edge clearances indicated on the drawings.
- 8.7 Existing reinforcing bars in the concrete structure may conflict with specific anchor locations. Unless noted on the drawings that the bars can be cut, the contractor shall review the existing structural drawings and shall undertake to locate the position of the reinforcing bars at the locations of the concrete anchors, by HILTI FERROSCAN, GPR, X-ray, chipping or other means.
- 9.0 CONSTRUCTION AND SAFETY:
- 9.1 Woods Engineering P.A.'s responsibility is limited to the details and information shown on these drawings. It is the responsibility of the Contractor to provide adequate safety measures required by local codes as well as OSHA Standards for the Construction Industry.

This should include, but not be limited to the following: Shoring to protect new as well as existing structures.

- Necessary Scaffolding.
- Material Handling Equipment. Trench Boxing.
- 10.0 SHOP DRAWING SUBMITTAL
- 10.1 See Project Manual
- 10.2 Contractor shall submit Electronic copies (PDF format) of each shop drawing for review. Shop drawings shall be reviewed by the Contractor prior to submission to the Engineer. The Contractor shall allow 10 working days for shop drawing approval.
- 11.0 SPECIAL INSPECTIONS:
- 11.1 Refer to Specification Section 014533 for all Special Inspections requirements

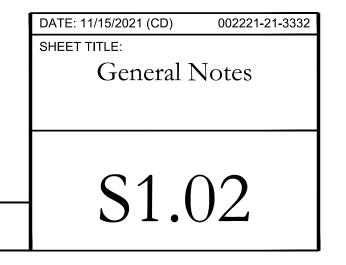
ABBREVIATIONS

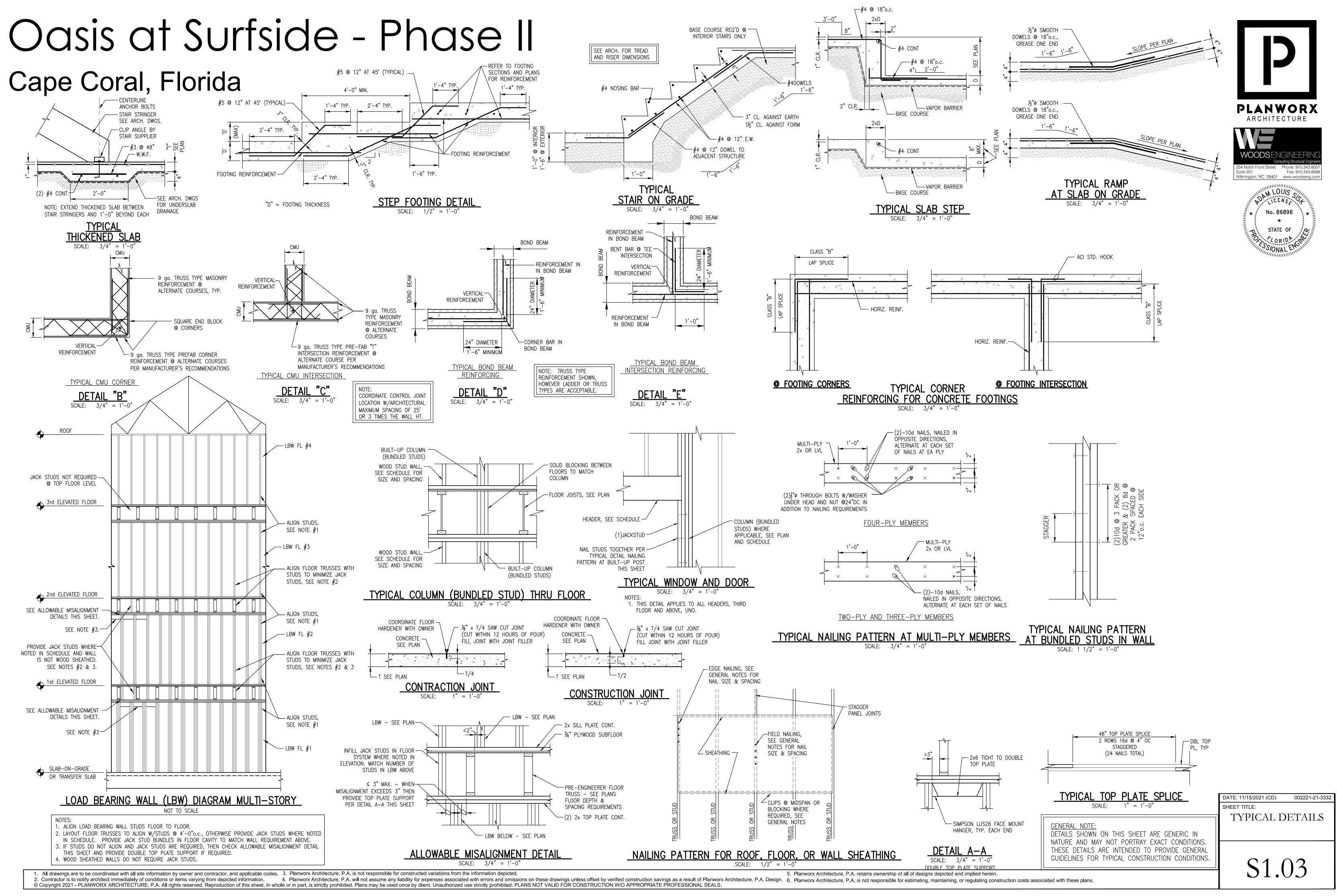
ADDILL	<u>MATIONS</u>		
0	AT	INT	INTERIOR
ي لا	AND	JBE	JOIST BEARING ELEVATION
AB	ANCHOR BOLTS	JT	JOINT
ACI	AMERICAN CONCRETE INSTITUTE	K	KIP-S
ADDL	ADDITIONAL	KB	KICKER BRACE
AFF	ABOVE FINISHED FLOOR	KSI	KIPS PER SQUARE INCH
AISC	AMERICAN INSTITUTE OF STEEL	(L)	LONG SIDE REINFORCEMENT
	CONSTRUCTION	LB	LONG BAR
AISI	AMERICAN IRON AND STEEL	LBS	POUNDS
	INSTITUTE	LLH	LONG LEG HORIZONTAL
ALT	ALTERNATE	LLV	LONG LEG VERTICAL
ARCH	ARCHITECTS – ARCHITECTURAL	LO	LOW
ASTM	AMERICAN SOCIETY FOR	LOC	LOCATION
	TESTING AND MATERIALS	LWC	LIGHT WEIGHT CONCRETE
AWS	AMERICAN WELDING SOCIETY	MAX	MAXIMUM
B, BOTT	BOTTOM	MC	MOMENT CONNECTION
BCX	BOTTOM CHORD EXTENSION	MECH	
BFF	BELOW FINISHED FLOOR		MECHANICAL
		MFR	MANUFACTURER
BLDG	BUILDING	MID	MIDDLE
BM	BEAM	MIN	MINIMUM
BOS	BOTTOM OF STEEL	MISC	MISCELLANEOUS
BRG	BEARING	MOW	MIDDLE OF WALL
BTWN	BETWEEN	MP	MASONRY PILASTER
CJ	CONTRACTION JOINT	d	NAILS – PENNY
CL	CENTERLINE	No	NUMBER
CLR	CLEAR	NS	NEAR SIDE
CMU	CONCRETE MASONRY UNITS	NTS	NOT TO SCALE
COL	COLUMN	NWC	NORMAL WEIGHT CONCRETE
CONC	CONCRETE	OC OC	ON CENTER
CONN	CONNECTION	OFB	OUTSIDE FACE OF BRICK
	CONSTRUCTION JOINT		
		OFM	OUTSIDE FACE OF MASONRY
CONT	CONTINUOUS	OFS	OUTSIDE FACE OF STUD
CONTR	CONTRACTOR	OPNG	OPENING
CSJ	COMPOSITE STEEL JOIST	OPP	OPPOSITE HAND
CTRD	CENTERED	PEBS	PRE-ENGINEERED BUILDING
DBA	DEFORMED BAR ANCHOR		SUPPLIER
DEFL	DEFLECTION	PED	PEDESTAL
DEPR	DEPRESSION – DEPRESSED	PL	PLATE
DET	DETAIL	PSF	POUNDS PER SQUARE FOOT
DIAG	DIAGONAL	PSI	POUNDS PER SQUARE INCH
ø	DIAMETER	PSL	
DIM	DIMENSION	PLF	POUNDS PER LINEAR FOOT
DIST		PT	PRESSURE TREATED
DWG(S)	DRAWING(S)	REF	REFERENCE
DWU(S)			
DWL(S)	DOWEL(S)		REINFORCING
EA	EACH		REQUIRED
ELEV	ELEVATION	(S)	SHORT SIDE REINFORCEMENT
EMBED	EMBEDDED – EMBEDMENT	SB	SHORT BAR
ENG	ENGINEER	SCHD	SCHEDULE
EOR	ENGINEER OF RECORD	SF	STEP FOOTING
EQ	EQUAL	SIM	
EQUIP	EQUIPMENT	SOG	SLAB ON GRADE SPECIFICATION(S) SPRUCE PINE FUR
EF	EACH FACE	SPEC(S)	SPECIFICATION(S)
EJ	EXPANSION JOINT	SPF	SPRUCE PINE FUR
EOD	FDGE OF DECK	SQ	SOLIARE
FOS	EDGE OF SLAB	SQ	STANDARD
FOW	EDGE OF WALL	STIFE	STIFFENER
EW		STIRR	
EXIST		STIKK	
EXP		SIL	STELL
		SIR	STRUCTURAL
EXT	EXTERIOR	SW	SHEAR WALL SOUTHERN YELLOW PINE
FDN		SYP	SOUTHERN YELLOW PINE
FFE		T	
FS		TCX	TOP CHORD EXTENSION
FTG		TOC	TOP OF CONCRETE
GA		TOS	TOP OF STEEL
GALV		TOW	TOP OF WALL
GT	GIRDER TRUSS	TYP	TYPICAL
HD	HEADED	UNO	UNLESS NOTED OTHERWISE
HI	HIGH	VB	VEHICLE BARRIER
HORIZ		VERT	VERTICAL
HSS	HOLLOW STRUCTURAL SECTION	VIF	VERIFY IN FIELD
HT	HIP TRUSS	W	WITH
IFM	INSIDE FACE OF MASONRY	WWF	WELDED WIRE FABRIC
		** **	

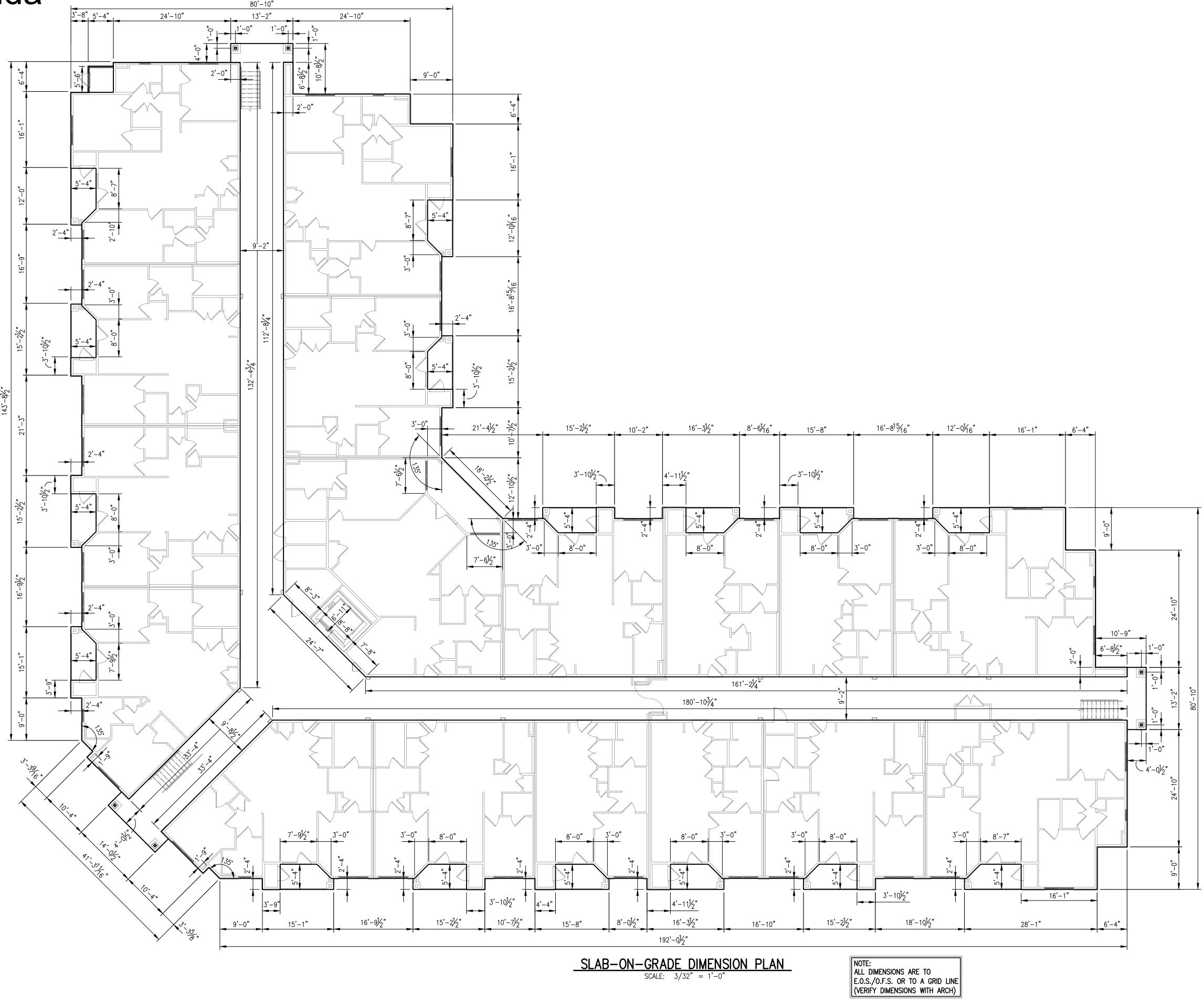


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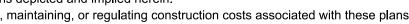
DATE: 11/15/2021 (CD) 002221-21-3332

Slab-on-Grade

Dimension Plan

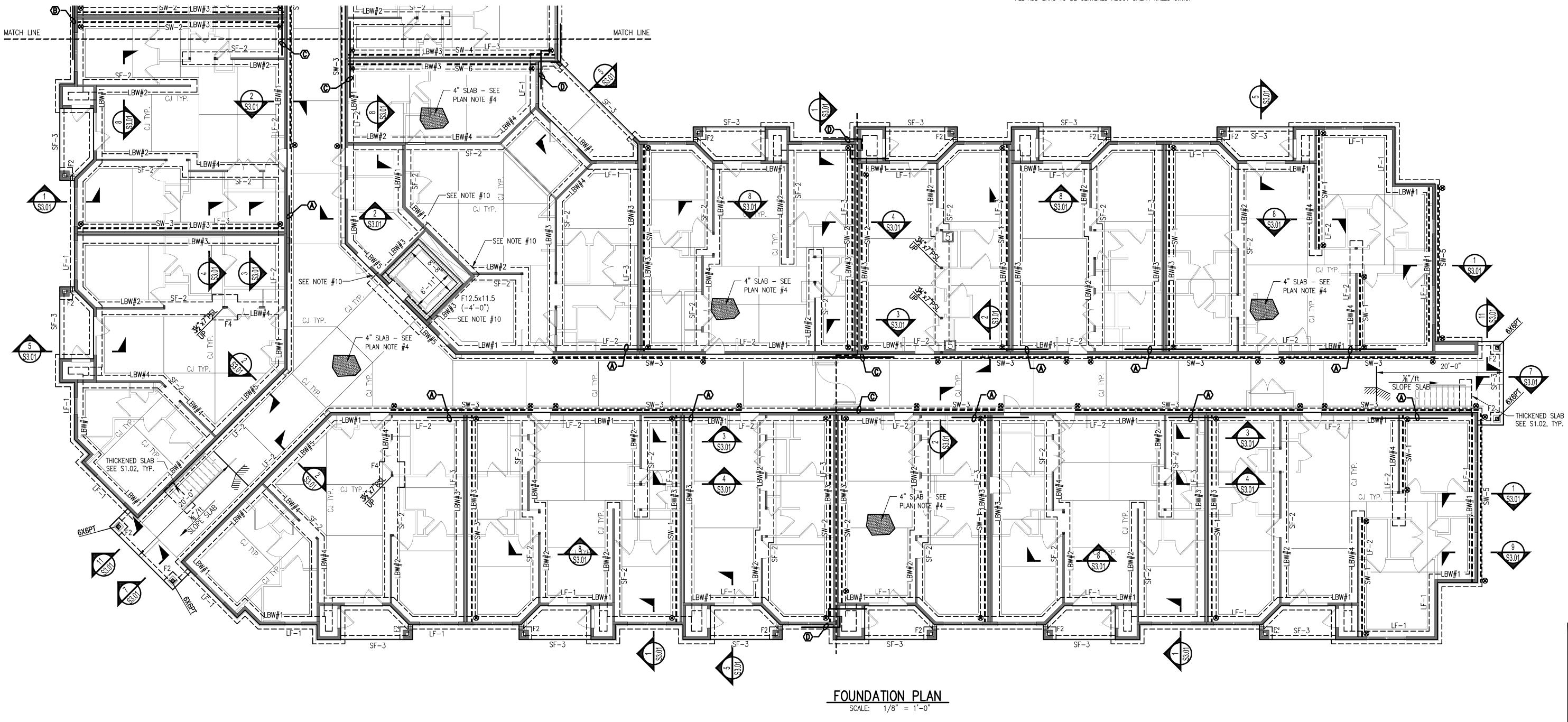
S2.00

SHEET TITLE:



	SPREAD FOOTING (FX) SCHEDULE					STRIP FOOTING (SF-X) SCHEDULE	
MARK	SIZE length x width x thickness	REINFORCEMENT (BOTTOM BARS EACH WAY UNO)	REMARKS	MARK	SIZE width x thickness x length	REINFORCEMENT (BOTTOM BARS UNO)	REMARKS
F2	2'-0" x 2'-0" x 2'-0"	(2) <b>#</b> 5 E.W.		SF-1	2'-0" x 2'-0" x CONT.	(3) #5 CONT. BOTT / (1) #4 CONT. TOP	MONOLITHIC WITH SLAB
F3	3'-0" x 3'-0" x 2'-0"	(3) <b>#</b> 5 E.W.		SF-2	2'-0" x 1'-0" x CONT.	(3) #4 CONT.	MONOLITHIC WITH SLAB
F4	4'-0" x 4'-0" x 1'-0"	(4) <b>#</b> 5 E.W.		SF-3	0'-8" x 2'-0" x CONT.	(1) #4 CONT. TOP & BOTTOM	MONOLITHIC WITH SLAB
F4A	4'-0" x 4'-0" x 2'-0"	(4) <b>#</b> 5 E.W.		SF-4	4'-0" x 1'-4" x CONT.	(5) <b>#</b> 5 CONT.	_
F13.5x11.5	13'-6 x 11'-6" x 1'-0"	#5@12"o.c. EW BOTT.					

	LATERAL FOOTING (LF-X) SCHEDULE				
MARK	SIZE	REINFO	RCEMENT	REMARKS	
	width x thickness x length	BOTTOM			
LF-1	2'-0" x 2'-0" x CONT.	(4) <b>#</b> 5 CONT.	(3) <b>#</b> 5 CONT.	SEE PLAN FOR ADD BARS	
LF-2	3'-0" x 1'-4" x CONT.	(4) <b>#</b> 5 CONT.	(4) <b>#</b> 5 CONT.	SEE PLAN FOR ADD BARS	
LF-3	3'-0" x 2'-0" x CONT.	(5) #6 CONT.	(5) #6 CONT.	SEE PLAN FOR ADD BARS	
LF-4	4'-0" x 2'-0" x CONT.	(6) <b>#</b> 5 CONT.	(4) <b>#</b> 5 CONT.	SEE PLAN FOR ADD BARS	
LF-5	2-0" x 1'-0" x CONT.	(3) #4 CONT.	(3) #4 CONT.	SEE PLAN FOR ADD BARS	



1. All drawings are to be coordinated with all site information by owner and contractor, and applicable codes. 3. Planworx Architecture, P.A. is not responsible for constructed variations from the information depicted. 2. Contractor is to notify architect immediately of conditions or items varying from depicted information. 4. Planworx Architecture, P.A. will not assume any liability for expenses associated with errors and omissions on these drawings unless offset by verified construction savings as a result of Planworx Architecture, P.A. will not assume any liability for expenses associated with these plans. © Copyright 2021 - PLANWORX ARCHITECTURE, P.A. All rights reserved. Reproduction of this sheet, in whole or in part, is strictly prohibited. PLANS NOT VALID FOR CONSTRUCTION W/O APPROPRIATE PROFESSIONAL SEALS.

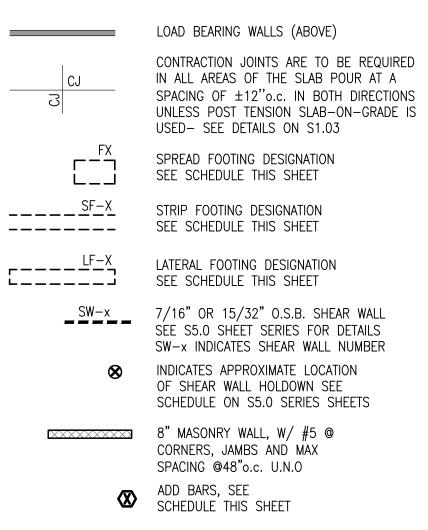
NOTE: THE DESIGN SHOWN IS FOR A CONVENTIONAL FOUNDATION SYSTEM, AND SHOULD BE USED FOR DIMENSIONING PURPOSES ONLY. PRIOR TO CONSTRUCTION, IF A POST-TENSIONED SLAB CONSTRUCTION IS PREFERRED BY THE OWNER, A POST-TENSIONED SLAB ON GRADE DESIGN SHALL BE PREPARED BY A FLORIDA LICENSED STRUCTURAL ENGINEER AND SUBMITTED FOR REVIEW AND APPROVAL BY THE ARCHITECT AND STRUCTURAL ENGINEER. LATERAL FOOTING (LF-x)SIZE AND REINFORCEMENT CANNOT BE REDUCED.

LOAD BEARING WALL (LBW #X) SCHEDULE						
FLOOR			STUD WALL REQU	JIREMENT BY TYPE		
LEVEL	LBW #1	LBW #2	LBW #3	LBW #4	LBW #5	LBW #6
4th	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
3rd	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
2nd	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
1st	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(3) 2x4 @ 16"o.c.	(2) 2x6 @ 16"o.c.	2x8 @ 16"o.c.

NOTE = 1. ALL STUDS TO BE SPF #2 (NORTH) OR BETTER - EXCEPT AS NOTED BELOW AND IN SCHEDULE 2. ALL STUDS ARE REQUIRED TO ALIGN WITH TRUSSES AND STUDS ABOVE. JACK STUDS ARE

REQUIRED IN THE FLOOR CAVITY WHERE STUDS OR TRUSS ABOVE IS LOCATED OUTSIDE THE

ALLOWABLE MISALIGNMENT DETAIL - SEE S1.0 SHEET SERIES. 3. SEE GENERAL WALL FRAMING DETAILS ON SHEET S1.0 SHEET SERIES



	ADD BAR SCHEDULE
А	ADD (1)#5 TOP x 12'-0" LONG
В	ADD (2)#7 TOP AND (1)#5 BOTT. x 12'-0" LONG
С	ADD (4)#7 TOP AND (2)#5 BOTT. x 12'-0" LONG
D	ADD (2)#7 TOP AND (1)#5 BOTT. x 6'-0" LONG W/ HOOK AS SHOWN

NOTE: ALL ADD BARS TO BE CENTERED ABOUT SHEAR WALLS U.N.O.

5. Planworx Architecture, P.A. retains ownership of all of designs depicted and implied herein.

### FOUNDATION LEGEND:

### FOUNDATION PLAN NOTES:

- 1. SEE S1.0 SERIES SHEETS FOR ADDITIONAL GENERAL NOTES, FOUNDATION NOTES, CONCRETE NOTES, REINFORCING STEEL NOTES AND TYPICAL DETAILS. TYPICAL DETAILS ARE GENERALLY NOT SHOWN ON PLAN BUT RATHER ARE INTENDED TO DEFINE TYPICAL CONSTRUCTION CONDITIONS.
- SPACING OF  $\pm 12^{\circ}$  o.c. IN BOTH DIRECTIONS 2. DATUM ELEVATION = TOP OF SLAB ELEVATION = ASSUMED 0'-0" = 10.2' M.S.L. OTHER ELEVATIONS ARE NOTED AS (+ OR -) FROM DATUM ELEVATION.
  - 3. FOOTINGS ARE MONOLITHIC WITH SLAB UNLESS NOTED AS (-X'-X") FROM DATUM ELEVATION.
  - 4. SLAB-ON-GRADE SHALL BE 4" THICK 3000 psi CONCRETE WITH 3.0lbs/yd.3 OF SYNTHETIC MACRO-FIBERS (TUF-STRAND SF BY EUCLID, FIBER MAC SERIES BY BASF, OR FORTA-FERRO BY FORTA CORP, OR APPROVED EQUAL) ON 15 mil VAPOR BARRIER, ON 6" WELL COMPACTED GRANULAR FILL ON WELL COMPACTED SUB GRADE. VERIFY COMPACTION w/QUALIFIED GEOTECHNICAL ENGINEER.
  - 5. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND OTHER DISCIPLINE DRAWINGS FOR OPENINGS AND DEPRESSIONS NOT SHOWN ON THESE DRAWINGS.
  - 6. SEE S5.0 SHEET SERIES FOR SHEAR WALL INFORMATION AND REQUIREMENTS.
  - 7. SEE ARCHITECTURAL DRAWINGS FOR BREEZEWAY SLAB SLOPE.
  - 8. PROVIDE STEEL SLEEVE FOR PLUMBING LINES UNDER FOUNDATIONS. SLEEVE SHALL BE 2" LARGER IN DIAMETER THAN PLUMBING LINE.
  - 9. G.C. TO VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS, (ALL DIMENSIONS ARE TO E.O.S./O.F.B. OR TO A GRID LINE.) WHEN A SECTION IS CUT OR A DETAIL IS LABELED FOR A PARTICULAR CONDITION, THAT SECTION OR DETAIL SHALL APPLY FOR ALL SIMILAR CONDITIONS REGARDLESS OF WHETHER CUT OR LABELED, U.N.O.
  - 10. TURN DOWN SF-x FOOTING ONTO DROPPED ELEVATOR FOOTING, TYP.
  - 11. WHEN A SECTION IS CUT OR A DETAIL IS LABELED FOR A PARTICULAR CONDITION, THAT SECTION OR DETAIL SHALL APPLY FOR ALL SIMILAR CONDITIONS REGARDLESS OF WHETHER CUT OR LABELED, U.N.O



<u>KEY PLAN</u>

NOT TO SCALE

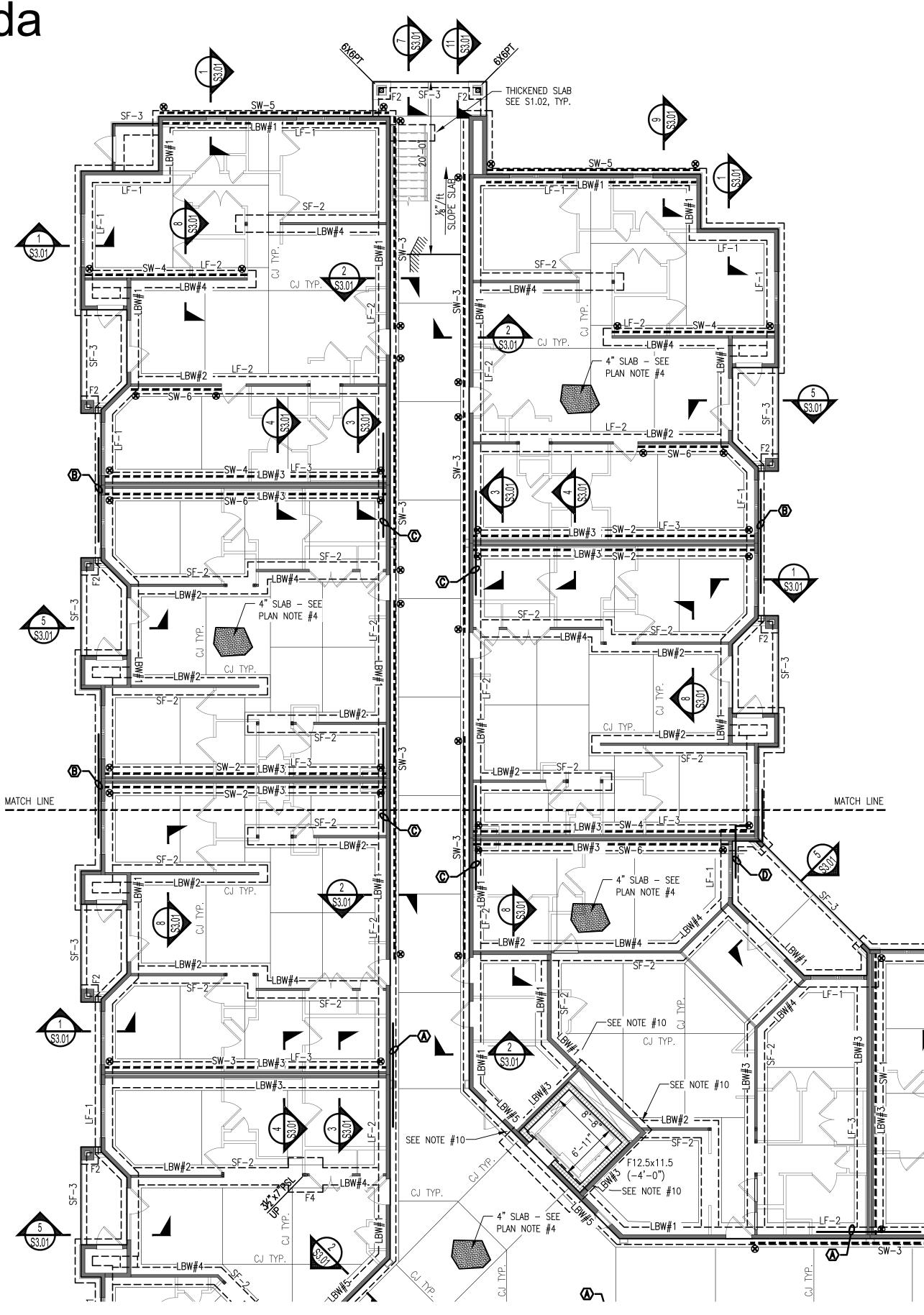
Foundation Plan

S2.01A

002221-21-3332

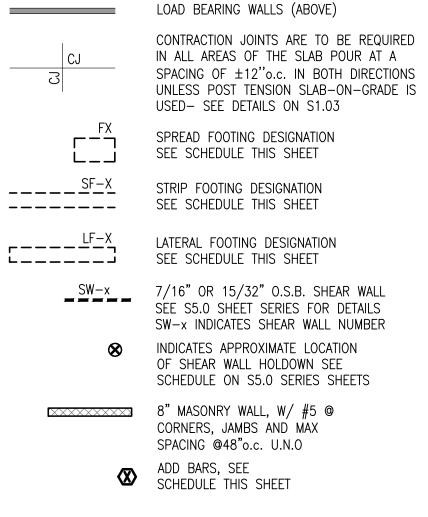
DATE: 11/15/2021 (CD)

SHEET TITLE:



FOUNDATION PLAN SCALE: 1/8" = 1'-0"

### FOUNDATION LEGEND:



	ADD BAR SCHEDULE
А	ADD (1)#5 TOP x 12'-0" LONG
В	ADD (2)#7 TOP AND (1)#5 BOTT. x 12'–0" LONG
С	ADD (4)#7 TOP AND (2)#5 BOTT. x 12'-0" LONG
D	ADD (2)#7 TOP AND (1)#5 BOTT. x 6'-0" LONG W/
NOTE:	

ALL ADD BARS TO BE CENTERED ABOUT SHEAR WALLS U.N.O.

	SPREAD FOOTING (FX) SCHEDULE				
MARK	SIZE length x width x thickness	REINFORCEMENT (BOTTOM BARS EACH WAY UNO)	REMARKS		
F2	2'-0" x 2'-0" x 2'-0"	(2) <b>#</b> 5 E.W.			
F3	3'-0" x 3'-0" x 2'-0"	(3) <b>#</b> 5 E.W.			
F4	4'-0" x 4'-0" x 1'-0"	(4) <b>#</b> 5 E.W.			
F4A	4'-0" x 4'-0" x 2'-0"	(4) <b>#</b> 5 E.W.			
F13.5x11.5	13'-6 x 11'-6" x 1'-0"	#5@12"o.c. EW BOTT.			

STRIP FOOTING (SF-X) SCHEDULE					
MARK	SIZE width x thickness x length	REINFORCEMENT (BOTTOM BARS UNO)	REMARKS		
SF-1	2'-0" x 2'-0" x CONT.	(3) #5 CONT. BOTT / (1) #4 CONT. TOP	MONOLITHIC WITH SLAB		
SF-2	2'-0" x 1'-0" x CONT.	(3) #4 CONT.	MONOLITHIC WITH SLAB		
SF-3	0'-8" x 2'-0" x CONT.	(1) #4 CONT. TOP & BOTTOM	MONOLITHIC WITH SLAB		
SF-4	4'-0" x 1'-4" x CONT.	(5) <b>#</b> 5 CONT.	_		

	LATERAL FOOTING (LF-X) SCHEDULE						
MARK	SIZE	REINFOF	RCEMENT	REMARKS			
	width x thickness x length	BOTTOM	TOP				
LF-1	2'-0" x 2'-0" x CONT.	(4) #5 CONT.	(3) <b>#</b> 5 CONT.	SEE PLAN FOR ADD BARS			
LF-2	3'-0" x 1'-4" x CONT.	(4) #5 CONT.	(4) #5 CONT.	SEE PLAN FOR ADD BARS			
LF-3	3'-0" x 2'-0" x CONT.	(5) #6 CONT.	(5) #6 CONT.	SEE PLAN FOR ADD BARS			
LF-4	4'-0" x 2'-0" x CONT.	(6) #5 CONT.	(4) #5 CONT.	SEE PLAN FOR ADD BARS			
LF-5	2-0" x 1'-0" x CONT.	(3) #4 CONT.	(3) #4 CONT.	SEE PLAN FOR ADD BARS			

FLOOR			STUD WALL REQU	IREMENT BY TYPE			
LEVEL	LBW #1	LBW #2	LBW #3	LBW #4	LBW #5	LBW #6	
4th	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c	
3rd	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c	
2nd	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c	
1st	1st 2x6 @ 16"o.c. (2) 2x4 @ 16"o.c. (2) 2x4 @ 16"o.c. (3) 2x4 @ 16"o.c. (2) 2x6 @ 16"o.c. 2x8 @ 16"o.c.						
NOTE = 1. ALL STUDS TO BE SPF #2 (NORTH) OR BETTER – EXCEPT AS NOTED BELOW AND IN SCHEDULE 2. ALL STUDS ARE REQUIRED TO ALIGN WITH TRUSSES AND STUDS ABOVE, JACK STUDS ARE							



#### FOUNDATION PLAN NOTES:

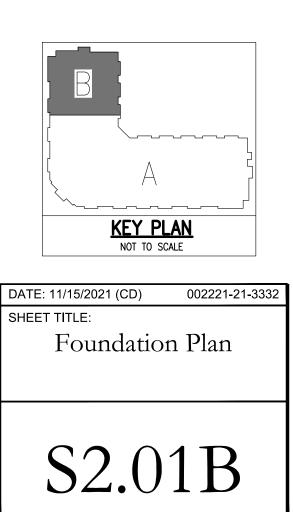
- HOOK AS SHOWN

- 1. SEE S1.0 SERIES SHEETS FOR ADDITIONAL GENERAL NOTES, FOUNDATION NOTES, CONCRETE NOTES, REINFORCING STEEL NOTES AND TYPICAL DETAILS. TYPICAL DETAILS ARE GENERALLY NOT SHOWN ON PLAN BUT RATHER ARE INTENDED TO DEFINE TYPICAL CONSTRUCTION CONDITIONS.
- SPACING OF  $\pm 12^{\circ}$  o.c. IN BOTH DIRECTIONS 2. DATUM ELEVATION = TOP OF SLAB ELEVATION = ASSUMED 0'-0" = 10.2' M.S.L. OTHER ELEVATIONS ARE NOTED AS (+ OR -) FROM DATUM ELEVATION.
  - 3. FOOTINGS ARE MONOLITHIC WITH SLAB UNLESS NOTED AS (-X'-X") FROM DATUM ELEVATION.
  - 4. SLAB-ON-GRADE SHALL BE 4" THICK 3000 psi CONCRETE WITH 3.0lbs/yd.3 OF SYNTHETIC MACRO-FIBERS (TUF-STRAND SF BY EUCLID, FIBER MAC SERIES BY BASF, OR FORTA-FERRO BY FORTA CORP, OR APPROVED EQUAL) ON 15 mil VAPOR BARRIER, ON 6" WELL COMPACTED GRANULAR FILL ON WELL COMPACTED SUB GRADE. VERIFY COMPACTION w/QUALIFIED GEOTECHNICAL ENGINEER.
  - 5. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND OTHER DISCIPLINE DRAWINGS FOR OPENINGS AND DEPRESSIONS NOT SHOWN ON THESE DRAWINGS.
  - 6. SEE S5.0 SHEET SERIES FOR SHEAR WALL INFORMATION AND REQUIREMENTS.
  - 7. SEE ARCHITECTURAL DRAWINGS FOR BREEZEWAY SLAB SLOPE.
  - 8. PROVIDE STEEL SLEEVE FOR PLUMBING LINES UNDER FOUNDATIONS. SLEEVE SHALL BE 2" LARGER IN DIAMETER THAN PLUMBING LINE.
  - 9. G.C. TO VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS, (ALL DIMENSIONS ARE TO E.O.S./O.F.B. OR TO A GRID LINE.) WHEN A SECTION IS CUT OR A DETAIL IS LABELED FOR A PARTICULAR CONDITION, THAT SECTION OR DETAIL SHALL APPLY FOR ALL SIMILAR CONDITIONS REGARDLESS OF WHETHER CUT OR LABELED, U.N.O.
  - 10. TURN DOWN SF-x FOOTING ONTO DROPPED ELEVATOR FOOTING, TYP.
  - 11. WHEN A SECTION IS CUT OR A DETAIL IS LABELED FOR A PARTICULAR CONDITION, THAT SECTION OR DETAIL SHALL APPLY FOR ALL SIMILAR CONDITIONS REGARDLESS OF WHETHER CUT OR LABELED, U.N.O

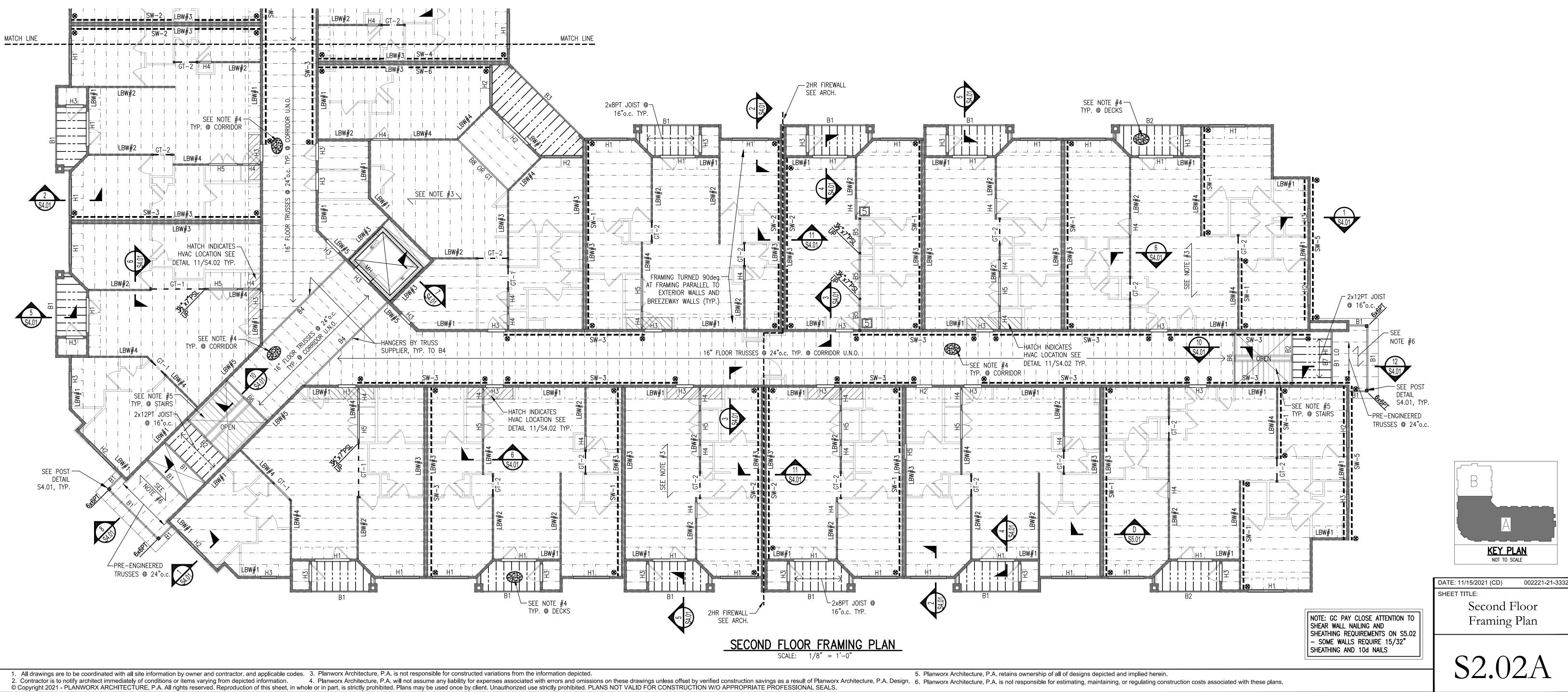
3. SEE GENERAL WALL FRAMING DETAILS ON SHEET ST.U SHEET SERIES

NOTE: THE DESIGN SHOWN IS FOR A CONVENTIONAL FOUNDATION SYSTEM, AND SHOULD BE USED FOR DIMENSIONING PURPOSES ONLY. PRIOR TO CONSTRUCTION, IF A POST-TENSIONED SLAB CONSTRUCTION IS PREFERRED BY THE OWNER, A POST-TENSIONED SLAB ON GRADE DESIGN SHALL BE PREPARED BY A FLORIDA LICENSED STRUCTURAL ENGINEER AND SUBMITTED FOR REVIEW AND APPROVAL BY THE ARCHITECT AND STRUCTURAL ENGINEER. LATERAL FOOTING (LF-x) SIZE AND REINFORCEMENT CANNOT BE REDUCED.





BE	AM SCHEDULE			) STUD )N PLA	
MARK	BEAM REQUIREMENT	FL <b>#</b> 1	FL #2	FL <b>#</b> 3	
B1	(2) 2x12 PT	3	3	3	
B2	3½" x 11¼" PT GLULAM	4	3	3	
B3	5½" x 11¼" PT GLULAM	4	3	3	
B4	(4) 1¾" x 16" LVLs	6	5	4	
B5	(2) 1¾" x 18" LVLs		SEE	PLAN	
B6	3½" x 16" PT GLULAM	4	3	3	
B7	(3) 2x12 PT	3	3	3	
B8	(2) 1¾" x 16" LVLs	5	4	3	
B9	(2) 1¾" x 11¼" LVLs	SEE PLAN	_	_	
B10					



	HEADER SCHEDULE	JAME	B REQ	UIREM	ents
MARK	HEADER REQUIREMENTS	FL #1	FL #2	FL #3	FL #4
H1	(3) 2x8 W/ $\frac{7}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H2	(3) $2 \times 10 \text{ W} / \frac{7}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H3	(3) 2x8 W/ $\frac{1}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	1K/1J	1K/1J
H4	(2) 2x8 W/ $\frac{1}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H5	(2) 2x12 W/ $\frac{7}{6}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	3K/2J	2K/2J	2K/1J	2K/1J
H6	(3) 2x10 W/ 7/6" O.S.B. PR PLYWOOD BETWEEN PLYS	4K/2J	_	-	_

FLOOR FRAMING LEGEND
----------------------

<u>FLOOR F</u>	<u>RAMING LEGEND</u>
······	18" FLOOR TRUSSES
	@ 24" o.c. U.N.O.
	PRE-ENGINEERED ROOF
	TRUSSES @ 24"o.c.
	2x8PT JOISTS @ 16"o.c. (SEE ARCH. FOR SLOPE REQUIREMENTS)
Hx	HEADER, SEE SCHEDULE THIS SHEET
Bx	WOOD BEAM, SEE SCHEDULE THIS SHEET
	2x WALLS (ABOVE)
	LOAD BEARING WALLS (BELOW)
GT	INDICATES GIRDER TRUSS SEE SCHEDULE THIS SHEET
	INDICATES CHORD TRUSS SEE DETAIL ON S4.01
	BUNDLED STUDS DOWN (TYPICAL U.N.O.) SEE GENERAL FRAMING NOTES ON SHEET S1.01
<u>SW-x</u>	7/16" OR 15/32" O.S.B. SHEAR WALL SEE S5.0 SHEET SERIES FOR DETAILS SW—x INDICATES SHEAR WALL NUMBER
8	INDICATES APPROXIMATE LOCATION OF SHEAR WALL HOLDOWN SEE SCHEDULE ON S5.02
MH	MASONRY HEADER MH1: (1)#5 — GROUT (2) COURSES AE
	8" MASONRY WALL, W/ #5 @

#5 @ CORNERS, JAMBS AND MAX SPACING @48"o.c. U.N.O

LOAD BEARING WALL (LBW $\#X$ ) SCHEDULE						
FLOOR			STUD WALL REQU	IREMENT BY TYPE		
LEVEL	LBW #1	LBW #2	LBW #3	LBW #4	LBW #5	LBW #6
4th	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
3rd	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
2nd	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
1st	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(3) 2x4 @ 16"o.c.	(2) 2x6 @ 16"o.c.	2x8 @ 16"o.c.
NOTE = $1$	NOTE = 1. ALL STUDS TO BE SPF #2 (NORTH) OR BETTER – EXCEPT AS NOTED BELOW AND IN SCHEDULE					

2. ALL STUDS ARE REQUIRED TO ALIGN WITH TRUSSES AND STUDS ABOVE. JACK STUDS ARE REQUIRED IN THE FLOOR CAVITY WHERE STUDS OR TRUSS ABOVE IS LOCATED OUTSIDE THE

ALLOWABLE MISALIGNMENT DETAIL - SEE S1.0 SHEET SERIES. 3. SEE GENERAL WALL FRAMING DETAILS ON SHEET S1.0 SHEET SERIES

#### FLOOR FRAMING PLAN NOTES:

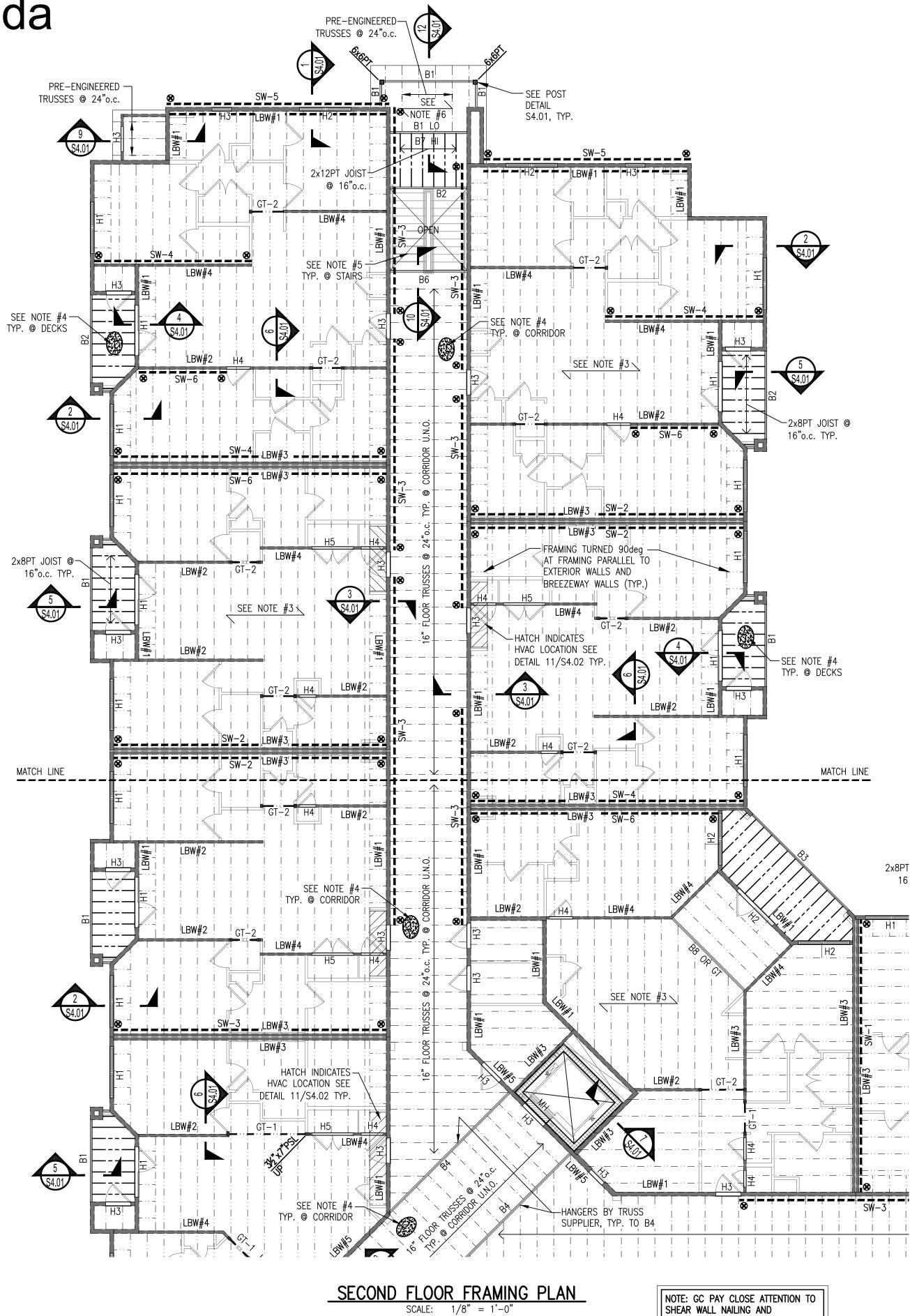
- 1. FOR GENERAL FRAMING INFORMATION, SEE SHEET S1.01
- 2. HEADERS, BEAMS & LOAD BEARING WALLS SHOWN ARE FOR FRAMING BELOW THIS LEVEL. SHEAR WALLS REFER TO WALLS BELOW.
- 3. SUBFLOOR SHALL BE EXTERIOR GRADE 3/4" TONGUE AND GROOVE O.S.B..
- 4. DECK AND CORRIDOR SUB FLOOR BE  $\frac{3}{4}$ " PT PLYWOOD WITH 2" N.W. PEA GRAVEL CONCRETE TOPPING AT DECKS AND 1/2" @ COORDIORS, 4000psi W/ AIR ENTRAINMENT WITH LIGHT BROOM FINISH. REINFORCE W/ 2.5lbs/yd3 OF SYNTHETIC MARCRO-FIBER OR WWF 6x6xW2.0xW2.0. PROVIDE CONTROL JOINTS AT CORNERS AND APPROXIMATELY 8'-0"o.c.
- 5. STAIRS SHALL HAVE STEEL STRINGERS WITH CONCRETE TREADS PER ARCH.
- 6. ROOF SHEATHING SHALL BE /8" EXTERIOR GRADE PLYWOOD SPAN AS NOTED ON PLAN.
- 7. WHEN A SECTION IS CUT OR A DETAIL IS LABELED FOR A PARTICULAR CONDITION, THAT SECTION OR DETAIL SHALL APPLY FOR ALL SIMILAR CONDITIONS REGARDLESS OF WHETHER CUT OR LABELED, U.N.O

GIRDER	TRUSS SCHEDULE	_	UNDLED J.N.O. 0		-
MARK	GIRDER TRUSS	FL #1	FL <b>#</b> 2	FL <b>#</b> 3	FL #4
GT-1	18" TRUSS SUPPLIER	31∕2"x7" PSL	5	3	3
GT-2	18" TRUSS SUPPLIER	4	3	3	3
GT-3	18" TRUSS SUPPLIER	5	4	3	3



COURSES ABOVE

### Oasis at Surfside - Phase II Cape Coral, Florida PRE-ENGINEERED-TRUSSES @ 24"o.c.



SHEAR WALL NAILING AND SHEATHING REQUIREMENTS ON S5.02 - SOME WALLS REQUIRE 15/32" SHEATHING AND 10d NAILS

<u>FLOOR F</u>	RAMING LEGEND
	18" FLOOR TRUSSES @ 24" o.c. U.N.O.
	PRE-ENGINEERED ROOF
_ · _ · _ · _ · _ · _ · _	TRUSSES @ 24"o.c.
	2x8PT JOISTS @ 16"o.c. (SEE ARCH. FOR SLOPE REQUIREMENTS)
Hx	HEADER, SEE SCHEDULE THIS SHEET
<u> </u>	WOOD BEAM, SEE SCHEDULE THIS SHEET 2x WALLS (ABOVE)
	LOAD BEARING WALLS (BELOW)
GT	INDICATES GIRDER TRUSS SEE SCHEDULE THIS SHEET
CT	INDICATES CHORD TRUSS SEE DETAIL ON S4.01
	BUNDLED STUDS DOWN (TYPICAL U.N.O.) SEE GENERAL FRAMING NOTES ON SHEET S1.01
<u>SW-x</u>	7/16" OR 15/32" O.S.B. SHEAR WALL SEE S5.0 SHEET SERIES FOR DETAILS SW—x INDICATES SHEAR WALL NUMBER
8	INDICATES APPROXIMATE LOCATION OF SHEAR WALL HOLDOWN SEE SCHEDULE ON S5.02
MH	MASONRY HEADER MH1: (1)#5 – GROUT (2) COURSES ABOVE
	8" MASONRY WALL, W/ #5 @ CORNERS, JAMBS AND MAX SPACING @48"o.c. U.N.O

HEADER SCHEDULE		JAME	B REQ	UIREM	ents
MARK	HEADER REQUIREMENTS	FL <b>#</b> 1	FL <b>#</b> 2	FL <b>#</b> 3	FL #4
H1	(3) 2x8 W/ $7_{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H2	(3) 2x10 W/ $\frac{7}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H3	(3) 2x8 W/ $7_{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	1K/1J	1K/1J
H4	(2) 2x8 W/ $7_{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H5	(2) 2x12 W/ $\frac{7}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	3K/2J	2K/2J	2K/1J	2K/1J
H6	(3) 2x10 W/ $7_{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	4K/2J	-	-	_

GIRDER	TRUSS SCHEDULE	_	UNDLED	
MARK	GIRDER TRUSS	FL #1	FL <b>#</b> 2	FL #
GT-1	18" TRUSS SUPPLIER	31∕2"x7" PSL	5	3
GT-2	18" TRUSS SUPPLIER	4	3	3
GT-3	18" TRUSS SUPPLIER	5	4	3

LOAD BEARING WALL (LBW #X) SCHEDULE						
FLOOR		STUD WALL REQUIREMENT BY TYPE				
LEVEL	LBW #1	LBW #2	LBW #3	LBW #4	LBW #5	LBW #6
4th	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
3rd	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
2nd	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
1st	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(3) 2x4 @ 16"o.c.	(2) 2x6 @ 16"o.c.	2x8 @ 16"o.c.
<ul> <li>NOTE = 1. ALL STUDS TO BE SPF #2 (NORTH) OR BETTER - EXCEPT AS NOTED BELOW AND IN SCHEDULE</li> <li>2. ALL STUDS ARE REQUIRED TO ALIGN WITH TRUSSES AND STUDS ABOVE. JACK STUDS ARE REQUIRED IN THE FLOOR CAVITY WHERE STUDS OR TRUSS ABOVE IS LOCATED OUTSIDE THE ALLOWABLE MISALIGNMENT DETAIL - SEE S1.0 SHEET SERIES.</li> <li>3. SEE GENERAL WALL FRAMING DETAILS ON SHEET S1.0 SHEET SERIES</li> </ul>						

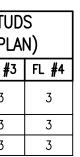
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#### FLOOR FRAMING PLAN NOTES:

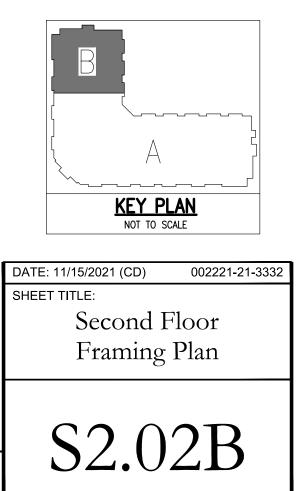
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- 3. SUBFLOOR SHALL BE EXTERIOR GRADE 3/4" TONGUE AND GROOVE O.S.B.
- 4. DECK AND CORRIDOR SUB FLOOR BE  $\frac{3}{4}$ " PT PLYWOOD WITH 2" N.W. PEA GRAVEL CONCRETE TOPPING AT DECKS AND 11/3" @ COORDIORS, 4000psi W/ AIR ENTRAINMENT WITH LIGHT BROOM FINISH. REINFORCE W/ 2.5lbs/yd<sup>3</sup> OF SYNTHETIC MARCRO-FIBER OR WWF 6x6xW2.0xW2.0. PROVIDE CONTROL JOINTS AT CORNERS AND APPROXIMATELY 8'-0"o.c.
- HAVE STEEL STRINGERS WITH CONCRETE TREADS PER ARCH.
- 6. ROOF SHEATHING SHALL BE /8" EXTERIOR GRADE PLYWOOD SPAN AS NOTED ON PLAN.
- 7. WHEN A SECTION IS CUT OR A DETAIL IS LABELED FOR A PARTICULAR CONDITION, THAT SECTION OR DETAIL SHALL APPLY FOR ALL SIMILAR CONDITIONS REGARDLESS OF WHETHER CUT OR LABELED, U.N.O





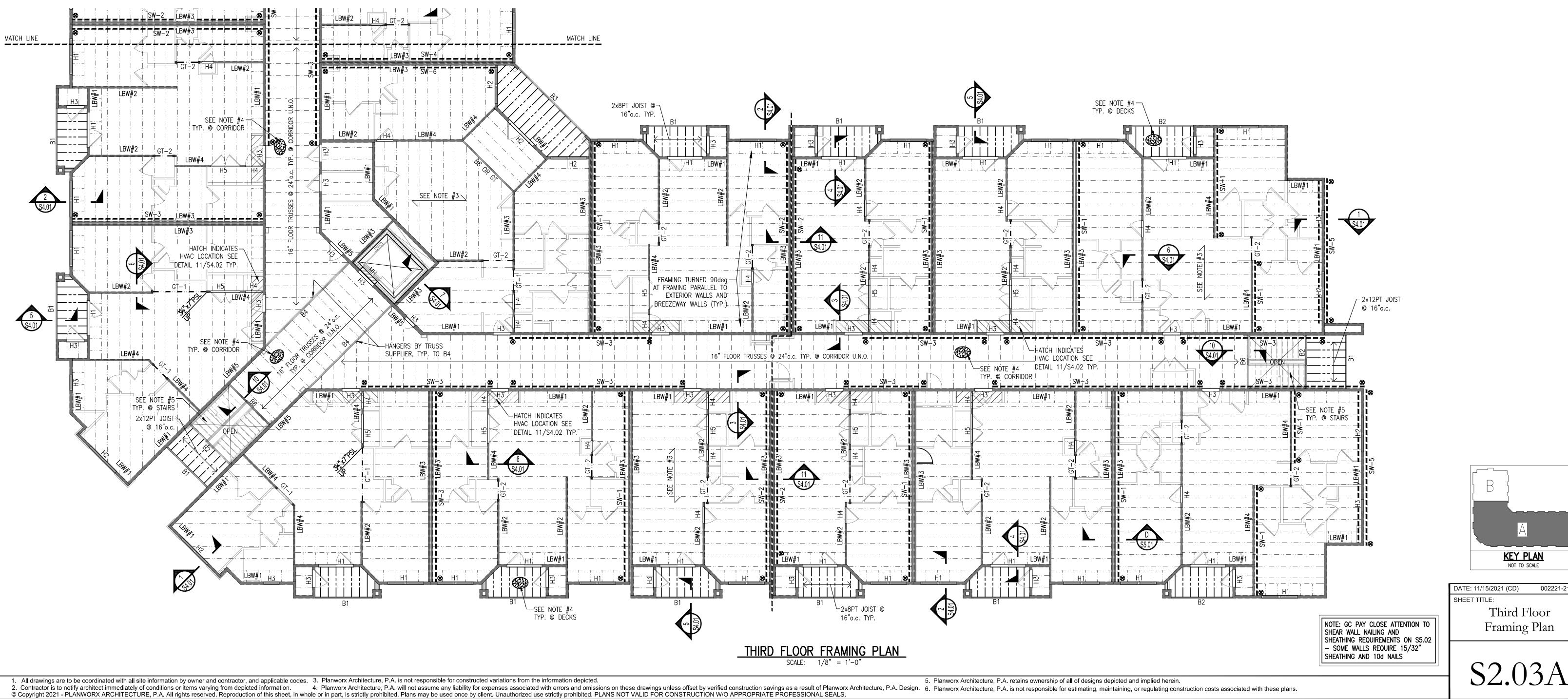
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BEAM SCHEDULE				) stud )n pla	
MARK	BEAM REQUIREMENT	FL <b>#</b> 1	FL <b>#</b> 2	FL <b>#</b> 3	FL <b>#</b> 4
B1	(2) 2x12 PT	3	3	3	2
B2	3½" x 11¼" PT GLULAM	4	3	3	3
B3	5½" x 11¼" PT GLULAM	4	3	3	3
B4	(4) 1¾" x 16" LVLs	6	5	4	3
B5	(2) 1¾" × 18" LVLs		SEE	PLAN	
B6	3½" x 16" PT GLULAM	4	3	3	3
B7	(3) 2x12 PT	3	3	3	3
B8	(2) 1¾" x 16" LVLs	5	4	3	3
B9	(2) 1¾" x 11¼" LVLs	SEE PLAN	_	_	_
B10					



BE	BEAM SCHEDULE		BUNDLED STUDS (U.N.O. ON PLAN		
MARK	BEAM REQUIREMENT	FL #1 FL #2 FL #3			FL #4
B1	(2) 2x12 PT	3	3	3	2
B2	3½" x 11¼" PT GLULAM	4	3	3	3
B3	5½" x 11¼" PT GLULAM	4	3	3	3
B4	(4) 1¾" x 16" LVLs	6	5	4	3
B5	(2) 1¾" x 18" LVLs		SEE	PLAN	
B6	3½" x 16" PT GLULAM	4	3	3	3
B7	(3) 2x12 PT	3	3	3	3
B8	(2) 1¾" x 16" LVLs	5	4	3	3
B9	(2) 1¾" x 11¼" LVLs	SEE PLAN	_	_	_
B10					

2



	HEADER SCHEDULE	JAME	B REQ	UIREM	ENTS
MARK	HEADER REQUIREMENTS	FL #1	FL <b>#</b> 2	FL <b>#</b> 3	FL #4
H1	(3) 2x8 W/ $\frac{1}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H2	(3) 2x10 W/ $\frac{7}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H3	(3) 2x8 W/ $\frac{1}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	1K/1J	1K/1J
H4	(2) 2x8 W/ $\frac{1}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H5	(2) 2x12 W/ $\%_6$ " O.S.B. PR PLYWOOD BETWEEN PLYS	3K/2J	2K/2J	2K/1J	2K/1J
H6	(3) 2x10 W/ 7/6" O.S.B. PR PLYWOOD BETWEEN PLYS	4K/2J	-	-	_

FLOOR FRAMING LEGEN	D
	_

	18" FLOOR TRUSSES @ 24" o.c. U.N.O.
	2x8PT JOISTS @ 16"o.c. (SEE ARCH. FOR SLOPE REQUIREMENTS)
Hx	HEADER, SEE SCHEDULE THIS SHEET
Bx	WOOD BEAM, SEE SCHEDULE THIS SHEET
	2x WALLS (ABOVE)
	LOAD BEARING WALLS (BELOW)
GT	INDICATES GIRDER TRUSS SEE SCHEDULE THIS SHEET
	INDICATES CHORD TRUSS SEE DETAIL ON S4.01
	BUNDLED STUDS DOWN (TYPICAL U.N.O.) SEE GENERAL FRAMING NOTES ON SHEET S1.01
SW-x	7/16" OR 15/32" O.S.B. SHEAR WALL SEE S5.0 SHEET SERIES FOR DETAILS SW-x INDICATES SHEAR WALL NUMBER
8	INDICATES APPROXIMATE LOCATION OF SHEAR WALL HOLDOWN SEE SCHEDULE ON S5.02
MH	MASONRY HEADER MH1: (1)#5 – GROUT (2) COURSES AE
XXXXXXXXXX	8" MASONRY WALL, W/ #5 @ CORNERS, JAMBS AND MAX SPACING @48"o.c. U.N.O

	LOAD BEARING WALL (LBW $\#X$ ) SCHEDULE					
FLOOR	FLOOR STUD WALL REQUIREMENT BY TYPE					
LEVEL	LBW #1	LBW #2	LBW #3	LBW #4	LBW #5	LBW #6
4th	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
3rd	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
2nd	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
1st	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(3) 2x4 @ 16"o.c.	(2) 2x6 @ 16"o.c.	2x8 @ 16"o.c.
NOTE = $1$	ALL STUDS TO BE	SPF #2 (NORTH)	OR BETTER - EXCE	PT AS NOTED BELO	W AND IN SCHEDULE	

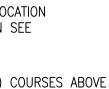
2. ALL STUDS ARE REQUIRED TO ALIGN WITH TRUSSES AND STUDS ABOVE. JACK STUDS ARE REQUIRED IN THE FLOOR CAVITY WHERE STUDS OR TRUSS ABOVE IS LOCATED OUTSIDE THE

ALLOWABLE MISALIGNMENT DETAIL - SEE S1.0 SHEET SERIES. 3. SEE GENERAL WALL FRAMING DETAILS ON SHEET S1.0 SHEET SERIES

### FLOOR FRAMING PLAN NOTES:

- 1. FOR GENERAL FRAMING INFORMATION, SEE SHEET S1.01
- 2. HEADERS, BEAMS & LOAD BEARING WALLS SHOWN ARE FOR FRAMING BELOW THIS LEVEL. SHEAR WALLS REFER TO WALLS BELOW.
- 3. SUBFLOOR SHALL BE EXTERIOR GRADE 3/4" TONGUE AND GROOVE O.S.B.
- 4. DECK AND CORRIDOR SUB FLOOR BE 3/4" PT PLYWOOD WITH 2" N.W. PEA GRAVEL CONCRETE TOPPING AT DECKS AND 1/2" @ COORDIORS, 4000psi W/ AIR ENTRAINMENT WITH LIGHT BROOM FINISH. REINFORCE W/ 2.5lbs/yd3 OF SYNTHETIC MARCRO-FIBER OR WWF 6x6xW2.0xW2.0. PROVIDE CONTROL JOINTS AT CORNERS AND APPROXIMATELY 8'-0"o.c.
- STAIRS SHALL HAVE STEEL STRINGERS WITH CONCRETE TREADS PER ARCH.
- 6. WHEN A SECTION IS CUT OR A DETAIL IS LABELED FOR A PARTICULAR CONDITION, THAT SECTION OR DETAIL SHALL APPLY FOR ALL SIMILAR CONDITIONS REGARDLESS OF WHETHER CUT OR LABELED, U.N.O

GIRDER	TRUSS SCHEDULE		UNDLED J.N.O. 0		-
MARK	GIRDER TRUSS	FL <b>#</b> 1	FL <b>#</b> 2	FL <b>#</b> 3	FL #4
GT-1	18" TRUSS SUPPLIER	31∕2"x7" PSL	5	3	3
GT-2	18" TRUSS SUPPLIER	4	3	3	3
GT-3	18" TRUSS SUPPLIER	5	4	3	3





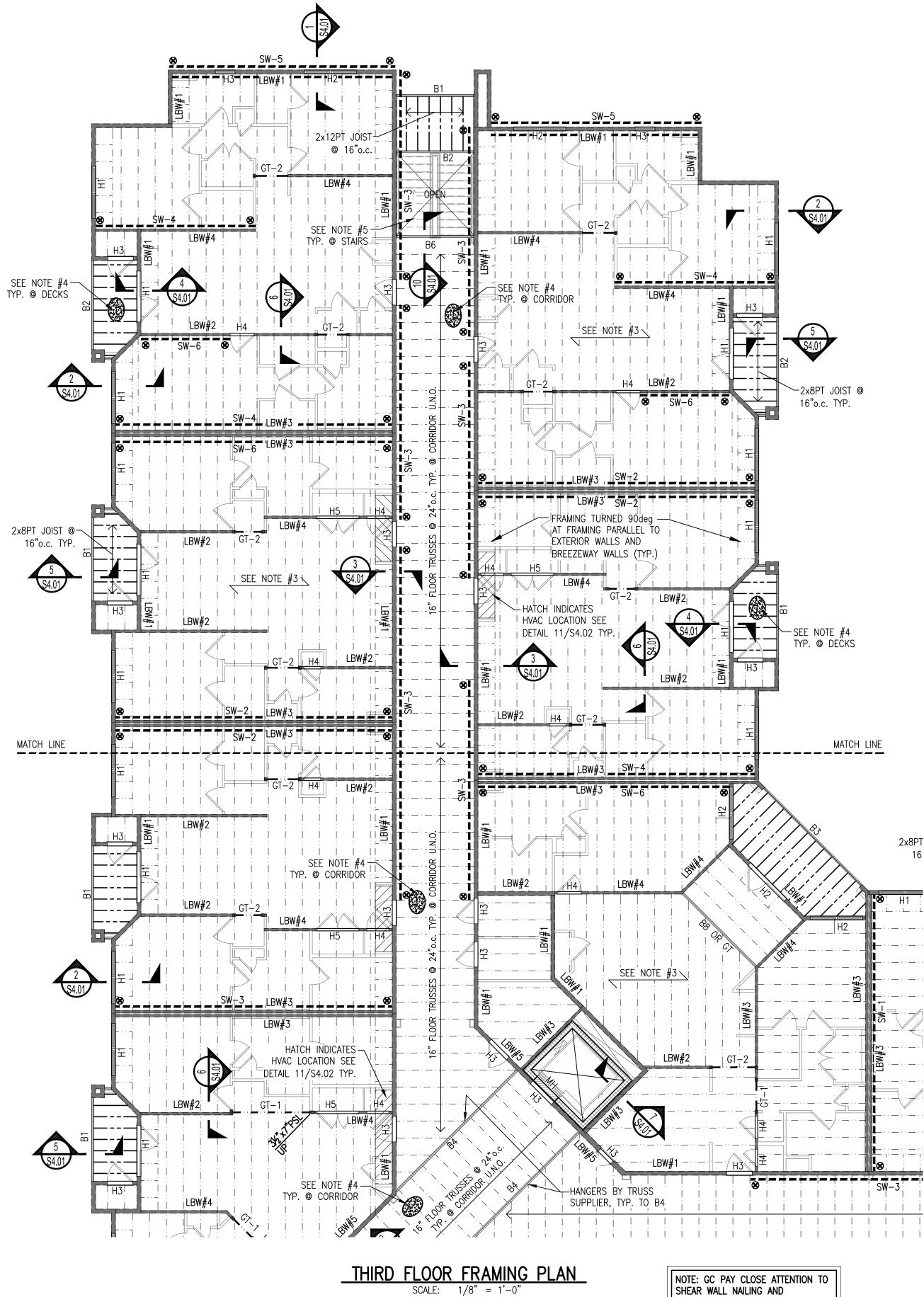
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KEY PLAN

Third Floor

Framing Plan

002221-21-3332



SHEAR WALL NAILING AND SHEATHING REQUIREMENTS ON S5.02 - SOME WALLS REQUIRE 15/32" SHEATHING AND 10d NAILS

### FLOOR FRAMING LEGEND

	18" FLOOR TRUSSES @ 24" o.c. U.N.O.
	2x8PT JOISTS @ 16"o.c. (SEE ARCH. FOR SLOPE REQUIREMENTS)
Hx	HEADER, SEE SCHEDULE THIS SHEET
Bx	WOOD BEAM, SEE SCHEDULE THIS SHEET
	2x WALLS (ABOVE)
	LOAD BEARING WALLS (BELOW)
GT	INDICATES GIRDER TRUSS SEE SCHEDULE THIS SHEET
	INDICATES CHORD TRUSS SEE DETAIL ON S4.01
	BUNDLED STUDS DOWN (TYPICAL U.N.O.) SEE GENERAL FRAMING NOTES ON SHEET S1.01
	7/16" OR 15/32" O.S.B. SHEAR WALL SEE S5.0 SHEET SERIES FOR DETAILS SW—x INDICATES SHEAR WALL NUMBER
8	INDICATES APPROXIMATE LOCATION OF SHEAR WALL HOLDOWN SEE SCHEDULE ON S5.02
MH	MASONRY HEADER MH1: (1)#5 – GROUT (2) COURSES ABOVE
*****	8" MASONRY WALL, W/ #5 @ CORNERS, JAMBS AND MAX SPACING @48"o.c. U.N.O

HEADER SCHEDULE		JAME	B REQ	UIREM	ENTS
MARK	HEADER REQUIREMENTS	FL <b>#</b> 1	FL <b>#</b> 2	FL <b>#</b> 3	FL <b>#</b> 4
H1	(3) 2x8 W/ $7_{6}$ ° O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H2	(3) 2x10 W/ $\%_6$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
Н3	(3) 2x8 W/ $7_{6}$ " 0.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	1K/1J	1K/1J
H4	(2) 2x8 W/ $7_{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H5	(2) 2x12 W/ $\frac{7}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	3K/2J	2K/2J	2K/1J	2K/1J
H6	(3) 2x10 W/ $\mathcal{H}_6$ " O.S.B. PR PLYWOOD BETWEEN PLYS	4K/2J	_	_	_

GIRDER	TRUSS SCHEDULE	_	UNDLED J.N.O. 0	
MARK	GIRDER TRUSS	FL #1	FL #2	FL #
GT-1	18" TRUSS SUPPLIER	31∕2"x7" PSL	5	3
GT-2	18" TRUSS SUPPLIER	4	3	3
GT-3	18" TRUSS SUPPLIER	5	4	3

LOAD BEARING WALL (LBW #X) SCHEDULE						
FLOOR		STUD WALL REQUIREMENT BY TYPE				
LEVEL	LBW #1	LBW #2	LBW #3	LBW #4	LBW #5	LBW #6
4th	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
3rd	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
2nd	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
1st	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(3) 2x4 @ 16"o.c.	(2) 2x6 @ 16"o.c.	2x8 @ 16"o.c.
NOTE = 1. ALL STUDS TO BE SPF #2 (NORTH) OR BETTER - EXCEPT AS NOTED BELOW AND IN SCHEDULE 2. ALL STUDS ARE REQUIRED TO ALIGN WITH TRUSSES AND STUDS ABOVE. JACK STUDS ARE REQUIRED IN THE FLOOR CAVITY WHERE STUDS OR TRUSS ABOVE IS LOCATED OUTSIDE THE ALLOWABLE MISALIGNMENT DETAIL - SEE S1.0 SHEET SERIES. 3. SEE CENERAL WALL ERAMINE ON SHEET SI O SHEET SERIES.						

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#### FLOOR FRAMING PLAN NOTES:

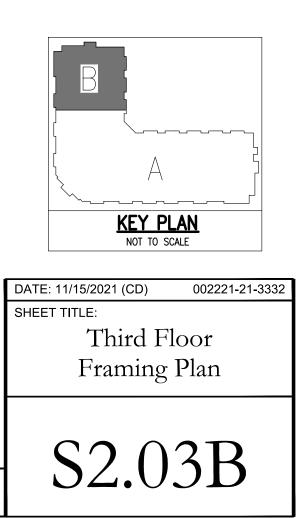
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- STEEL STRINGERS WITH CONCRETE TREADS PER ARCH
- WHEN A SECTION IS CUT OR A DETAIL IS LABELED FOR A PARTICULAR CONDITION, THAT SECTION OR DETAIL SHALL APPLY FOR ALL SIMILAR CONDITIONS REGARDLESS OF WHETHER CUT OR LABELED, U.N.O



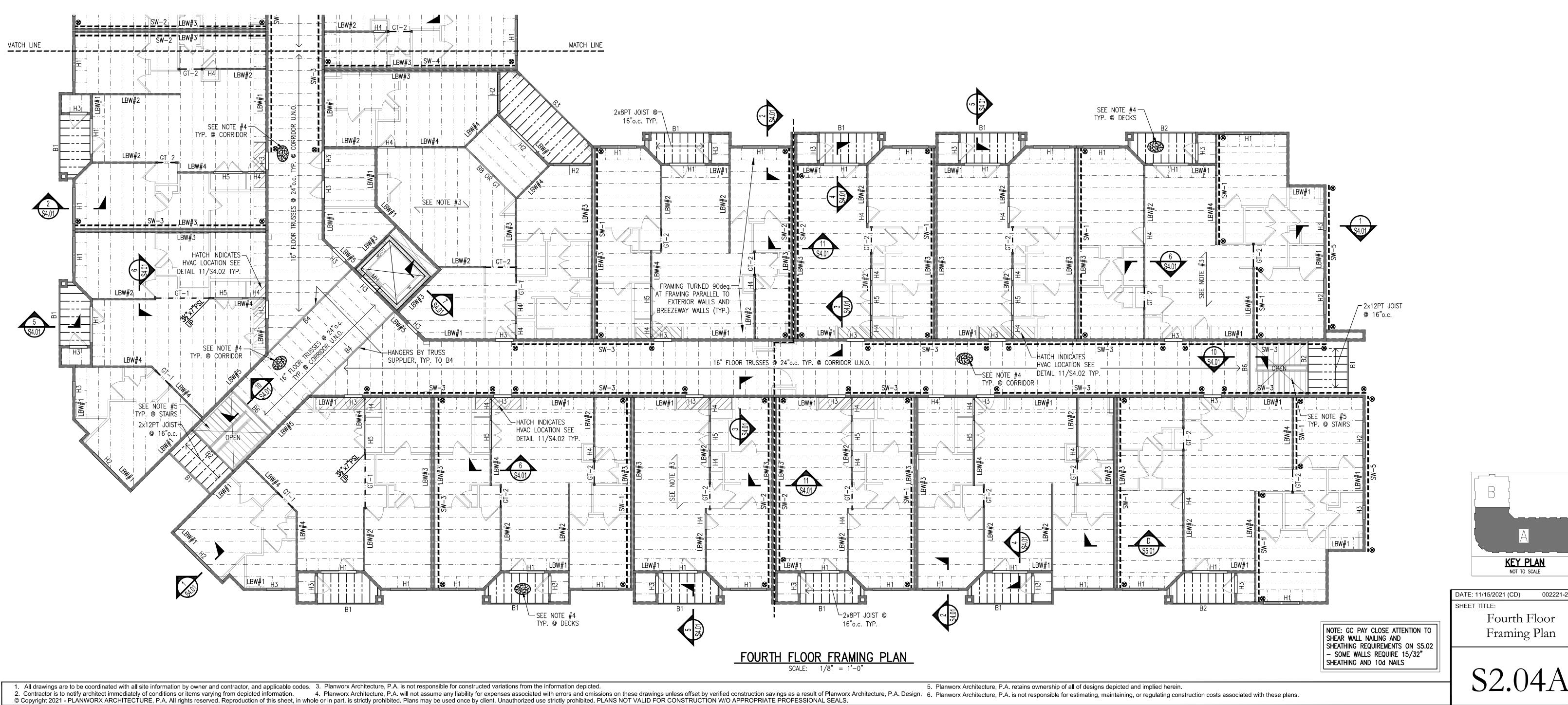


BE	AM SCHEDULE			) stud )n pla	-
MARK	BEAM REQUIREMENT	FL <b>#</b> 1	FL <b>#</b> 2	FL <b>#</b> 3	FL #4
B1	(2) 2x12 PT	3	3	3	2
B2	3½" x 11¼" PT GLULAM	4	3	3	3
B3	5½" x 11¼" PT GLULAM	4	3	3	3
B4	(4) 1¾" x 16" LVLs	6	5	4	3
B5	(2) 1¾" × 18" LVLs		SEE	PLAN	
B6	3½" x 16" PT GLULAM	4	3	3	3
B7	(3) 2x12 PT	3	3	3	3
B8	(2) 1¾" x 16" LVLs	5	4	3	3
B9	(2) 1¾" x 11¼" LVLs	SEE PLAN	_	_	_
B10					

3. SEE GENERAL WALL FRAMING DETAILS ON SHEET S1.0 SHEET SERIES



BEAM SCHEDULE		BUNDLED STUDS (U.N.O. ON PLAN)			
MARK	BEAM REQUIREMENT	FL #1	FL <b>#</b> 2	FL <b>#</b> 3	FL <b>#</b> 4
B1	(2) 2x12 PT	3	3	3	2
B2	3½" x 11¼" PT GLULAM	4	3	3	3
B3	5½" x 11¼" PT GLULAM	4	3	3	3
B4	(4) 1¾" x 16" LVLs	6	5	4	3
B5	(2) 1¾" x 18" LVLs		SEE	PLAN	
B6	3½" x 16" PT GLULAM	4	3	3	3
B7	(3) 2x12 PT	3	3	3	3
B8	(2) 1¾" x 16" LVLs	5	4	3	3
В9	(2) 1¾" x 11¼" LVLs	SEE PLAN	_	_	_
B10					



	HEADER SCHEDULE	JAME	B REQ	UIREM	ENTS
MARK	HEADER REQUIREMENTS	FL #1	FL <b>#</b> 2	FL <b>#</b> 3	FL #4
H1	(3) 2x8 W/ $\frac{1}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H2	(3) 2x10 W/ $\frac{7}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H3	(3) 2x8 W/ $\frac{1}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	1K/1J	1K/1J
H4	(2) 2x8 W/ $\frac{1}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H5	(2) 2x12 W/ $\%_6$ " O.S.B. PR PLYWOOD BETWEEN PLYS	3K/2J	2K/2J	2K/1J	2K/1J
H6	(3) 2x10 W/ 7/6" O.S.B. PR PLYWOOD BETWEEN PLYS	4K/2J	-	-	_

<u>FLOOR FRAMING LEGEND</u>	FLOOR	FRAMING	LEGEND
-----------------------------	-------	---------	--------

	18" FLOOR TRUSSES @ 24" o.c. U.N.O.
	2x8PT JOISTS @ 16"o.c. (SEE ARCH. FOR SLOPE REQUIREMENTS)
Hx	HEADER, SEE SCHEDULE THIS SHEET
Bx	WOOD BEAM, SEE SCHEDULE THIS SHEET
	2x WALLS (ABOVE)
	LOAD BEARING WALLS (BELOW)
GT	INDICATES GIRDER TRUSS SEE SCHEDULE THIS SHEET
	INDICATES CHORD TRUSS SEE DETAIL ON S4.01
	BUNDLED STUDS DOWN (TYPICAL U.N.O.) SEE GENERAL FRAMING NOTES ON SHEET S1.01
SW-x	7/16" OR 15/32" O.S.B. SHEAR WALL SEE S5.0 SHEET SERIES FOR DETAILS SW-x INDICATES SHEAR WALL NUMBER
8	INDICATES APPROXIMATE LOCATION OF SHEAR WALL HOLDOWN SEE SCHEDULE ON S5.02
MH	MASONRY HEADER MH1: (1)#5 – GROUT (2) COURSES AE
XXXXXXXXXX	8" MASONRY WALL, W/ #5 @ CORNERS, JAMBS AND MAX SPACING @48"o.c. U.N.O

LOAD BEARING WALL (LBW #X) SCHEDULE						
FLOOR			STUD WALL REQU	IREMENT BY TYPE		
LEVEL	LBW #1	LBW #2	LBW #3	LBW #4	LBW #5	LBW #6
4th	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
3rd	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
2nd	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
1st	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(3) 2x4 @ 16"o.c.	(2) 2x6 @ 16"o.c.	2x8 @ 16"o.c.
NOTE = $1$	ALL STUDS TO BE	SPF #2 (NORTH)	OR BETTER - EXCE	PT AS NOTED BELO	W AND IN SCHEDULE	

NULE = 1. ALL SIUDS ID BE SPF #2 (NORTH) OR BEITER - EXCEPT AS NUTED BELOW AND IN SCHEDULE 2. ALL STUDS ARE REQUIRED TO ALIGN WITH TRUSSES AND STUDS ABOVE. JACK STUDS ARE REQUIRED IN THE FLOOR CAVITY WHERE STUDS OR TRUSS ABOVE IS LOCATED OUTSIDE THE

ALLOWABLE MISALIGNMENT DETAIL - SEE S1.0 SHEET SERIES.



### FLOOR FRAMING PLAN NOTES:

- 1. FOR GENERAL FRAMING INFORMATION, SEE SHEET S1.01
- 2. HEADERS, BEAMS & LOAD BEARING WALLS SHOWN ARE FOR FRAMING BELOW THIS LEVEL. SHEAR WALLS REFER TO WALLS BELOW.
- 3. SUBFLOOR SHALL BE EXTERIOR GRADE 3/4" TONGUE AND GROOVE O.S.B..
- 4. DECK AND CORRIDOR SUB FLOOR BE 3/4" PT PLYWOOD WITH 2" N.W. PEA GRAVEL CONCRETE TOPPING AT DECKS AND 11/3" @ COORDIORS, 4000psi W/ AIR ENTRAINMENT WITH LIGHT BROOM FINISH. REINFORCE W/ 2.5lbs/yd3 OF SYNTHETIC MARCRO-FIBER OR WWF 6x6xW2.0xW2.0. PROVIDE CONTROL JOINTS AT CORNERS AND APPROXIMATELY 8'-0"o.c.
- 5. STAIRS SHALL HAVE STEEL STRINGERS WITH CONCRETE TREADS PER ARCH.
- 6. WHEN A SECTION IS CUT OR A DETAIL IS LABELED FOR A PARTICULAR CONDITION, THAT SECTION OR DETAIL SHALL APPLY FOR ALL SIMILAR CONDITIONS REGARDLESS OF WHETHER CUT OR LABELED, U.N.O

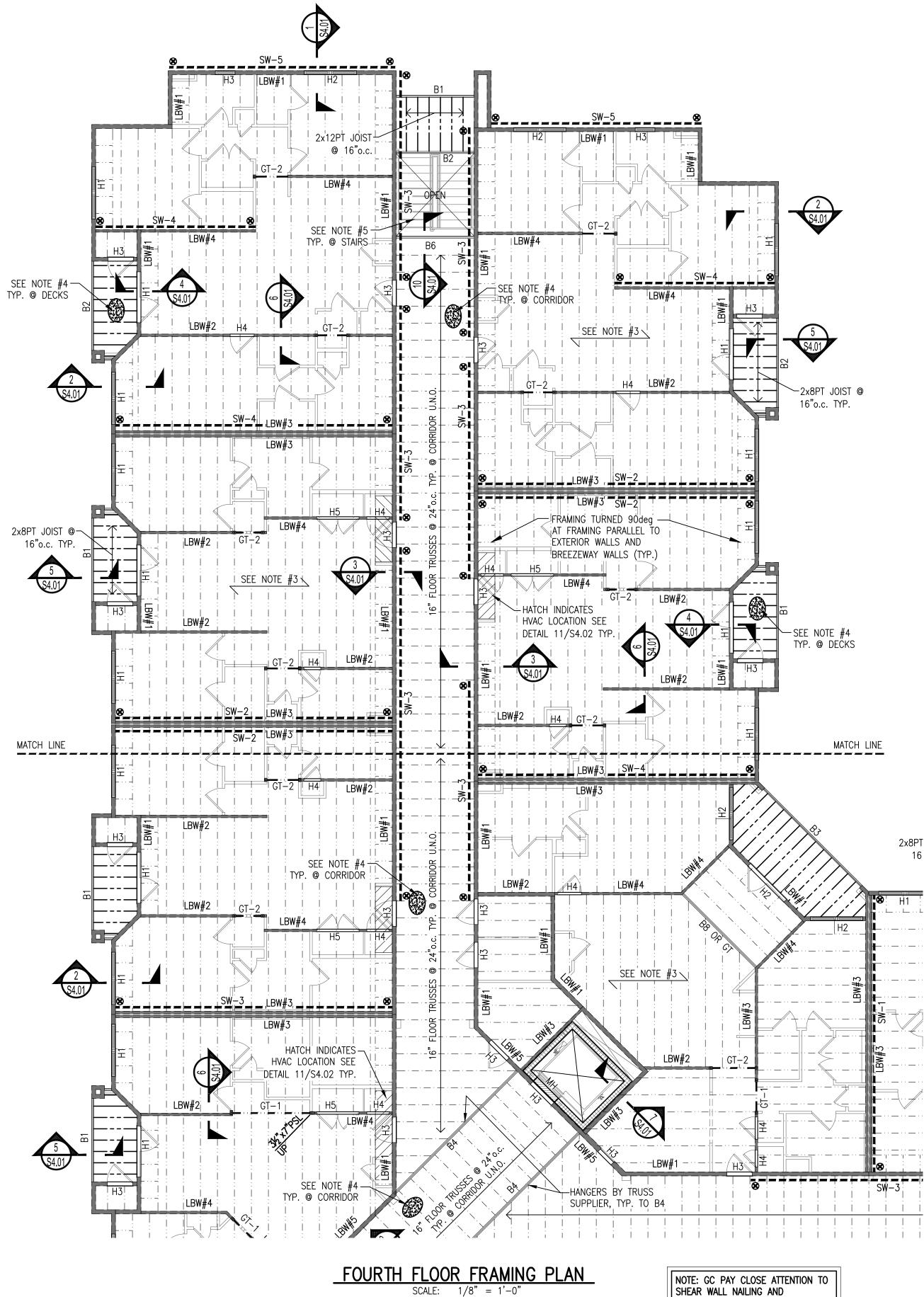
GIRDER	GIRDER TRUSS SCHEDULE		UNDLED		-
MARK	GIRDER TRUSS	FL <b>#</b> 1	FL <b>#</b> 2	FL <b>#</b> 3	FL #4
GT-1	18" TRUSS SUPPLIER	31∕2"x7" PSL	5	3	3
GT-2	18" TRUSS SUPPLIER	4	3	3	3
GT-3	18" TRUSS SUPPLIER	5	4	3	3

.OCATION N SEE COURSES ABOVE

PLANWORX ARCHITECTURE
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No. 86896

KEY PLAN NOT TO SCALE

002221-21-3332



SHEAR WALL NAILING AND SHEATHING REQUIREMENTS ON S5.02 - SOME WALLS REQUIRE 15/32" SHEATHING AND 10d NAILS

### FLOOR FRAMING LEGEND

	18" FLOOR TRUSSES @ 24" o.c. U.N.O.
	2x8PT JOISTS @ 16"o.c. (SEE ARCH. FOR SLOPE REQUIREMENTS)
Hx	HEADER, SEE SCHEDULE THIS SHEET
Bx	WOOD BEAM, SEE SCHEDULE THIS SHEET
	2x WALLS (ABOVE)
	LOAD BEARING WALLS (BELOW)
GT	INDICATES GIRDER TRUSS SEE SCHEDULE THIS SHEET
CT	INDICATES CHORD TRUSS SEE DETAIL ON S4.01
	BUNDLED STUDS DOWN (TYPICAL U.N.O.) SEE GENERAL FRAMING NOTES ON SHEET S1.01
<u>SW-x</u>	7/16" OR 15/32" O.S.B. SHEAR WALL SEE S5.0 SHEET SERIES FOR DETAILS SW—x INDICATES SHEAR WALL NUMBER
8	INDICATES APPROXIMATE LOCATION OF SHEAR WALL HOLDOWN SEE SCHEDULE ON S5.02
MH	MASONRY HEADER MH1: (1)#5 – GROUT (2) COURSES ABOVE
*****	8" MASONRY WALL, W/ #5 @ CORNERS, JAMBS AND MAX SPACING @48"o.c. U.N.O

HEADER SCHEDULE			JAMB REQUIREMENTS			
MARK	HEADER REQUIREMENTS	FL <b>#</b> 1	FL #2	FL <b>#</b> 3	FL <b>#</b> 4	
H1	(3) 2x8 W/ $7_{6}$ ° 0.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J	
H2	(3) 2x10 W/ $\%_6$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J	
Н3	(3) 2x8 W/ $7_6$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	1K/1J	1K/1J	
H4	(2) 2x8 W/ $\frac{7}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J	
H5	(2) 2x12 W/ $\frac{7}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	3K/2J	2K/2J	2K/1J	2K/1J	
H6	(3) 2x10 W/ $\mathcal{H}_6$ " O.S.B. PR PLYWOOD BETWEEN PLYS	4K/2J	_	-	_	

GIRDER	TRUSS SCHEDULE	BUNDLED STU (U.N.O. ON PL			
MARK	GIRDER TRUSS	FL #1	FL <b>#</b> 2	FL #	
GT-1	18" TRUSS SUPPLIER	31∕2"x7" PSL	5	3	
GT-2	18" TRUSS SUPPLIER	4	3	3	
GT-3	18" TRUSS SUPPLIER	5	4	3	

LOAD BEARING WALL (LBW #X) SCHEDULE							
FLOOR	OR STUD WALL REQUIREMENT BY TYPE						
LEVEL	LBW #1	LBW #2	LBW #3	LBW #4	LBW #5	LBW #6	
4th	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.	
3rd	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.	
2nd	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.	
1st	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(3) 2x4 @ 16"o.c.	(2) 2x6 @ 16"o.c.	2x8 @ 16"o.c.	
2	NOTE = 1. ALL STUDS TO BE SPF #2 (NORTH) OR BETTER - EXCEPT AS NOTED BELOW AND IN SCHEDULE 2. ALL STUDS ARE REQUIRED TO ALIGN WITH TRUSSES AND STUDS ABOVE. JACK STUDS ARE REQUIRED IN THE FLOOR CAVITY WHERE STUDS OR TRUSS ABOVE IS LOCATED OUTSIDE THE ALLOWABLE MISALIGNMENT DETAIL - SEE S1.0 SHEET SERIES. 3. SEE CENERAL WALL FRAMING DETAILS ON SHEET SERIES.						

1. All drawings are to be coordinated with all site information by owner and contractor, and applicable codes. 3. Planworx Architecture, P.A. is not responsible for constructed variations from the information depicted. 5. Planworx Architecture, P.A. retains ownership of all of designs depicted and implied herein. 2. Contractor is to notify architect immediately of conditions or items varying from depicted information. 4. Planworx Architecture, P.A. will not assume any liability for expenses associated with errors and omissions on these drawings unless offset by verified construction savings as a result of Planworx Architecture, P.A. is not responsible for estimating, maintaining, or regulating construction costs associated with these plans. © Copyright 2021 - PLANWORX ARCHITECTURE, P.A. All rights reserved. Reproduction of this sheet, in whole or in part, is strictly prohibited. Plans may be used once by client. Unauthorized use strictly prohibited. PLANS NOT VALID FOR CONSTRUCTION W/O APPROPRIATE PROFESSIONAL SEALS.

#### FLOOR FRAMING PLAN NOTES:

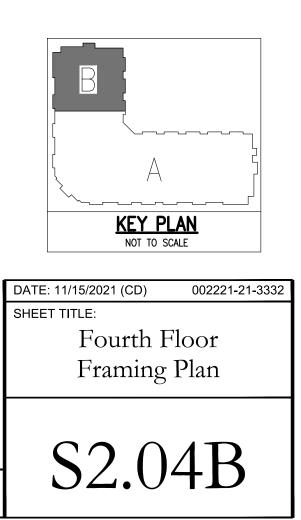
- 1. FOR GENERAL FRAMING INFORMATION, SEE SHEET S1.01
- 2. HEADERS, BEAMS & LOAD BEARING WALLS SHOWN ARE FOR FRAMING BELOW THIS LEVEL. SHEAR WALLS REFER TO WALLS BELOW.
- 3. SUBFLOOR SHALL BE EXTERIOR GRADE 3/4" TONGUE AND GROOVE O.S.B.
- 4. DECK AND CORRIDOR SUB FLOOR BE  $\frac{3}{4}$ " PT PLYWOOD WITH 2" N.W. PEA GRAVEL CONCRETE TOPPING AT DECKS AND 1½" @ COORDIORS, 4000psi W/ AIR ENTRAINMENT WITH LIGHT BROOM FINISH. REINFORCE W/ 2.5lbs/yd3 OF SYNTHETIC MARCRO-FIBER OR WWF 6x6xW2.0xW2.0. CONTROL JOINTS AT CORNERS AND APPROXIMATELY 8'-0"o.c.
- STRINGERS WITH CONCRETE TREADS PER ARCH
- WHEN A SECTION IS CUT OR A DETAIL IS LABELED FOR A PARTICULAR CONDITION, THAT SECTION OR DETAIL SHALL APPLY FOR ALL SIMILAR CONDITIONS REGARDLESS OF WHETHER CUT OR LABELED, U.N.O



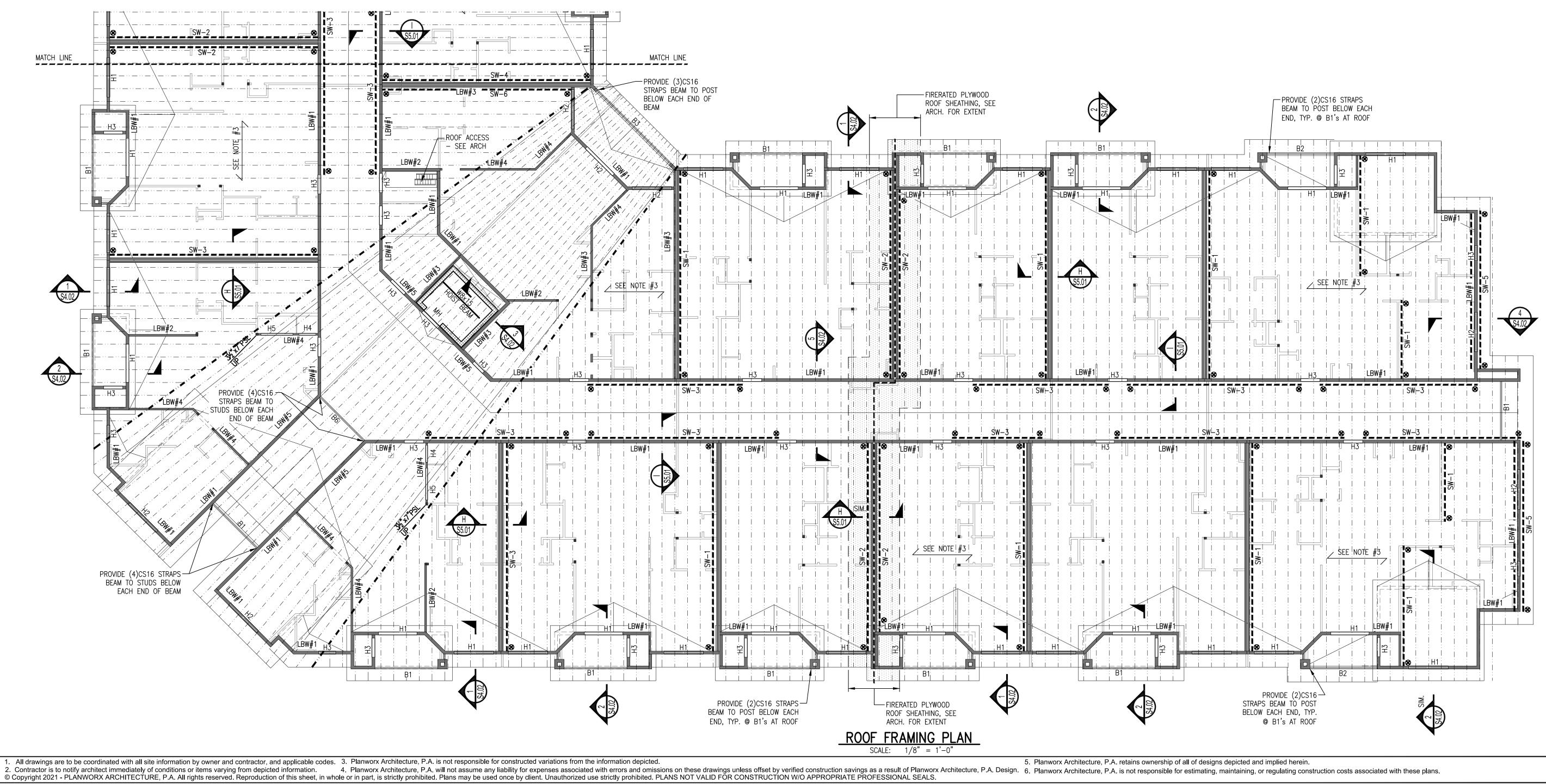


BEAM SCHEDULE		BUNDLED STUDS (U.N.O. ON PLAN)				
MARK	BEAM REQUIREMENT	FL <b>#</b> 1	FL <b>#</b> 2	FL <b>#</b> 3	FL #4	
B1	(2) 2x12 PT	3	3	3	2	
B2	3½" x 11¼" PT GLULAM	4	3	3	3	
B3	5½" x 11¼" PT GLULAM	4	3	3	3	
B4	(4) 1 <sup>3</sup> ⁄ <sub>4</sub> " x 16" LVLs	6	5	4	3	
B5	(2) 1 <sup>3</sup> ⁄ <sub>4</sub> " x 18" LVLs		SEE	PLAN		
B6	3½" x 16" PT GLULAM	4	3	3	3	
B7	(3) 2x12 PT	3	3	3	3	
B8	(2) 1 <sup>3</sup> ⁄ <sub>4</sub> " × 16" LVLs	5	4	3	3	
B9	(2) 1 <sup>3</sup> ⁄ <sub>4</sub> " x 11⁄⁄ <sub>4</sub> " LVLs	SEE PLAN	-	-	_	
B10						

3. SEE GENERAL WALL FRAMING DETAILS ON SHEET S1.0 SHEET SERIES

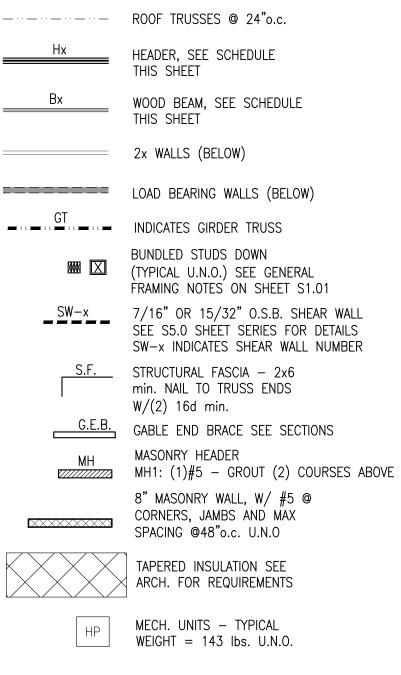


BE	AM SCHEDULE	BUNDLED STUDS (U.N.O. ON PLAN)				
MARK	BEAM REQUIREMENT	FL <b>#</b> 1	FL <b>#</b> 2	FL <b>#</b> 3	FL <b>#</b> 4	
B1	(2) 2x12 PT	3	3	3	2	
B2	3½" x 11¼" PT GLULAM	4	3	3	3	
B3	5½" x 11¼" PT GLULAM	4	3	3	3	
B4	(4) 1¾" x 16" LVLs	6	5	4	3	
B5	(2) 1¾" x 18" LVLs	SEE PLAN				
B6	3½" x 16" PT GLULAM	4	3	3	3	
B7	(3) 2x12 PT	3	3	3	3	
B8	(2) 1¾" x 16" LVLs	5	4	3	3	
B9	(2) 1¾" x 11¼" LVLs	SEE PLAN	-	-	-	
B10						



HEADER SCHEDULE			JAMB REQUIREMENTS				
MARK	HEADER REQUIREMENTS	FL <b>#</b> 1	FL <b>#</b> 2	FL <b>#</b> 3	FL <b>#</b> 4		
H1	(3) 2x8 W/ 7/6" O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J		
H2	(3) 2x10 W/ 7/6" O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J		
Н3	(3) 2x8 W/ 7/6" O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	1K/1J	1K/1J		
H4	(2) 2x8 W/ 7/6" O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J		
H5	(2) 2x12 W/ $\frac{1}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	3K/2J	2K/2J	2K/1J	2K/1J		
H6	(3) 2x10 W/ 7/6" O.S.B. PR PLYWOOD BETWEEN PLYS	4K/2J	_	_	_		

ROOF FRAMING LEGEND
---------------------



	LOAD BEARING WALL (LBW #X) SCHEDULE							
FLOOR		STUD WALL REQUIREMENT BY TYPE						
LEVEL	LBW #1	LBW #2	LBW #3	LBW #4	LBW #5	LBW #6		
4th	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.		
3rd	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.		
2nd	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.		
1st	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(3) 2x4 @ 16"o.c.	(2) 2x6 @ 16"o.c.	2x8 @ 16"o.c.		
NOTE = $1$	I. ALL STUDS TO BE	SPF #2 (NORTH)	OR BETTER – EXCE	PT AS NOTED BELO	W AND IN SCHEDULE			

2. ALL STUDS ARE REQUIRED TO ALIGN WITH TRUSSES AND STUDS ABOVE. JACK STUDS ARE REQUIRED IN THE FLOOR CAVITY WHERE STUDS OR TRUSS ABOVE IS LOCATED OUTSIDE THE

ALLOWABLE MISALIGNMENT DETAIL - SEE S1.0 SHEET SERIES. 3. SEE GENERAL WALL FRAMING DETAILS ON SHEET S1.0 SHEET SERIES

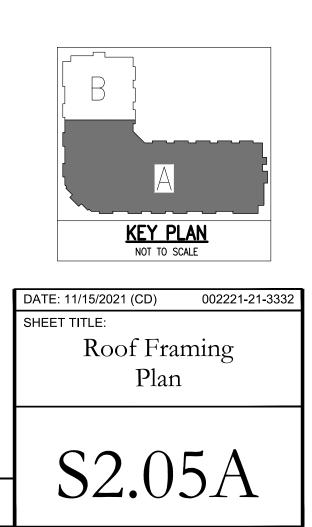
### **ROOF FRAMING PLAN NOTES:**

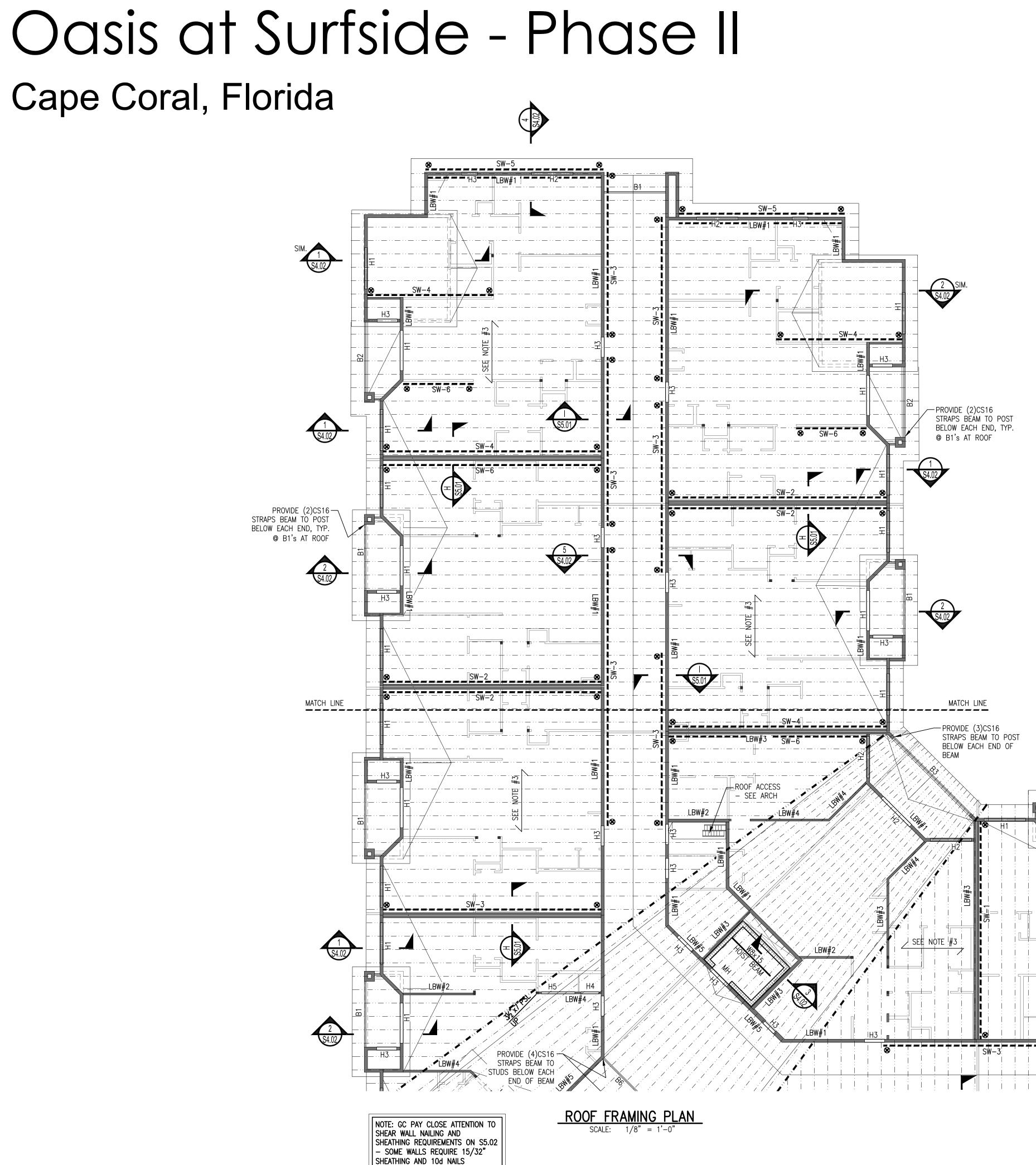
- 1. FOR GENERAL FRAMING INFORMATION, SEE SHEET S1.01
- 2. HEADERS, BEAMS, LOAD BEARING WALLS, AND SHEAR WALLS SHOWN ARE FOR FRAMING BELOW THIS LEVEL.
- 3. ROOF SHEATHING SHALL BE %" EXTERIOR GRADE PLYWOOD. SPAN AS NOTED ON
- 4. WHEN A SECTION IS CUT OR A DETAIL IS LABELED FOR A PARTICULAR CONDITION, THAT SECTION OR DETAIL SHALL APPLY FOR ALL SIMILAR CONDITIONS REGARDLESS OF WHETHER CUT OR LABELED, U.N.O
- 5. SEE SECTION 7-7 ON SHEET S4.02 FOR ROOF TOP MECHANICAL UNIT SUPPORTS.

NOTE: CONTRACTOR TO ASSUME A SIMPSON MGT HOLDOWN @ EACH END OF ALL GIRDER, HIP, AND VALLEY TRUSSES @ ROOF LEVEL. CONNECTIONS @ STUD BUNDLES TO BE FOLLOWED DOWN TO FOUNDATION. ALL OTHER TRUSSES SHALL HAVE A SIMPSON H10A CONNECTION U.N.O ACTUAL HD'S TO BE DETERMINED ONCE LOAD ARE GIVEN BY THE TRUSS SUPPLIER

NOTE: CONTRACTOR TO PROVIDE MIN. (3) STUDS UNDER ALL GIRDER TRUSSES UNLESS NOTED OTHERWISE.







### ROOF FRAMING LEGEND

	ROOF TRUSSES @ 24"o.c.
Hx	HEADER, SEE SCHEDULE THIS SHEET
Bx	WOOD BEAM, SEE SCHEDULE THIS SHEET
	2x WALLS (BELOW)
	LOAD BEARING WALLS (BELOW)
GT	INDICATES GIRDER TRUSS
	BUNDLED STUDS DOWN (TYPICAL U.N.O.) SEE GENERAL FRAMING NOTES ON SHEET S1.01
	7/16" OR 15/32" O.S.B. SHEAR WALL SEE S5.0 SHEET SERIES FOR DETAILS SW—x INDICATES SHEAR WALL NUMBER
S.F.	STRUCTURAL FASCIA — 2x6 min. NAIL TO TRUSS ENDS W/(2) 16d min.
<u> </u>	GABLE END BRACE SEE SECTIONS
MH	MASONRY HEADER MH1: (1)#5 – GROUT (2) COURSES ABOVE
XXXXXXXXXX	8" MASONRY WALL, W/ <b>#</b> 5 @ CORNERS, JAMBS AND MAX SPACING @48"o.c. U.N.O

HP

MECH. UNITS – TYPICAL WEIGHT = 143 lbs. U.N.O.

TAPERED INSULATION SEE ARCH. FOR REQUIREMENTS

HEADER SCHEDULE			B REQ	UIREM	ENTS
MARK	HEADER REQUIREMENTS	FL <b>#</b> 1	FL <b>#</b> 2	FL <b>#</b> 3	FL <b>#</b> 4
H1	(3) 2x8 W/ $7_{16}$ " 0.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H2	(3) 2x10 W/ $7_{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H3	(3) 2x8 W/ $7_{16}$ " 0.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	1K/1J	1K/1J
H4	(2) 2x8 W/ $7_{16}$ " 0.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H5	(2) 2x12 W/ $\frac{7}{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	3K/2J	2K/2J	2K/1J	2K/1J
H6	(3) 2x10 W/ $\%_6$ " O.S.B. PR PLYWOOD BETWEEN PLYS	4K/2J	-	-	-

GIRDER	TRUSS SCHEDULE	E BUNDLED STU (U.N.O. ON P			
MARK	GIRDER TRUSS	FL #1	FL <b>#</b> 2	FL #	
GT-1	18" TRUSS SUPPLIER	31∕2"x7" PSL	5	3	
GT-2	18" TRUSS SUPPLIER	4	3	3	
GT-3	18" TRUSS SUPPLIER	5	4	3	

LOAD BEARING WALL (LBW #X) SCHEDULE						
FLOOR	STUD WALL REQUIREMENT BY TYPE					
LEVEL	LBW #1	LBW #2	LBW #3	LBW #4	LBW #5	LBW #6
4th	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
3rd	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
2nd	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
1st	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(3) 2x4 @ 16"o.c.	(2) 2x6 @ 16"o.c.	2x8 @ 16"o.c.
NOTE = 1. ALL STUDS TO BE SPF #2 (NORTH) OR BETTER – EXCEPT AS NOTED BELOW AND IN SCHEDULE 2. ALL STUDS ARE REQUIRED TO ALIGN WITH TRUSSES AND STUDS ABOVE. JACK STUDS ARE REQUIRED IN THE FLOOR CAVITY WHERE STUDS OR TRUSS ABOVE IS LOCATED OUTSIDE THE ALLOWABLE MISALIGNMENT DETAIL – SEE S1.0 SHEET SERIES. 3. SEE CENERAL WALL FRAMING DETAILS ON SHEET STORES						

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### **ROOF FRAMING PLAN NOTES:**

- 1. FOR GENERAL FRAMING INFORMATION, SEE SHEET S1.01
- 2. HEADERS, BEAMS & LOAD BEARING WALLS SHOWN ARE FOR FRAMING BELOW THIS LEVEL. SHEAR WALLS REFER TO WALLS BELOW.
- 3. ROOF SHEATHING SHALL BE 5/8" EXTERIOR GRADE PLYWOOD SPAN AS NOTED ON PLAN.
- 4. WHEN A SECTION IS CUT OR A DETAIL IS LABELED FOR A PARTICULAR CONDITION. THAT SECTION OR DETAIL SHALL APPLY FOR ALL SIMILAR CONDITIONS REGARDLESS OF WHETHER CUT OR LABELED, U.N.O
- 5. SEE SECTION 7-7 SHEET S4.02 FOR ROOF TOP MECHANICAL UNIT SUPPORTS.



HOLDOWN @ EACH END OF ALL GIRDER, HIP, AND VALLEY TRUSSES @ ROOF LEVEL. CONNECTIONS @ STUD BUNDLES TO BE FOLLOWED DOWN TO FOUNDATION. ALL OTHER TRUSSES SHALL HAVE A SIMPSON H10A CONNECTION U.N.O ACTUAL HD'S TO BE DETERMINED ONCE LOAD ARE GIVEN BY THE TRUSS SUPPLIER

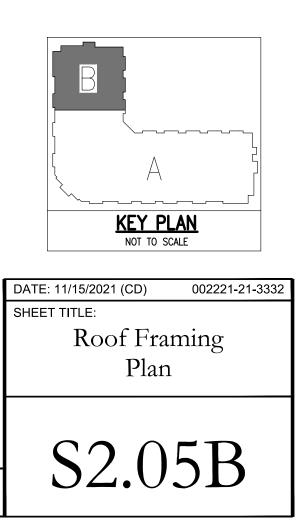
NOTE: CONTRACTOR TO ASSUME A SIMPSON MGT

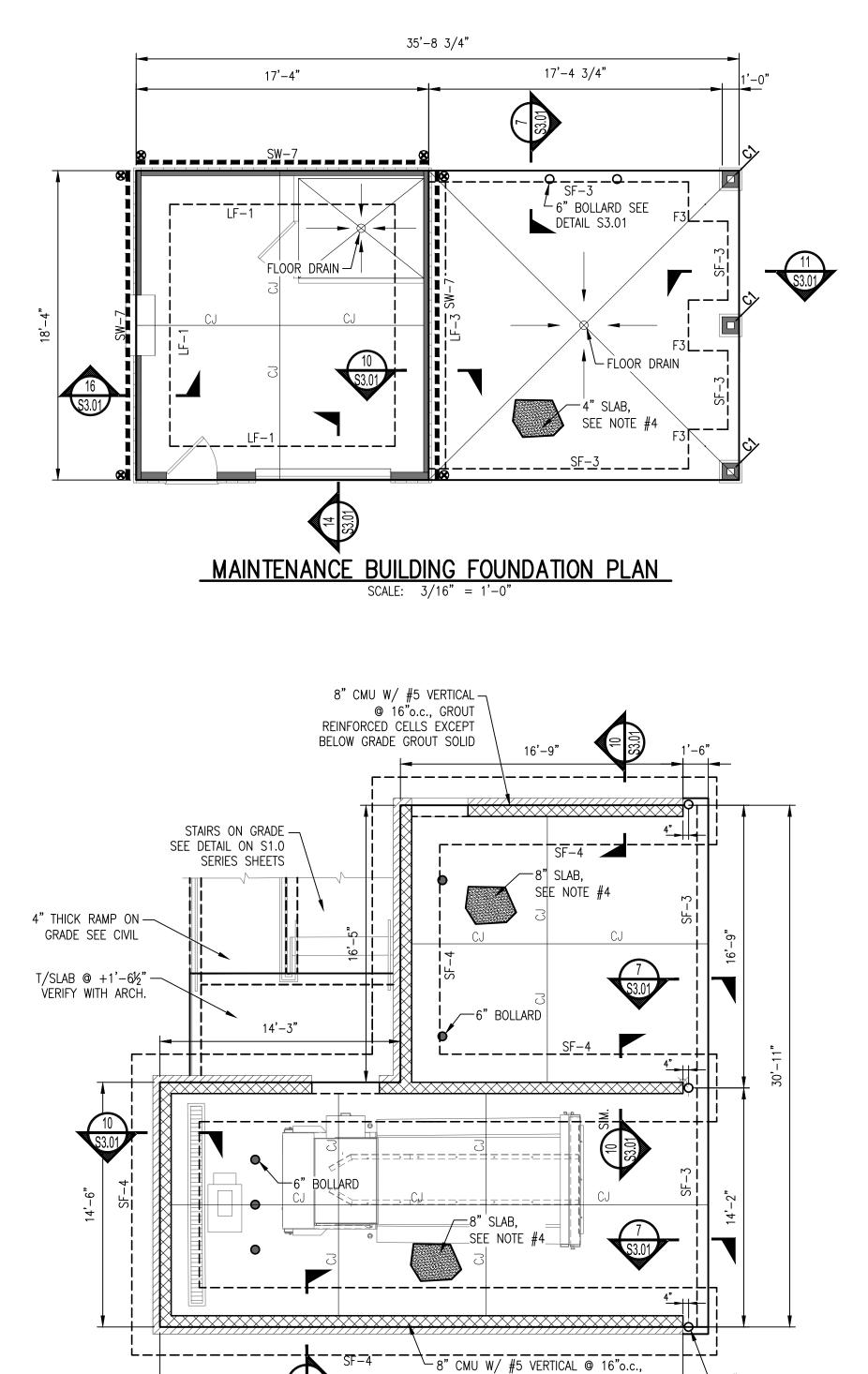
NOTE: CONTRACTOR TO PROVIDE MIN. (3) STUDS UNDER ALL GIRDER TRUSSES UNLESS NOTED OTHERWISE.



BE	AM SCHEDULE	BUNDLED STUDS (U.N.O. ON PLAN)			
MARK	BEAM REQUIREMENT	FL #1 FL #2 FL #3 FL			FL <b>#</b> 4
B1	(2) 2x12 PT	3	3	3	2
B2	3½" x 11¼" PT GLULAM	4	3	3	3
B3	5½" x 11¼" PT GLULAM	4	3	3	3
B4	(4) 1¾" x 16" LVLs	6	5	4	3
B5	(2) 1¾" x 18" LVLs	SEE PLAN			
B6	3½" x 16" PT GLULAM	4	3	3	3
B7	(3) 2x12 PT	3	3	3	3
B8	(2) 1¾" x 16" LVLs	5	4	3	3
B9	(2) 1¾" x 11¼" LVLs	SEE PLAN	_	_	_
B10					

3. SEE GENERAL WALL FRAMING DETAILS ON SHEET S1.0 SHEET SERIES





### TRASH COMPACTOR ENCLOSURE FOUNDATION PLAN

31'-0"

GROUT REINFORCED CELLS EXCEPT

BELOW GRADE GROUT SOLID

SCALE: 3/16'' = 1'-0''

	SPREAD FOOTING (FX) SCHEDULE				
MARK	SIZE length x width x thickness	REINFORCEMENT (BOTTOM BARS EACH WAY UNO)			
F2	2'-0" x 2'-0" x 2'-0"	(2) <b>#</b> 5 E.W.			
F3	3'-0" x 3'-0" x 2'-0"	(3) <b>#</b> 5 E.W.			
F4	4'-0" x 4'-0" x 1'-0"	(4) <b>#</b> 5 E.W.			
F4A	4'-0" x 4'-0" x 2'-0"	(4) <b>#</b> 5 E.W.			
F13.5x11.5	13'-6 x 11'-6" x 1'-0"	#5@12"o.c. EW BOTT.			

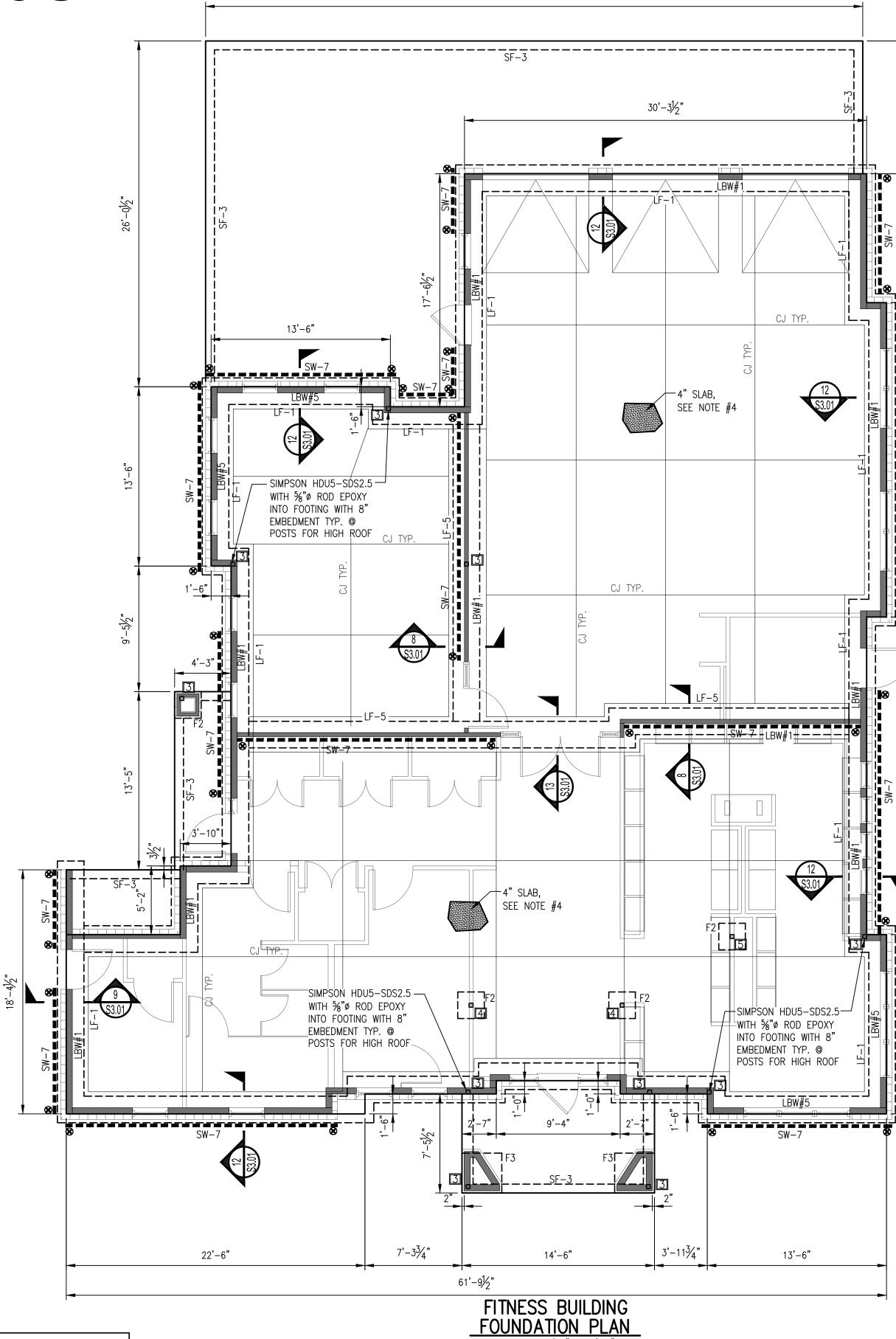
-6" BOLLARD SEE ARCH

FOR LOCATIONS AND

DETAIL THIS SHEET

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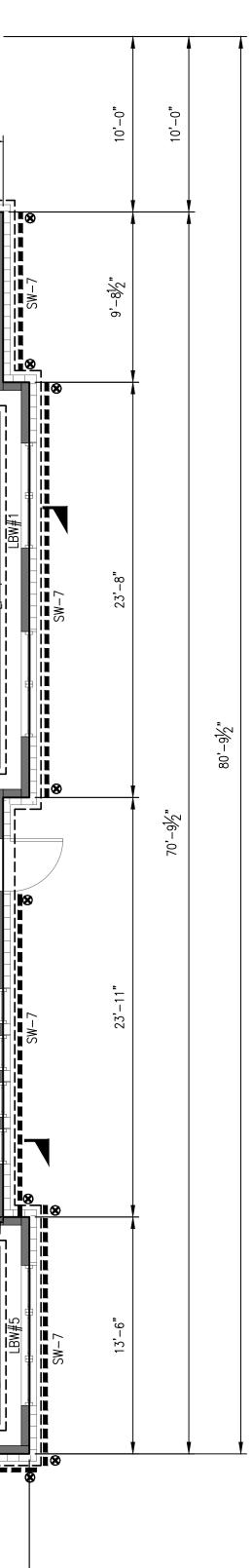


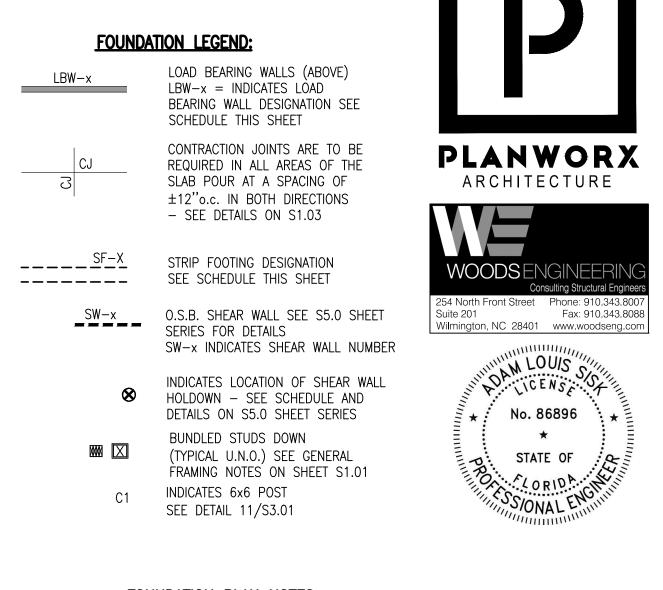


REMARKS	6	

	STRIP FOOTING (SF-X) SCHEDULE					
MARK	SIZE width x thickness x length	REINFORCEMENT (BOTTOM BARS UNO)	REMARKS			
SF-1	2'-0" x 2'-0" x CONT.	(3) #5 CONT. BOTT / (1) #4 CONT. TOP	MONOLITHIC WITH SLAB			
SF-2	2'-0" x 1'-0" x CONT.	(3) <b>#</b> 4 CONT.	MONOLITHIC WITH SLAB			
SF-3	0'-8" x 2'-0" x CONT.	(1) #4 CONT. TOP & BOTTOM	MONOLITHIC WITH SLAB			
SF-4	4'-0" x 1'-4" x CONT.	(5) <b>#</b> 5 CONT.	_			

5. Planworx Architecture, P.A. retains ownership of all of designs depicted and implied herein. 2. Contractor is to notify architect immediately of conditions or items varying from depicted information. 4. Planworx Architecture, P.A. will not assume any liability for expenses associated with errors and omissions on these drawings unless offset by verified construction savings as a result of Planworx Architecture, P.A. is not responsible for estimating, maintaining, or regulating construction costs associated with these plans. © Copyright 2021 - PLANWORX ARCHITECTURE, P.A. All rights reserved. Reproduction of this sheet, in whole or in part, is strictly prohibited. Plans may be used once by client. Unauthorized use strictly prohibited. PLANS NOT VALID FOR CONSTRUCTION W/O APPROPRIATE PROFESSIONAL SEALS.





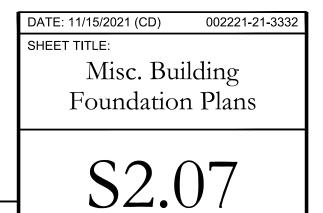
#### FOUNDATION PLAN NOTES:

- 1. SEE S1.0 SERIES SHEETS FOR ADDITIONAL GENERAL NOTES, FOUNDATION NOTES, CONCRETE NOTES, REINFORCING STEEL NOTES AND TYPICAL DETAILS. TYPICAL DETAILS ARE GENERALLY NOT SHOWN ON PLAN BUT RATHER ARE INTENDED TO DEFINE TYPICAL CONSTRUCTION CONDITIONS.
- 2. DATUM ELEVATION = TOP OF SLAB ELEVATION = ASSUMED 0'-0'' = ?'-?''M.S.L. OTHER ELEVATIONS ARE NOTED AS (+ OR -) FROM DATUM ELEVATION.
- 3. TOP OF FOOTINGS SHALL BE (-1'-4'') FROM DATUM ELEVATION, U.N.O.
- 4. SLAB-ON-GRADE SHALL BE 4" OR 8" THICK 3000 psi CONCRETE WITH 3.0lbs/yd.<sup>3</sup> OF SYNTHETIC MACRO-FIBERS (TUF-STRAND SF BY EUCLID, FIBER MAC SERIES BY BASF, OR FORTA-FERRO BY FORTA CORP, OR APPROVED EQUAL) ON 10 mil VAPOR BARRIER, ON 4" WELL COMPACTED GRANULAR FILL ON WELL COMPACTED SUB GRADE. VERIFY COMPACTION w/QUALIFIED GEOTECHNICAL ENGINEER.
- 5. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND OTHER DISCIPLINE DRAWINGS FOR OPENINGS AND DEPRESSIONS NOT SHOWN ON THESE DRAWINGS.
- 6. SEE S5.0 SHEETS FOR SHEAR WALL INFORMATION AND REQUIREMENTS
- 7. SEE ARCHITECTURAL DRAWINGS FOR BREEZEWAY SLAB SLOPE.
- 8. WHEN A SECTION IS CUT OR A DETAIL IS LABELED FOR A PARTICULAR CONDITION, THAT SECTION OR DETAIL SHALL APPLY FOR ALL SIMILAR CONDITIONS REGARDLESS OF WHETHER CUT OR LABELED, U.N.O

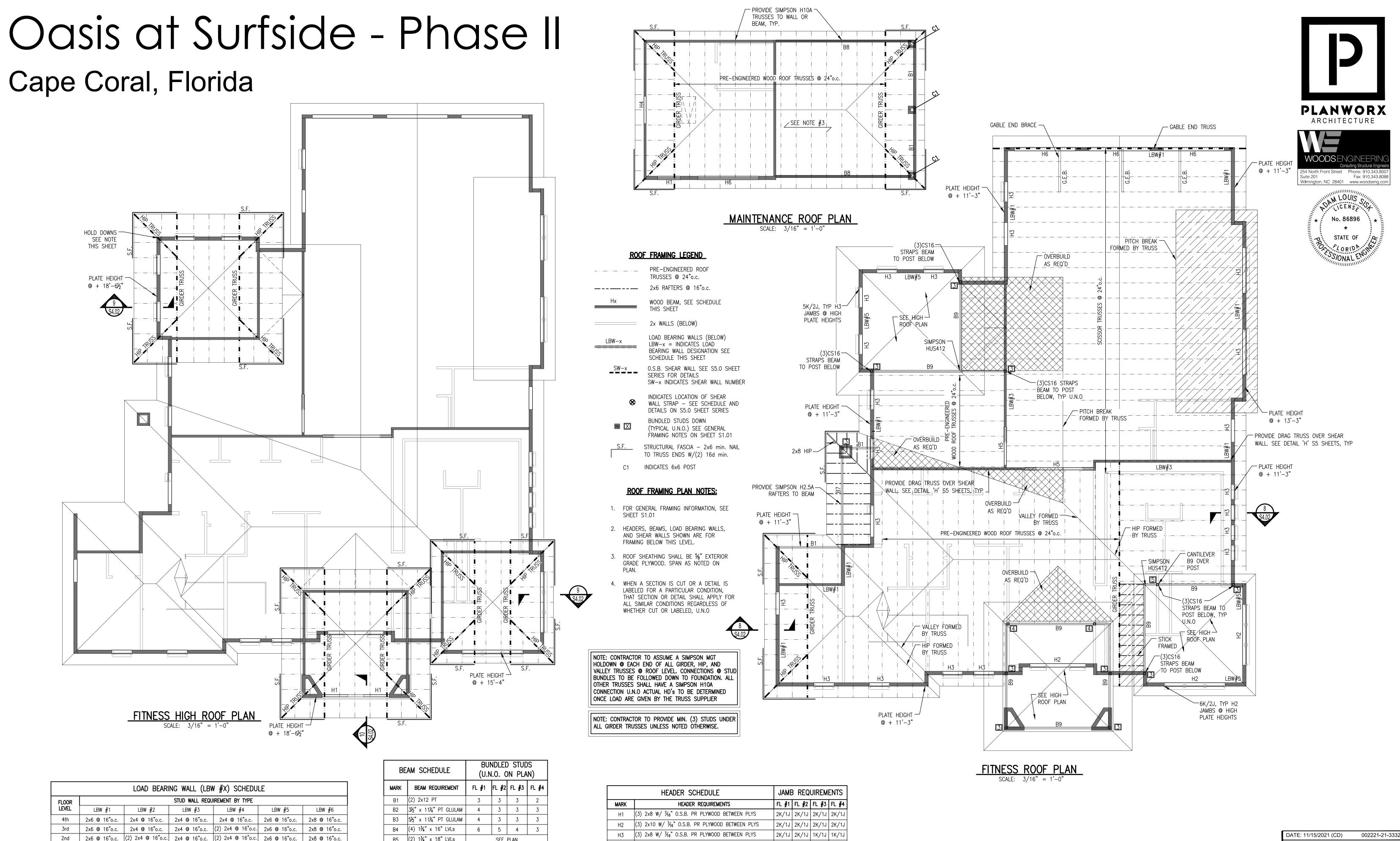
	LOAD BEARING WALL (LBW $\#X$ ) SCHEDULE					
FLOOR	FLOOR STUD WALL REQUIREMENT BY TYPE					
LEVEL	LBW #1	LBW #2	LBW #3	LBW #4	LBW #5	LBW #6
4th	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
3rd	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
2nd	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
1st	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(3) 2x4 @ 16"o.c.	(2) 2x6 @ 16"o.c.	2x8 @ 16"o.c.

NOTE = 1. ALL STUDS TO BE SPF #2 (NORTH) OR BETTER - EXCEPT AS NOTED BELOW AND IN SCHEDULE 2. ALL STUDS ARE REQUIRED TO ALIGN WITH TRUSSES AND STUDS ABOVE. JACK STUDS ARE REQUIRED IN THE FLOOR CAVITY WHERE STUDS OR TRUSS ABOVE IS LOCATED OUTSIDE THE ALLOWABLE MISALIGNMENT DETAIL - SEE S1.0 SHEET SERIES. 3. SEE GENERAL WALL FRAMING DETAILS ON SHEET S1.0 SHEET SERIES

LATERAL FOOTING (LF-X) SCHEDULE					
MARK	SIZE	REINFOF	REMARKS		
		width x thickness x length	BOTTOM	TOP	
LF-1	2'-0" x 2'-0" x CONT.	(4) #5 CONT.	(3) <b>#</b> 5 CONT.	SEE PLAN FOR ADD BARS	
LF-2	3'-0" x 1'-4" x CONT.	(4) #5 CONT.	(4) #5 CONT.	SEE PLAN FOR ADD BARS	
LF-3	3'-0" x 2'-0" x CONT.	(5) #6 CONT.	(5) #6 CONT.	SEE PLAN FOR ADD BARS	
LF-4	4'-0" x 2'-0" x CONT.	(6) #5 CONT.	(4) #5 CONT.	SEE PLAN FOR ADD BARS	
LF-5	2-0" x 1'-0" x CONT.	(3) #4 CONT.	(3) #4 CONT.	SEE PLAN FOR ADD BARS	



# Cape Coral, Florida



LOAD BEARING WALL (LBW #X) SCHEDULE						
FLOOR STUD WALL REQUIREMENT BY TYPE						
LEVEL	LBW #1	LBW #2	LBW #3	LBW #4	LBW #5	LBW #6
4th	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
3rd	2x6 @ 16"o.c.	2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
2nd	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	2x6 @ 16"o.c.	2x8 @ 16"o.c.
1st	2x6 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(2) 2x4 @ 16"o.c.	(3) 2x4 @ 16"o.c.	(2) 2x6 @ 16"o.c.	2x8 @ 16"o.c.
NOTF = 1	NOTE = 1. ALL STUDS TO BE SPF #2 (NORTH) OR BETTER - EXCEPT AS NOTED BELOW AND IN SCHEDULE					

2. ALL STUDS ARE REQUIRED TO ALIGN WITH TRUSSES AND STUDS ABOVE. JACK STUDS ARE REQUIRED IN THE FLOOR CAVITY WHERE STUDS OR TRUSS ABOVE IS LOCATED OUTSIDE THE ALLOWABLE MISALIGNMENT DETAIL - SEE S1.0 SHEET SERIES.

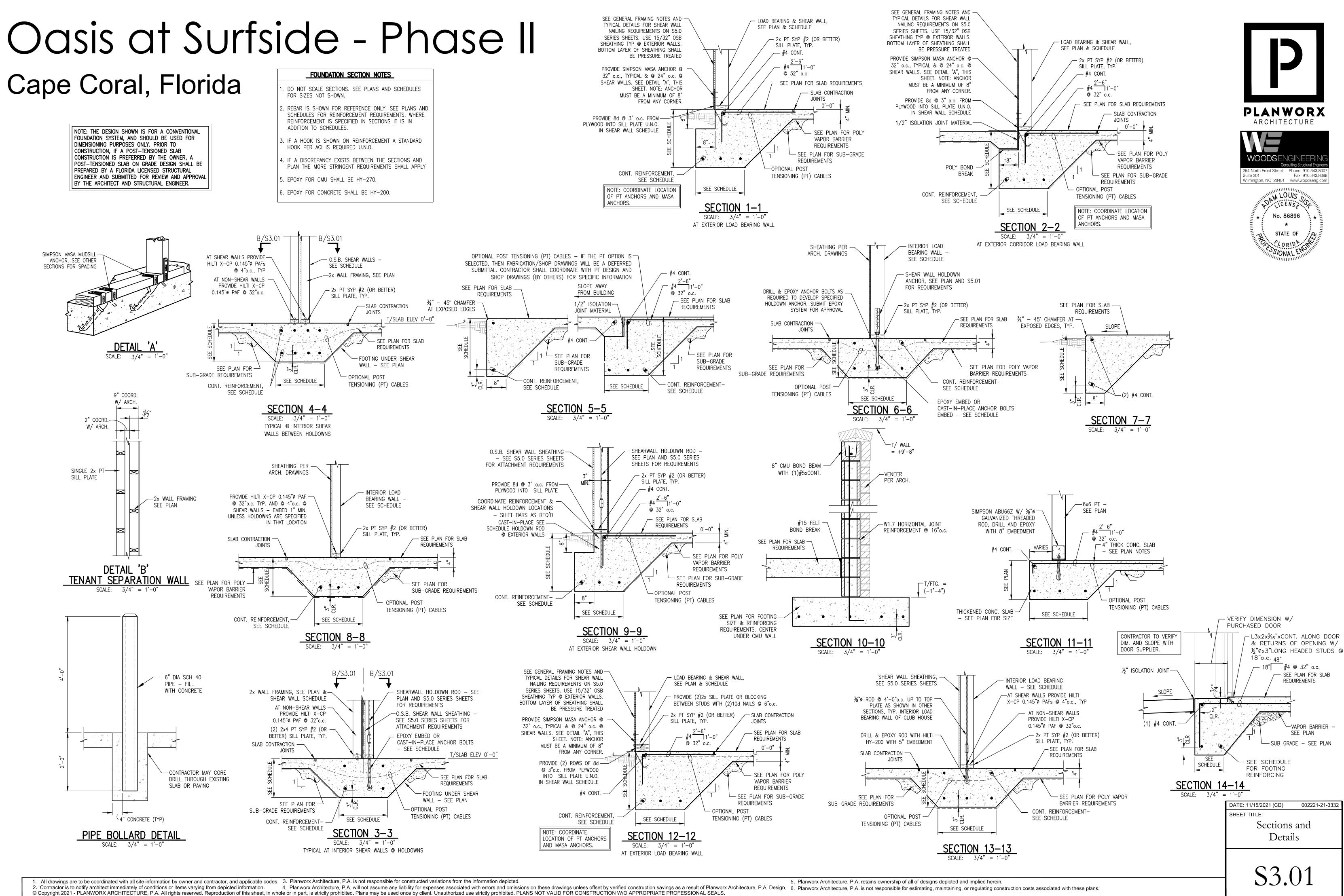
3. SEE GENERAL WALL FRAMING DETAILS ON SHEET S1.0 SHEET SERIES

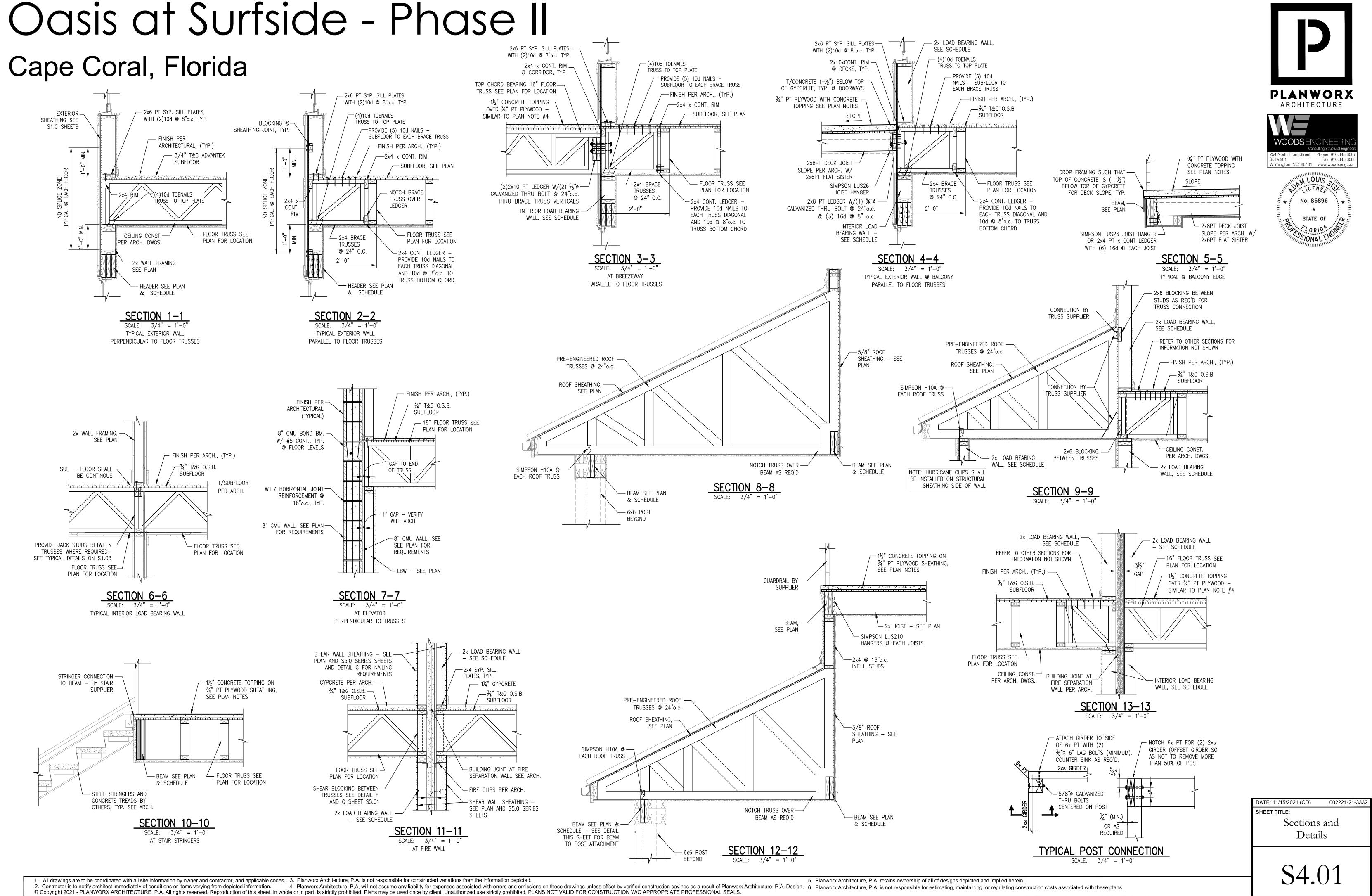
BEAM SCHEDULE		BUNDLED STUDS (U.N.O. ON PLAN)			-
MARK	BEAM REQUIREMENT	FL #1 FL #2 FL #3 FL			FL <b>#</b> 4
B1	(2) 2x12 PT	3	3	3	2
B2	3½" x 11¼" PT GLULAM	4	3	3	3
B3	5½" x 11¼" PT GLULAM	4	3	3	3
B4	(4) 1¾" x 16" LVLs	6	5	4	3
B5	(2) 1 <sup>3</sup> ⁄ <sub>4</sub> " x 18" LVLs	SEE PLAN			
B6	3½" x 16" PT GLULAM	4	3	3	3
B7	(3) 2x12 PT	3	3	3	3
B8	(2) 1¾" x 16" LVLs	5	4	3	3
B9	(2) 1 <sup>3</sup> ⁄ <sub>4</sub> " x 11⁄⁄ <sub>4</sub> " LVLs	SEE PLAN	-	-	-
B10					

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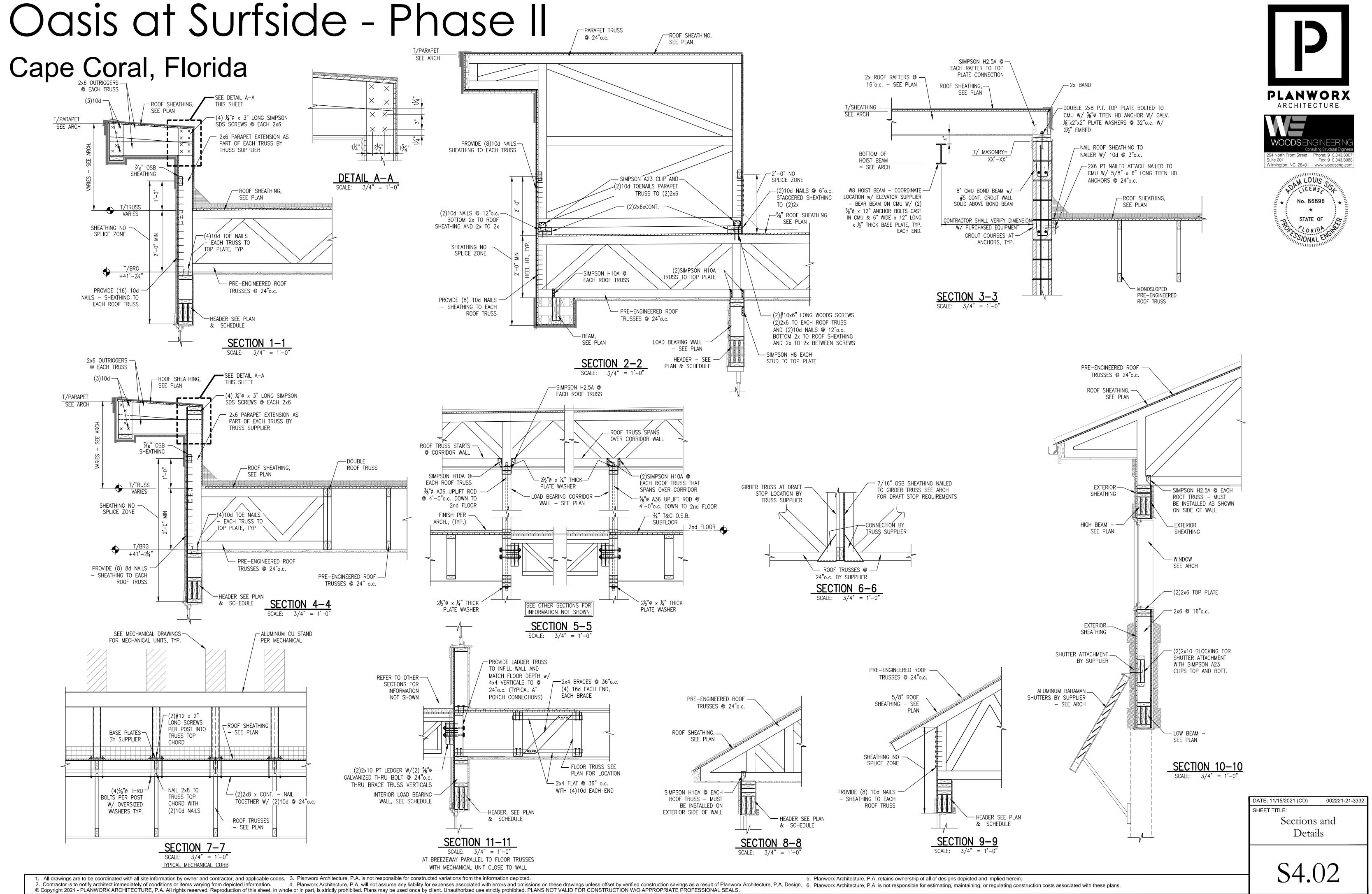
	HEADER SCHEDULE			UIREM	ENTS
MARK	HEADER REQUIREMENTS	FL #1	FL <b>#</b> 2	FL <b>#</b> 3	FL <b>#</b> 4
H1	(3) 2x8 W/ 7/6" O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H2	(3) 2x10 W/ 7/6" O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
Н3	(3) 2x8 W/ 7/6" O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	1K/1J	1K/1J
H4	(2) 2x8 W/ 7/6" O.S.B. PR PLYWOOD BETWEEN PLYS	2K/1J	2K/1J	2K/1J	2K/1J
H5	(2) 2x12 W/ 7/6" O.S.B. PR PLYWOOD BETWEEN PLYS	3K/2J	2K/2J	2K/1J	2K/1J
H6	(3) 2x10 W/ $7_{16}$ " O.S.B. PR PLYWOOD BETWEEN PLYS	4K/2J	-	-	-

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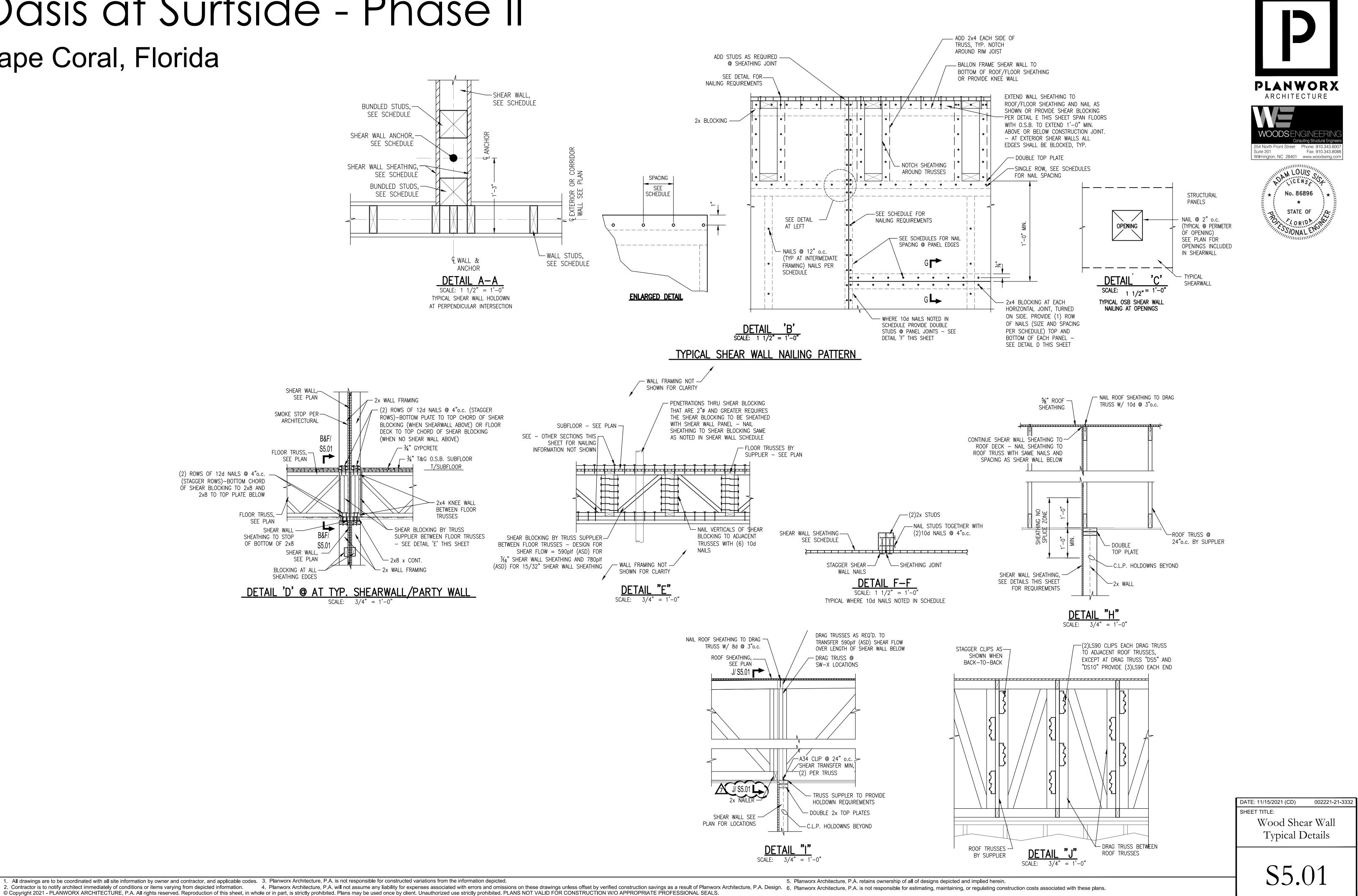




# T/PARAPET SEE ARCH



BUNDLED STUDS,-SEE SCHEDULE SEE SCHEDULE SEE SCHEDULE BUNDLED STUDS SEE SCHEDULE



		SHEAR WALL SW-1 (4 S	STORY)	
	SHEAR WALL	IS COMPRISED OF O.S.B SHEATHING V	WITH BLOCKING - SEE	DETAI
FLOOR	Sheathing requirements / Nail spacing	TOTAL # OF COMPRESSION STUDS EACH SIDE OF HOLDOWN ROD	MINIMUM THREADED ROD Ø	
4th to Roof	7/6" O.S.B. SHEATHING ONE SIDE W∕8d NAILS @ 3" o.c.	(2) 2x4	½"ø ROD	Shri Be
3rd to 4th	$\frac{7}{6}$ " O.S.B. SHEATHING ONE SIDE W/8d NAILS @ 3" o.c.	(3) 2x4	¾"ø ROD	Shri Be
2nd to 3rd	$\frac{7}{6}$ " O.S.B. SHEATHING ONE SIDE W/8d NAILS @ 3" o.c.	(4) 2x4	‰"ø ROD	Shri Be
SLAB to 2nd	<sup>15</sup> / <sub>32</sub> " O.S.B. SHEATHING ONE SIDE W/10d NAILS @ 3" o.c.	(4) 2x4	(2) <b>½</b> "ø ROD	EPC TABL

### CUEAD WALL CW 2/4 CTODY)

	SHEAR WALL SW-2 (4 STORY)					
	SHEAR WALL	IS COMPRISED OF O.S.B SHEATHING V	NITH BLOCKING - SEE	DETAILS SHEET S5.01		
FLOOR	SHEATHING REQUIREMENTS / NAIL SPACING	TOTAL # OF COMPRESSION STUDS EACH SIDE OF HOLDOWN ROD	MINIMUM THREADED ROD Ø	FLOOR CONNECTION		
4th to Roof	$\frac{7}{16}$ " O.S.B. SHEATHING ONE SIDE W/8d NAILS @ 3" o.c.	(2) 2x4	½"ø ROD	SHRINKAGE TAKE-UP DEVICE AND BEARING PLATE BY SUPPLIER		
3rd to 4th	$\frac{7}{16}$ " O.S.B. SHEATHING ONE SIDE W/8d NAILS @ 3" o.c.	(2) 2x4	½"ø ROD	SHRINKAGE TAKE-UP DEVICE AND BEARING PLATE BY SUPPLIER		
2nd to 3rd	$\frac{7}{16}$ " O.S.B. SHEATHING ONE SIDE W/8d NAILS @ 3" o.c.	(3) 2x4	⁵%"ø ROD	SHRINKAGE TAKE-UP DEVICE AND BEARING PLATE BY SUPPLIER		
SLAB to 2nd	$\frac{7}{16}$ " O.S.B. SHEATHING ONE SIDE W/8d NAILS @ 3" o.c.	(4) 2x4	¾"ø ROD	EPOXY RODS INTO SLAB – SEE TABLE FOR REQUIRED EMBEDMENT		

	SHEAR WALL SW-3 (4 STORY)					
	SHEAR WALL	IS COMPRISED OF O.S.B SHEATHING	WITH BLOCKING - SEE	DETAILS SHEET S5.01		
FLOOR	SHEATHING REQUIREMENTS / NAIL SPACING	TOTAL # OF COMPRESSION STUDS EACH SIDE OF HOLDOWN ROD	MINIMUM THREADED ROD Ø	FLOOR CONNECTION		
4th to Roof	$15_{32}$ " O.S.B. SHEATHING ONE SIDE W/8d NAILS @ 3" o.c.	(2) 2x6	½"ø ROD	SHRINKAGE TAKE-UP DEVICE AND BEARING PLATE BY SUPPLIER		
3rd to 4th	$15_{32}$ " O.S.B. SHEATHING ONE SIDE W/8d NAILS @ 3" o.c.	(2) 2x6	%"ø ROD	SHRINKAGE TAKE-UP DEVICE AND BEARING PLATE BY SUPPLIER		
2nd to 3rd	$15_{32}$ " O.S.B. SHEATHING ONE SIDE W/8d NAILS @ 3" o.c.	(2) 2x6	(2)5⁄%"ø ROD	SHRINKAGE TAKE-UP DEVICE AND BEARING PLATE BY SUPPLIER		
SLAB to 2nd	15/32" O.S.B. SHEATHING ONE SIDE W/10d NAILS @ 3" o.c.	(2) 2x6	(2)¾"ø ROD	EPOXY RODS INTO SLAB – SEE TABLE FOR REQUIRED EMBEDMENT		

	SHEAR WALL SW-4 (4 STORY)			
	SHEAR WALL IS COMPRISED OF O.S.B SHEATHING WITH BLOCKING - SEE DETAILS SHEET S5.01			
FLOOR	SHEATHING REQUIREMENTS / NAIL SPACING	TOTAL # OF COMPRESSION STUDS EACH SIDE OF HOLDOWN ROD	MINIMUM THREADED ROD Ø	FLOOR CONNECTION
4th to Roof	$\frac{7}{6}$ " O.S.B. SHEATHING ONE SIDE W/8d NAILS @ 3" o.c.	(2) 2x4	½"ø ROD	SHRINKAGE TAKE-UP DEVICE AND BEARING PLATE BY SUPPLIER
3rd to 4th	${\cal H}_6$ " O.S.B. SHEATHING ONE SIDE W/8d NAILS @ 3" o.c.	(3) 2x4	¾"ø ROD	SHRINKAGE TAKE-UP DEVICE AND BEARING PLATE BY SUPPLIER
2nd to 3rd	<sup>15</sup> / <sub>32</sub> " O.S.B. SHEATHING ONE SIDE W/10d NAILS @ 3" o.c.	(3) 2x4	(2)¾"ø ROD	SHRINKAGE TAKE-UP DEVICE AND BEARING PLATE BY SUPPLIER
SLAB to 2nd	${\cal H}_6$ " O.S.B. SHEATHING ONE SIDE W/8d NAILS @ 3" o.c.	(5) 2x4	(2)%% ROD	EPOXY RODS INTO SLAB – SEE TABLE FOR REQUIRED EMBEDMENT

	SHEAR WALL SW-5 (4 STORY)			
	SHEAR WALL	IS COMPRISED OF O.S.B SHEATHING V	WITH BLOCKING - SEE	DETAILS SHEET S5.01
FLOOR	Sheathing requirements / Nail spacing	TOTAL # OF COMPRESSION STUDS EACH SIDE OF HOLDOWN ROD	MINIMUM THREADED ROD Ø	FLOOR CONNECTION
4th to Roof	$15/_{32}$ " O.S.B. SHEATHING ONE SIDE W/8d NAILS @ 3" o.c.	(2) 2x6	½"ø ROD	SHRINKAGE TAKE-UP DEVICE AND BEARING PLATE BY SUPPLIER
3rd to 4th	$^{15}$ 0.S.B. SHEATHING ONE SIDE W/8d NAILS @ 3" o.c.	(2) 2x6	¾"ø ROD	SHRINKAGE TAKE-UP DEVICE AND BEARING PLATE BY SUPPLIER
2nd to 3rd	<sup>15</sup> / <sub>32</sub> " O.S.B. SHEATHING ONE SIDE W/10d NAILS @ 3" o.c.	(2) 2x6	(2)¾"ø ROD	SHRINKAGE TAKE-UP DEVICE AND BEARING PLATE BY SUPPLIER
SLAB to 2nd	<sup>15</sup> / <sub>32</sub> " O.S.B. SHEATHING BOTH SIDE W/8d NAILS @ 3" o.c.	(2) 2x6	(2)% <b>"</b> ø ROD	EPOXY RODS INTO SLAB – SEE TABLE FOR REQUIRED EMBEDMENT

### AILS SHEET S5.01

FLOOR CONNECTION	
IRINKAGE TAKE-UP DEVICE AND BEARING PLATE BY SUPPLIER	
IRINKAGE TAKE-UP DEVICE AND BEARING PLATE BY SUPPLIER	
IRINKAGE TAKE-UP DEVICE AND BEARING PLATE BY SUPPLIER	
POXY RODS INTO SLAB – SEE BLE FOR REQUIRED EMBEDMENT	

AILS SHEET S5.01 FLOOR CONNECTION RINKAGE TAKE-UP DEVICE AND BEARING PLATE BY SUPPLIER RINKAGE TAKE-UP DEVICE AND BEARING PLATE BY SUPPLIER RINKAGE TAKE-UP DEVICE AND

SHEAR WALL SW-6 (1 STORY)			
	SHEAR WALL IS COMPRISED OF O.S.B SHEATHING WITH BLOCKING - SEE DETAILS SHEET S		
FLOOR	SHEATHING REQUIREMENTS / NAIL SPACING	TOTAL # OF COMPRESSION STUDS EACH SIDE OF HOLDOWN ROD	FLOOR CONNECTION
SLAB to ROOF	$7_{6}$ " O.S.B. SHEATHING ONE SIDE W/8d NAILS @ 3" o.c.	(3) 2x6 OR (3) 2x4	SIMPSON HDU8-SDS2.5 W/ 7/8"ø ROOD
		., .,	W/ 7/8"ø ROOD

SHEAR WALL SW–7 (1 STORY)			
	SHEAR WALL IS COMPRISED OF 0.S.B SHEATHING WITH BLOCKING - SEE DETAILS SHEET S5.01		
FLOOR	Sheathing requirements / Nail spacing	TOTAL # OF COMPRESSION STUDS EACH SIDE OF HOLDOWN ROD	FLOOR CONNECTION
SLAB to ROOF	$7_{6}$ " O.S.B. SHEATHING ONE SIDE W/8d NAILS @ 3" o.c.	(2) 2x6	SIMPSON HDU5-SDS2.5 W/ 5/8"Ø ROOD

USE 15/32" SHEATHING @ EXTERIOR WALLS

### SHEAR WALL NOTES:

- 1. FOR GENERAL FRAMING INFORMATION, SEE SHEET S1.01
- 2. SEE DETAILS ON S5.01 FOR TYPICAL SHEAR WALL DETAILS.
- 3. SHEAR WALL SHEATHING SHALL BE  $\frac{7}{16}$  OR  $\frac{15}{32}$  OSB OR PLYWOOD WITH 8d OR 10d NAILS @ 3"o.c. AT EDGES AND 12"o.c. IN FIELD. - SEE SCHEDULES THIS SHEET
- 4. PROVIDE SIMPSON HDU5-SDS2.5 WITH %" ROD AND (2) STUDS AROUND DOOR OPENINGS AT THE BOTTOM FLOOR.
- 5. ALL RODS SHALL BE ASTM A36.
- 6. ALL STUDS SHALL MATCH LOAD BEARING WALL SPACING AND SPECIES.
- 7. PSL = 1.8E PARALLAM PSL BY WEYERHAEUSER.
- 8. ALL ROD HOLDOWNS TO BE LOCATED A MAXIMUM DISTANCE OF 12" FROM END OF WALL. (TYP. OF ALL SHEAR WALLS)
- 9. EPOXY SHALL BE HILTI HY-200. SEE TABLE THIS SHEET FOR REQUIRED EMBEDMENT.
- 10. ALL HOLDOWN RODS LOCATED ON EXTERIOR WALL SHALL BE CAST-IN-PLACE.

SHEAR WALL ROD EPOXY EMBEDMENT			
ROD SIZE	EMBEDMENT		
½"ø	7"		
5%"ø	9"		
3⁄4"ø	10"		
7⁄8"ø	14"		
- FPOXY SHALL F	RE HILTE HY_200		

- EPOXY SHALL BE HILTI HY-200 - ALL EXTERIOR HOLDOWN RODS SHALL BE CAST-IN-PLACE AS SHOWN ON SECTION 9-9 SHEET S-301

#### SHRINKAGE TAKE-UP DEVICE -----@ EACH FLOOR AND @ TOP OF WALL, BY SUPPLIER

BLOCK ALL EDGES & FASTEN ALL SHEAR WALL PANELS (OSB OR . PLYWOOD) @ 12" o/c IN FIELD. -SEE SCHEDULE FOR NAIL SIZES AND SPACING @ EDGES

> JOIST SEE SCHEDULE -AND PLAN FOR SIZES

SHRINKAGE TAKE-UP DEVICE @ EACH FLOOR AND @ TOP OF WALL, BY SUPPLIER

SHEAR WALL BUNDLED STUDS SHOWN IN SW SCHEDULE. STUDS SHOWN IN \_ SCHEDULE TO BE PLACED ON EACH SIDE OF HOLD DOWN ROD. STUDS SHALL MATCH LOAD BEARING WALL SCHEDULE PER PLAN U.N.O. ON SHEAR WALL SCHEDULE

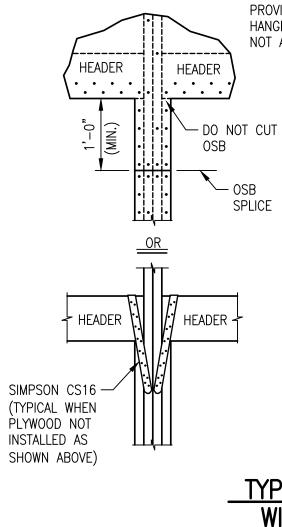
> SHRINKAGE TAKE-UP -DEVICE @ EACH FLOOR AND @ TOP OF WALL, BY SUPPLIER

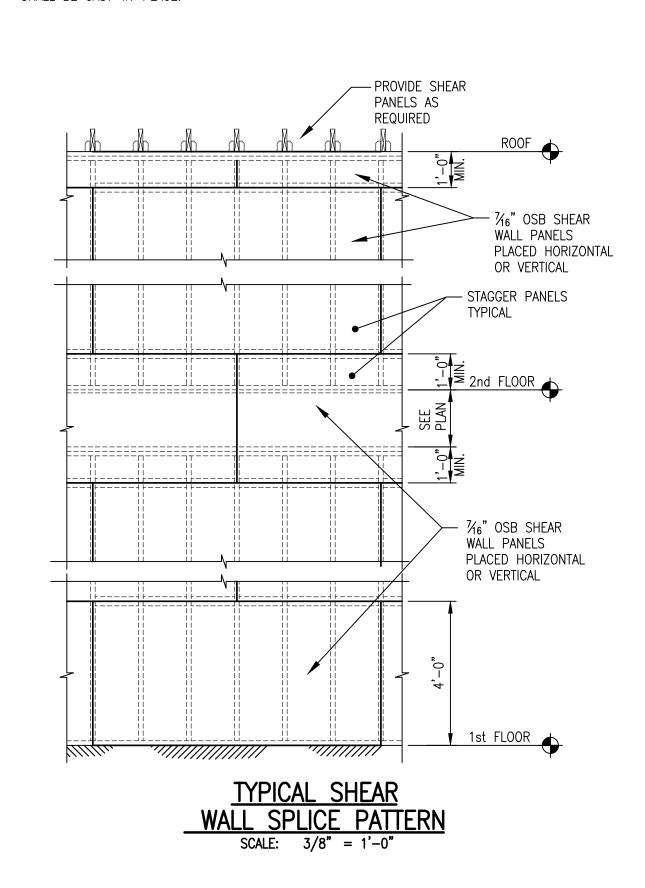
6" CLEAR SPACING BETWEEN -BUNDLED STUDS FOR ROD HOLDOWN AT ALL LOCATIONS.

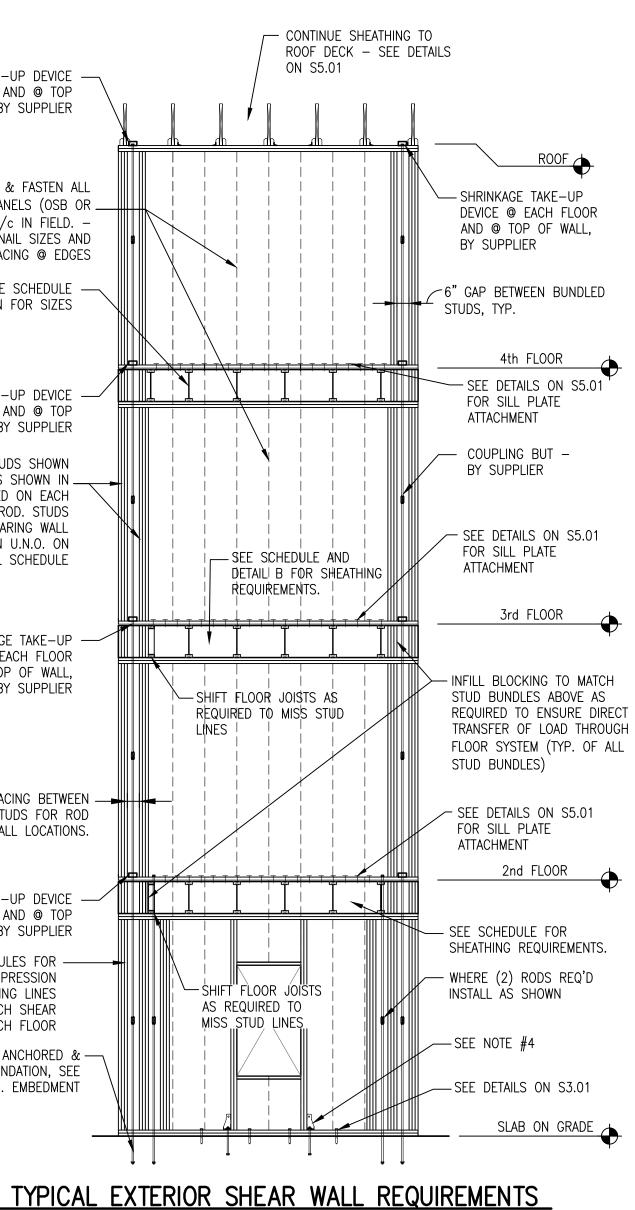
SHRINKAGE TAKE-UP DEVICE -----@ EACH FLOOR AND @ TOP OF WALL, BY SUPPLIER

SEE SCHEDULES FOR -NUMBER OF COMPRESSION STUDS AND BEARING LINES REQUIRED FOR EACH SHEAR WALL @ EACH FLOOR

ROD HOLDOWN ANCHORED & ----EPOXIED INTO FOUNDATION, SEE SCHEDULE FOR MIN. EMBEDMENT







SCALE: 1/4" = 1'-0"



PROVIDE SIMPSON HH6 HEADER -HANGER WHERE JAMB WIDTHS WILL (2) STUDS MINIMUM, CONTINUOUS FROM NOT ALLOW FOR JACK STUDS SILL PLATE TO DOUBLE TOP PLATE HEADER HEADER. HEADER . .HEADER - JACK STUD (TYPICAL) UNLESS WIDTH NOT SEE ENLARGED DETAIL AVAILABLE AT LEFT NOTE: CONTRACTOR TYPICAL FRAMING AT EXTERIOR SHALL USE JACK STUDS WHEN POSSIBLE. WINDOW OR DOOR OPENING SCALE: 3/8" = 1'-0"

DATE: 11/15/2021 (CD) 002221-21-3332 SHEET TITLE: Wood Shear Wall Typical Details

S5.02