STRUCTURAL NOTES

1. GENERAL

A. THE BUILDING IS DESIGNED UNDER THE PROVISIONS OF THE 2015 INTERNATIONAL BUILDING CODE AND ASCE 7-10.

B. THE FOLLOWING LOADS WERE USED IN THE DESIGN:

BUILDING RISK CATEGORY	II
FLOOR LIVE LOAD	
RESIDENTIAL UNITS	40 PSF
UNIT INTERNAL STAIRS	40 PSF / 300 LB PT LOAD
EXTERNAL EGRESS STAIRS	100 PSF / 300 LB PT LOAD
PRIVATE BALCONIES	60 PSF
ELEVATED CORRIDORS SERVING RESIDENTIAL UNITS	40 PSF
PUBLIC AREAS	100 PSF
NOTE: LIVE LOAD REDUCTION IS UTILIZED AS ALLOWED BY CODE	
ROOF LIVE LOAD	
MINIMUM UNIFORM ROOF LIVE LOAD	20 PSF / 300 PT LOAD
ROOF SNOW LOAD	
GROUND SNOW LOAD (Pg)	10 PSF
FLAT-ROOF SNOW LOAD (Pf)	7 PSF
RAIN-ON-SNOW SURCHARGE	5 PSF
SNOW EXPOSURE FACTOR (Ce)	1.0
SNOW LOAD IMPORTANCE FACTOR (Is)	1.0
THERMAL FACTOR (Ct)	1.0
WIND LOAD	
WIND SPEED (3-SECOND GUST)	
Vult	115 MPH
Vasd: (0.77*VULT)	90 MPH
WIND EXPOSURE	B
INTERNAL PRESSURE COEFFICIENT	+0.18, -0.18
COMPONENTS AND CLADDING WIND LOAD	

EFFECTIVE WIND AREA	ZONE	PRESSURE
10 FT.^2	4	+24.4 PSF
10 FT.^2	4	-26.4 PSF
10 FT.^2	5	+24.4 PSF
10 FT.^2	5	-32.5 PSF

EARTHQUAKE DESIGN	
SEISMIC IMPORTANCE FACTOR (Ie)	1.00
SEISMIC DESIGN CATEGORY	C
SITE CLASSIFICATION	D
SEISMIC RESPONSE COEFFICIENTS	
Ss	0.258
S1	0.120
Sds	0.274
Sd1	0.186
DESIGN BASE SHEAR	43 KIPS
SEISMIC-FORCE RESISTING SYSTEM PER	
ASCE 7-10 TABLE 12.2-1	TYPE A15
R	6.5
Cd	4
Cs	0.042
TL	12
ANALYSIS METHOD	ELFP
HANDRAIL AND GUARD LOADS	
HANDRAIL AND GUARD	50 PLF / 200 LB PT LOAD
INTERMEDIATE RAIL	50 LB PT LOAD

C. SEE ARCHITECTURAL DRAWINGS FOR ANGLES, CLIPS, PLATES, ETC., AND OTHER MISCELLANEOUS ITEMS. VERIFY AND COORDINATE ALL FRAMES, OPENINGS, ETC. WITH THE MECHANICAL AND ELECTRICAL CONTRACTORS.

D. SUBMIT SHOP DRAWINGS FOR THE FOLLOWING ITEMS. SUBMITTALS INCLUDE BUT MAY NOT BE LIMITED TO:

--CONCRETE MIX DESIGN
--REINFORCING STEEL
--STRUCTURAL STEEL
--PRE-ENGINEERED BUILDING COMPONENTS
--WOOD TRUSSES
--STEEL STAIRS

DO NOT USE CONTRACT DRAWINGS AS A BASE FOR SHOPS. REVIEW IS LIMITED TO DESIGN CONFORMANCE. CONTRACTOR IS RESPONSIBLE FOR DIMENSIONS.

E. CONTRACTOR SHALL COORDINATE WITH THE QUALIFIED AGENCY RETAINED BY THE OWNER TO PERFORM INSPECTION AND TESTING. INSPECTIONS REQUIRED INCLUDE, BUT MAY NOT BE LIMITED TO:

--SOILS AND FOUNDATIONS
--CONCRETE
--STRUCTURAL STEEL
--MASONRY

2. EARTHWORK

A. FOUNDATIONS ARE DESIGNED TO BEAR ON ENGINEERED FILL OR NATURAL SOIL WITH A CAPACITY OF 2,000 PSF, BASED ON RECOMMENDATIONS IN THE GEOTECHNICAL REPORT PREPARED BY OMI, INC. DATED 12/31/2020. THIS VALUE IS TO BE VERIFIED IN THE FIELD BY THE BUILDING INSPECTOR OR A QUALIFIED TESTING AGENCY.

B. BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 2 FOOT-0 INCH BELOW FINISH EXTERIOR GRADE. WHERE REQUIRED, STEP FOOTINGS IN RATIO OF 2 HORIZONTAL TO 1 VERTICAL.

C. COMPACTED BACKFILL BELOW BUILDING SLABS AND FOOTINGS: ALL SOIL FILL MATERIAL MUST BE APPROVED BY SOILS ENGINEER PRIOR TO PLACEMENT. PROOFROLL SUBGRADE REMOVING AND REPLACING SOFT OR COMPRESSIVE MATERIALS. FILL MATERIAL SHALL BE PLACED IN LAYERS NOT TO EXCEED 8 INCHES AND COMPACTED TO MIN. 95 PERCENT OF THE DRY MAXIMUM DENSITY AS DETERMINED BY ASTM D698.

D. AT ROCKY AREAS AROUND AND BELOW EL 1055 FT MSL: ROCK LEDGES, PINNACLES OR BOULDERS, IF ENCOUNTERED AND CONFLICTING WITH THE PROPOSED FOUNDATION SYSTEM, SHALL BE REMOVED BY BLASTING, RIPPING OR HOE RAMMING PER THE GEOTECHNICAL REPORT. ONCE THE AREA HAS BEEN EXCAVATED, A MINIMUM 1 FOOT LAYER OF #2 STONE SHALL BE PLACED ACROSS THE AREA. THE #2 STONE SHALL BE CAPPED WITH A 6" LAYER OF "CRUSHER RUN" (1.5" TO DUST, STONE). SUBSEQUENT LAYERS OF ENGINEERED FILL SHALL THEN BE PLACED TO BUILDING PAD ELEVATIONS, BELOW THE 4" #57 UNDERSLAB BASE.

E. AT SOIL AREAS AROUND AND ABOVE EL 1062 FT MSL: AREAS APPROXIMATLEY AT BUILDING PAD EL AND AREAS THAT WILL RECEIVE ENGINEERED FILL SHALL BE OBSERVED BY THE GER PRIOR TO PLACING THE 1 FOOT LAYER OF #2 STONE AND 6" CAP OF CRUSHER RUN. SUBSEQUENT LAYERS OF ENGINEERED FILL SHALL THEN BE PLACED TO BUILDING PAD ELEVATIONS, BELOW THE 4" #57 UNDERSLAB BASE.

3. CONCRETE

A. CONCRETE CONSTRUCTION SHALL BE PER THE APPLICABLE BUILDING CODE, ACI 318 AND ACI 301, LATEST EDITIONS.

B. CONCRETE SHALL ATTAIN THE FOLLOWING 28 DAY COMPRESSIVE STRENGTHS PER ASTM A39.

--FOOTINGS, PIERS	3,000 PSI
--SLAB-ON-GRADE	3,500 PSI
--RETAINING WALLS	4,000 PSI

C. VERIFY CONCRETE STRENGTHS WITH A MINIMUM OF ONE SET OF NINE 4X8-INCH COMPRESSION CYLINDERS, (3 @ 7 DAYS, 3 @ 28, 3 SPARE).

D. EXTERIOR CONCRETE SHALL BE AIR-ENTRAINED TO PROVIDE AN AIR CONTENT OF 6+/-1.5 PERCENT BY VOLUME.

E. PROVIDE CLEAR DISTANCE TO OUTERMOST REINFORCING AS FOLLOWS:

CONCRETE CAST AGAINST EARTH	3 INCHES
CONCRETE EXPOSED TO EARTH OR WEATHER:	
#5 OR SMALLER	1-1/2 INCHES
#6 OR LARGER	2 INCHES

F. NON-SHRINK GROUT FOR COLUMNS BASE PLATES SHALL ATTAIN A 28 DAY COMPRESSIVE STRENGTH: F'c = 5,000 PSI.

G. REINFORCING STEEL SHALL CONFORM TO A615-GR60; MESH SHALL CONFORM TO ASTM A185 WITH MINIMUM LAPS OF 8 INCHES. PLACING PLANS AND SHOP FABRICATION DETAILS SHALL BE IN ACCORDANCE WITH "THE MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES". FURNISH SUPPORT BARS AND A CCESSORIES IN ACCORDANCE WITH C.R.S.I. STANDARDS.

H. PROVIDE CORNER BARS TO MATCH HORIZONTAL REINFORCING IN WALLS AND FOOTINGS. SPLICE LAPS SHALL BE A MINIMUM OF 36 BAR DIAMETERS, UNLESS NOTED OTHERWISE. PROVIDE DOWELS BETWEEN FOOTINGS AND WALLS OR PIERS TO MATCH SIZE AND SPACING OF VERTICAL REINFORCING.

I. WALLS WITH LATERAL EARTH PRESSURES SHALL BE ADEQUATELY SHORED OR FLOOR/ROOF CONSTRUCTION SHALL BE IN PLACE AND SECURED PRIOR TO BACKFILLING.

J. INSTALLATION OF ELECTRICAL CONDUIT WITHIN THE CONCRETE SLAB-ON-GRADE IS PROHIBITED.

4. MASONRY

A. MASONRY CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE APPLICABLE BUILDING CODE AND THE "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" TMS 402/ACI-530/ASCE 5 AND THE "SPECIFICATIONS FOR MASONRY STRUCTURES" TMS 602/ACI-530.1/ASCE 6, LATEST EDITIONS.

B. MASONRY TO CONFORM TO THE FOLLOWING SPECIFICATIONS:

HOLLOW LOAD-BEARING C.M.U	ASTM C90
CONCRETE BUILDING BRICK	ASTM C55, GRADE A
MORTAR	ASTM C270, TYPE M OR S
GROUT	ASTM C476

C. MASONRY ASSEMBLIES SHALL HAVE COMPRESSIVE STRENGTH (F'M) GREATER THAN OR EQUAL TO 2000 PSI.

D. WALLS SHALL BE CONSTRUCTED USING A FULL BED OF MORTAR. VERTICAL REINFORCING SHALL BE GROUTED IN PLACE WITH 2500 PSI GROUT (GROUT SLUMP SHALL FALL BETWEEN 8 AND 11 INCHES) POUR HEIGHT AND LIFT HEIGHT SHALL NOT EXCEED 5 FEET - 0 INCHES.

E. PROVIDE CONTINUOUS HORIZONTAL JOINT REINFORCING IN MASONRY WALLS AT 16 INCHES O.C. PROVIDE AT 8 INCHES O.C. AT PARAPETS.

F. CAVITY WALLS OF BRICK AND BLOCK SHALL BE CONSTRUCTED WITH JOINT REINFORCING IN MASONRY AND ADJUSTABLE METAL ANCHORS TO BRICK.

G. UNLESS NOTED OTHERWISE, PROVIDE 16 INCH LONG BY 24 INCHES HIGH SOLID OR GROUTED BLOCK UNDER BEARING ENDS OF BEAMS.

H. PROVIDE 48 INCH REINFORCEMENT LAP AT CONTINUOUS BOND BEAM STEPS.

I. COMPOSITE WALLS SHALL HAVE THE COLLAR JOINT BETWEEN BRICK AND BLOCK GROUTED SOLID AND THE WALLS SHALL BE BUILT WITH BOTH WYTHES SIMULTANEOUSLY.

J. MASONRY WALLS SHALL HAVE CONTROL JOINTS AT 30 FEET ON CENTER UNLESS NOTED OTHERWISE.

K. REINFORCING STEEL SHALL CONFORM TO ASTM A615-GR60. LAP BARS A MINIMUM OF 48 BAR DIAMETERS. GROUT ALL REINFORCED CORES SOLID.

L. UNLESS SHOWN ON PLAN, LINTELS FOR MASONRY WALLS SHALL BE AS FOLLOWS:

OPENINGS TO 3 FT, 0 IN	3-1/2 X 3-1/2 X 1/4
3 FT, 1 IN TO 5 FT, 0 IN	4 X 3-1/2 X 5/16 - 3-1/2 HORIZONTAL
5 FT, 1 IN TO 6 FT, 6 IN	5 X 3-1/2 X 5/16 - 3-1/2 HORIZONTAL
OVER 6 FT, 6 IN	CONSULT ARCHITECT/ENGINEER

PROVIDE 1 ANGLE FOR EACH 4 INCHES OF WALL THICKNESS. LINTELS SHALL BEAR 6 INCHES MINIMUM EACH END U.N.O.

M. PROVIDE TWO-PIECE ADJUSTABLE ANCHORS TO MASONRY AT A MAXIMUM SPACING OF 24 INCHES O.C. AT ALL VERTICAL AND HORIZONTAL STRUCTURAL STEEL MEMBERS.

N. CAVITY WALLS OF BRICK WITH STUD BACKUP SHALL BE CONSTRUCTED WITH TWO-PIECE ADJUSTABLE METAL ANCHORS AT A MAXIMUM SPACING OF 16 INCHES O.C. HORIZONTAL (INTO STUDS) AND 24 INCHES O.C. VERICAL. AT BRICK WALLS OVER 3O FEET HIGH, PROVIDE ANCHORS AT 16 INCHES O.C. HORIZONTAL AND VERTICAL.

O. ALL NON-LOADBEARING MASONRY WALLS SHALL BE PROVIDED WITH VERTICAL SLIP CONNECTIONS AT THE TOP OF THE WALL, U.N.O.

5. STEEL

A. STEEL CONSTRUCTION SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE APPLICABLE BUILDING CODE AND SHALL CONFORM TO AISC 360. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING ASTM STANDARDS:

WIDE FLANGE SHAPES	A992 – GR50
STEEL PLATES, CHANNELS AND ANGLES	A36
STRUCTURAL PIPES	A53 – GR B
STRUCTURAL RECTIROUND (HSS)	A500 - GR B
ANCHOR RODS (3/4" DIAM. OR LESS)	F1554 – 36 KSI
ANCHOR RODS (7/8" DIAM. OR GREATER)	F1554 – 55 KSI
HIGH-STRENGTH BOLTS	A325
HEADED STUDS	A108

B. BOLTED CONNECTIONS TO USE A325-TYPE N, HIGH STRENGTH BOLTS IN BEARING TYPE CONNECTIONS TIGHTENED TO A SNUG TIGHT CONDITION IN ACCORDANCE WITH RCSC SPECIFICATIONS.

C. SHOP CONNECTIONS TO BE WELDED OR BOLTED. FIELD CONNECTIONS TO BE BOLTED UNLESS OTHERWISE SHOWN. BOLT HOLES TO BE STANDARD ROUND HOLES (d+1/16 INCHES) UNLESS OTHERWISE NOTED. SHORT SLOTS SHALL BE PERMITTED NORMAL TO THE LOAD DIRECTION IN SLIP CRITICAL AND BEARING TYPE CONNECTIONS AS PER AISC REQUIREMENTS.

D. IF BEAM REACTIONS ARE DENOTED ON THE DRAWINGS, BEAM-WEB CONNECTIONS FOR SHEAR AT EACH END SHALL BE DETAILED TO SUPPORT THE LOADS SHOWN OR PROVIDE THE FOLLOWING MINIMUM NUMBER OF BOLTS, WHICHEVER IS GREATER. STIFFENED SEATS SHALL BE DETAILED TO SUPPORT THE LOADS SHOWN ON THE DRAWINGS OR THE MINIMUM FACTORED LOADS INDICATED BELOW, WHICHEVER IS GREATER.

	BEAM/WEB	STIFF. SEAT
W8 OR W10	2 BOLTS	2OK
W12 OR W14	3 BOLTS	3OK
W16 OR W18	4 BOLTS	4OK
W21 OR W24	5 BOLTS	6OK

E. STRUCTURAL STEEL SHALL BE GIVEN ONE SHOP COAT OF APPROVED SHOP PRIMER APPLIED TO CLEAN AND DRY SURFACES. DO NOT PAINT STEEL THAT WILL BE FIREPROOFED OR EMBEDDED IN CONCRETE.

F. STEEL BEAMS SHALL BE WELDED TO STEEL BEARING PLATES WITH 3 INCH LONG BY 1/4-INCH FILLET WELD EACH SIDE OF FLANGE (MINIMUM).

G. WELDING OF STRUCTURAL STEEL SHALL BE WITH E70XX ELECTRODES.

H. STEEL FLITCH BEAMS SHALL BE CONNECTED WITH 1/2" DIAMETER THROUGH BOLTS @ 16" O.C. WITH THE FIRST ROW OF BOLTS 6" FROM EACH END. SEE SCHEDULE ON S1.401.

6. WOOD FRAMING (CONVENTIONAL/TYPE V)

A. FRAMING LUMBER FOR STUDS, HEADERS AND JOISTS SHALL BE HEM FIR #2, SPRUCE-PINE-FIR (SPF) #2, OR BETTER, WITH A MAXIMUM MOISTURE CONTENT OF 19-PERCENT, HAVING THE FOLLOWING MINIMUM PROPERTIES (BASED ON 2X12 MEMBERS):

BENDING STRESS "Fb"	= 850 PSI FOR SINGLE MEMBER USE
HORIZONTAL SHEAR "Fv"	= 135 PSI
COMPRESSION PERPENDICULAR TO GRAIN "Fc"	= 405 PSI
COMPRESSION PARALLEL TO GRAIN "Fc11"	= 1150 PSI
MODULUS OF ELASTICITY "E"	= 1,300,000 PSI

NOTE: SPF (SOUTH) IS NOT ACCEPTABLE.

B. ALL EXPOSED EXTERIOR FRAMING AND FRAMING IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE-TREATED (PT). FRAMING SHALL BE PRESSURE-TREATED WITH ALAKALINE COPPER QUAT (ACQ) OR COPPER AZOLE (CBA-A AND CA-B), NOT SODIUM BORATE (SBX). PT LUMBER SHALL NOT BE INCISED.

C. STRUCTURAL POSTS AND TREATED LUMBER (PT) SHALL BE SOUTHERN PINE (SP) #2 OR BETTER, HAVING THE FOLLOWING MINIMUM PROPERTIES (BASED ON 2X12 MEMBERS):

STRENGTH	
BENDING STRESS "Fb"	= 750 PSI SINGLE MEMBER USE
HORIZONTAL SHEAR "Fv"	= 175 PSI
COMPRESSION PERPENDICULAR TO GRAIN "Fc"	= 565 PSI
COMPRESSION PARALLEL TO GRAIN "Fc11"	= 1,250 PSI
MODULUS OF ELASTICITY "E"	= 1,400,000 PSI

D. LAMINATED VENEER LUMBER (LVL OR MICROLAM) BEAMS SHALL CONFORM TO ASTM D 5456 AND SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES (BASED ON 1-3/4 X 11-7/8 MEMBERS):

BENDING STRESS "Fb"	= 2600 PSI
HORIZONTAL SHEAR "Fv"	= 285 PSI
MODULUS OF ELASTICITY "E"	= 2,000,000 PSI
BEARING STRESS "FPERP"	= 750 PSI
EQUIV SPECIFIC GRAVITY FOR CONNECTION DESIGN	= 0.50

E. PARALLEL STRAND LUMBER (PSL) COLUMNS SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES:

BENDING STRESS "Fb"	= 2,400 PSI
HORIZONTAL SHEAR "Fv"	= 190 PSI
COMPRESSION PARALLEL TO GRAIN "Fc11"	= 2,500 PSI
MODULUS OF ELASTICITY "E"	= 1,800,000 PSI

F. AT EXTERIOR WALLS, PROVIDE SOLID BLOCKING AT 4 FEET ON CENTER BETWEEN BAND JOIST AND FIRST INTERIOR PARALLEL JOIST.

G. PREFABRICATED JOIST HANGERS, BEAM HANGERS, POST CAPS, AND POST BASES SHALL BE SIZED AND ATTACHED PER MANUFACTURER'S RECOMMENDATIONS TO ACHIEVE AT LEAST THE MINIMUM MANUFACTURER LISTED CAPACITIES, UNO ON THE DRAWINGS. FASTENERS AND CONNECTORS UTILIZED WITH TREATED LUMBER (PT OR FRT) SHALL MEET G185 HOT-DIPPED GALVANIZING.

H. ANCHOR BOLTS CONNECTING PRESSURE-TREATED WOOD PLATES TO FOUNDATIONS, MASONRY WALLS, OR CONCRETE SLABS SHALL BE HOT-DIPPED GALVANIZED.

I. BUILT-UP STUD COLUMNS SHALL HAVE ONE JACK STUD AND THE REMAINING STUDS SHALL BE KING STUDS. MULTIPLE STUDS SHALL BE NAILED WITH 10D NAILS AT 8 INCHES O.C. PROVIDE SOLID BLOCKING OR CRIPPLE STUDS IN FLOOR SYSTEM AT ALL POINT LOADS ABOVE.

J. FREESTANDING POSTS SHALL HAVE PREFAB POSTCAP AND BASE. POSTS WITHIN WALL NEED ONLY HAVE PREFAB CAP ATTACHED TO BEAM, UNO. POSTS WITHIN WALL BEARING ON MASONRY OR CONCRETE SHALL HAVE PREFAB BASE, UNO.

K. STANDARD MEMBER CONNECTIONS SHALL BE PER FASTENING SCHEDULE IN SECTION 23 OF THE INTERNATIONAL BUILDING CODE (IBC), UNO.

L. STUD BEARING WALLS TO BE PROVIDED WITH 2 CONTINUOUS TOP PLATES AND 1 CONTINUOUS BOTTOM PLATE WITH A MINIMUM OF ONE ROW OF HORIZONTAL BRIDGING AT MID-HEIGHT OF WALL UNLESS NOTED OTHERWISE. SPLICES OF TOP PLATES SHALL OCCUR OVER STUD AND SHALL BE STAGGERED A MINIMUM OF FOUR FEET.

M. NAILS FOR FRAMING AND SHEATHING CONNECTIONS SPECIFIED IN THE DRAWINGS AND ASSOCIATED NOTES SHALL CONFORM TO ASTM F1667 AND SHALL MEET THE FOLLOWING MINIMUM SIZE REQUIREMENTS:

TYPE	DIAMETER x LENGTH
8d	0.131" x 2-1/2"
10d	0.148" x 3"
12d	0.148" x 3-1/4"
16d	0.162" x 3-1/2"
20d	0.192" x 4"

SHANK DIAMETER	MINIMUM STRENGTH
0.099" TO 0.142"	100 KSI
0.143" TO 0.177"	90 KSI
0.178" TO 0.254"	80 KSI

NOTE: NAILS USED IN STANDARD CONNECTIONS SHALL BE SIZED PER THE REQUIREMENTS OF THE BUILDING CODE.

N. ROOF MEMBERS SHALL BE CONNECTED AT EACH BEARING POINT WITH ONE PREFABRICATED GALVANIZED METAL ANCHOR. ANCHORS SHALL BE 18 GAGE MINIMUM AND SHALL BE ATTACHED TO HAVE A CAPACITY TO RESIST A 450# UPLIFT LOADING, UNLESS SHOWN OTHERWISE ON DRAWINGS.

P. THE MINIMUM DEPTH AND MAXIMUM SPACING OF WOOD TRUSSES IS SHOWN ON DRAWINGS. THE SUPPLIER SHALL ADJUST SPACING AS REQUIRED TO MEET THE LOADINGS DESIGNATED ON THE DRAWING.

Q. PROVIDE LSL BAND BOARD IN WOOD TRUSS SYSTEMS AT ALL PERIMETER BEARING WALLS. ALTERNATIVELY, PROVIDE 2-3/4 INCH PLYWOOD BANDS GLUED AND SCREWED TOGETHER. PROVIDE SQUASH BLOCKS AND STIFFENERS AS REQUIRED TO DISTRIBUTE LOADINGS AND AS REQUIRED BY MANUFACTURER. PROVIDE SOLID BLOCKING AT INTERIOR JOIST SUPPORTS WITH BEARING WALLS ABOVE.

R. DO NOT SPLICE STRUCTURAL MEMBERS BETWEEN SUPPORTS.

S. PREFABRICATED TRUSSES SHALL BE DESIGNED FOR THE LOADS SCHEDULED ON THE DRAWINGS. SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR REVIEW. THE DESIGN OF THE BRACING REQUIRED TO LATERALLY STABILIZE THE TRUSSES AND TRUSS MEMBERS SHALL BE THE RESPONSIBILITY OF THE SPECIALTY TRUSS ENGINEER. AFFIX SEAL OF ENGINEER REGISTERED IN THE STATE OF THE PROPOSED PROJECT. TEMPORARY BRACING DURING ERECTION IS THE RESPONSIBILITY OF THE CONTRACTOR.

7. SHEATHING

A. FLOOR SHEATHING SHALL BE 23/32 (3/4) INCH APA RATED STURD-I (COMBINATION SUBFLOOR-UNDERLAYMENT) WOOD STRUCTURAL PANEL, TONGUE AND GROOVE, WITH SPAN RATING OF 48/24. PANELS SHALL HAVE LONG DIMENSION ORIENTED ACROSS THREE OR MORE JOISTS AND SHALL BE FASTENED WITH CONSTRUCTION ADHESIVE AND NAILS AT PANEL EDGES AND INTERMEDIATE SUPPORTS AS SCHEDULED ON THE DRAWINGS. UNLESS NOTED OTHERWISE, PANEL EDGES NEED NOT BE BLOCKED. INSTALL PER MANUFACTURER'S INSTALLATION INSTRUCTIONS.

B. EXTERIOR WALLS SHALL BE 7/16 (1/2) INCH APA RATED WOOD STRUCTURAL PANELS, UNO AS SHEAR WALL. FASTEN PANELS TO STUDS WITH 8D NAILS AT 6 INCHES ON CENTER AT PANEL EDGES AND 12 INCHES ON CENTER AT INTERMEDIATE SUPPORTS.UNLESS NOTED OTHERWISE, PANEL EDGES NEED NOT BE BLOCKED. IF EXTERIOR WALLS ARE DENOTED AS SHEAR WALLS, THEY SHALL BE SHEATHED, FASTENED AND BLOCKED AS SCHEDULED ON THE DRAWINGS.

C. SHEARWALLS SHALL BE SHEATHED, FASTENED AND BLOCKED AS SCHEDULED ON THE DRAWINGS.

D. ROOF SHEATHING SHALL BE 23/32 (3/4) INCH APA RATED WOOD STRUCTURAL PANEL, TONGUE AND GROOVE, WITH SPAN RATING OF 48/24. PANELS SHALL HAVE LONG DIMENSION ORIENTED ACROSS THREE OR MORE JOISTS AND SHALL BE FASTENED WITH CONSTRUCTION ADHESIVE AND NAILS AT PANEL EDGES AND INTERMEDIATE SUPPORTS AS SCHEDULED ON THE DRAWINGS. UNLESS NOTED OTHERWISE, PANEL EDGES NEED NOT BE BLOCKED.

8. POST-INSTALLED ANCHORS IN CONCRETE AND MASONRY

A. GENERAL

INSTALL ANCHORS IN STRICT CONFORMANCE WITH THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS AND PROCEDURES. ALL POST-INSTALLED ANCHORS IN CONCRETE SHALL HAVE ICC APPROVAL FOR USE IN CRACKED CONCRETE.

SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE SUBMITTED FOR APPROVAL PRIOR TO USE. CONTRACTOR SHALL PROVIDE LOAD CAPACITIES DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT.

PROVIDE STAINLESS STEEL FASTENERS FOR EXTERIOR USE OR WHEN PERMANENTLY EXPOSED TO WEATHER. PROVIDE GALVANIZED CARBON STEEL ANCHORS AT OTHER LOCATIONS, UNLESS OTHERWISE NOTED.

B. PRODUCTS

ANCHORS IN CONCRETE:
--EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT TZ.
--UNDERCUT ANCHORS SHALL BE HILTI HDA.
--SCREW ANCHORS SHALL BE HILTI KWIK HUS.
--ADHESIVE ANCHORS SHALL BE HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HIT-Z ROD OR WITH HILTI HOLLOW DRILL BIT SYSTEM WITH HAS-E THREADED ROD.

ANCHORS IN MASONRY:
--EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT TZ. GROUT MASONRY CELLS SOLID WITH 2000 PSI GROUT AT ANCHOR LOCATIONS.
--SCREW ANCHORS SHALL BE HILTI KWIK HUS. GROUT MASONRY CELLS SOLID WITH 2000 PSI GROUT AT ANCHOR LOCATIONS.
--ADHESIVE ANCHORS IN SOLID MASONRY SHALL BE HILTI HIT-HY 270 ADHESIVE ANCHORING SYSTEM. STEEL ANCHOR ELEMENT SHALL BE HILTI HAS-E CONTINUOUSLY THREADED ROD OR HILTI HIS-N INTERNALLY THREADED INSERT.
--ADHESIVE ANCHORS IN HOLLOW OR MULTI-WYTHE MASONRY SHALL BE HILTI HIT-HY 270 ADHESIVE ANCHORING SYSTEM. STEEL ANCHOR ELEMENT SHALL BE HILTI HAS-E CONTINUOUSLY THREADED ROD OR HILTI HIT-IC INTERNALLY THREADED INSERT. THE APPROPRIATE SIZE SCREEN TUBE SHALL BE USED PER THE ADHESIVE MANUFACTURER'S RECOMMENDATION.

C. INSTALLATION

ALL INSTALLATION PROCEDURES SHALL BE PER MANUFACTURERS RECOMMENDATIONS. COORDINATE AND/OR PROVIDE FOR THIRD PARTY INSPECTION AS REQUIRED BY BUILDING CODE OR LOCAL JURISDICTION.

ANCHOR CAPACITY IS DEPENDENT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHOR TO EDGE OF CONCRETE OR MASONRY. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE DISTANCE INDICATED ON THE DRAWINGS; IF NOT SHOWN, COMPLY WITH MINIMUM SPACING AND EDGE DISTANCE FOR FULL ANCHOR CAPACITY, AS SPECIFIED BY MANUFACTURER.

EXISTING REINFORCING BARS IN THE CONCRETE OR MASONRY STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. DO NOT CUT OR DAMAGE REINFORCING BARS UNLESS SPECIFICALLY PERMITTED IN THE DRAWINGS.

PRIOR TO DRILLING, THE CONTRACTOR SHALL LOCATE REINFORCING BAR POSITIONS IN THE IMMEDIATE VICINITY OF PROPOSED POST-INSTALLED ANCHORS USING GPR, X-RAY, OR OTHER NON-DESTRUCTIVE MEANS.

WHEN CONFLICTS BETWEEN PROPOSED ANCHORS AND EXISTING REINFORCING BARS EXIST, SUBMIT RESULTS OF BAR LOCATIONS TO ARCHITECT / ENGINEER FOR REVIEW AND FURTHER DIRECTION.

WARNING: THE STRUCTURAL INTEGRITY OF THE BUILDING SHOWN ON THESE PLANS IS DEPENDENT UPON COMPLETION ACCORDING TO PLANS AND SPECIFICATIONS. STRUCTURAL MEMBERS ARE NOT SELF-BRACING UNTIL PERMANENTLY AFFIXED TO THE STRUCTURE. THE STRUCTURAL ENGINEERS ASSUME NO LIABILITY FOR THE STRUCTURE DURING CONSTRUCTION.
09/18

ABBREVIATIONS & LEGEND

A		K	
AB	ANCHOR BOLT	K	KIP
ADDL	ADDITIONAL	KO	KNOCK-OUT
ADJ	ADJACENT	KSI	KIPS PER SQ. INCH
AFF	ABOVE FINISH FLOOR		
ALT	ALTERNATE	L	
APPROX	APPROXIMATE(LY)	L	LINTEL MARK
ARCH	ARCHITECT(URAL)	LLH	LONG LEG HORIZONTAL
		LLV	LONG LEG VERTICAL
B		LL	LIVE LOAD
B	BEAM MARK	LP	LOW POINT
BF	BOTTOM OF FOOTING ELEVATION	LVL	LAMINATED VENEER LUMBER
	BRACED FRAME MARK		
BLKG	BLOCKING	M	
BLDG	BUILDING	MANUF	MANUFACTURER(ED)
BM	BEAM	MAS	MASONRY
BOD	BOTTOM OF DECK	MAX	MAXIMUM
BOS	BOTTOM OF STEEL	MIN	MINIMUM
BOTT	BOTTOM	MISC	MISCELLANEOUS
BP	BEARING PLATE MARK	MO	MASONRY OPENING
BRG	BEARING	MATL	MATERIAL
BSMT	BASEMENT	MTL	METAL
BTWN	BETWEEN		
C		N	
C	COLUMN MARK	NTS	NOT TO SCALE
CIP	CAST IN PLACE	NS	NEAR SIDE
CJ	CONTROL/CONSTRUCTION JOINT	NIC	NOT IN CONTRACT
CLR	CLEAR(ANCE)	O	
CMU	CONCRETE MASONRY UNIT	OC	ON CENTER(S)
COL	COLUMN	OPNG	OPENING
COM	CENTER OF MASONRY WALL	OPP	OPPOSITE
COMP	COMPOSITE	OF	OUTSIDE FACE
CONC	CONCRETE		
CONN	CONNECTION	P	
CONST	CONSTRUCTION	P	PIER MARK
CONT	CONTINUOUS	PC	PRECAST CONCRETE
COORD	COORDINATE(TION)	PDF	POWER DRIVEN FASTENER
COS	CENTER OF STUD	PEB	PRE-ENGINEERED BUILDING
		PERIM	PERIMETER
D		PL	PLATE
DBA	DEFORMED BAR ANCHORS	PLF	POUNDS PER LINEAR FOOT
DTL	DETAIL	PP	PRECAST PLANK MARK
DIAM	DIAMETER	PROJ	PROJECTION
DIAG	DIAGONAL	PSF	POUNDS PER SQ. FOOT
DN	DOWN	PSI	POUNDS PER SQ. INCH
DWG	DRAWING	PSL	PARALLEL STRAND LUMBER COLUMN
DBL	DOUBLE	PT	POST TENSION(ED)/PRESSURE TREATED
DL	DEAD LOAD		
E		Q	
EA	EACH	QTY	QUANTITY
EE	EACH END		
EF	EACH FACE	R	
EL	ELEVATION	RAD	RADIUS
ELEV	ELEVATOR	RD	ROOF DRAIN
EOD	EDGE OF DECK	REV	REVISION, REVISE(D)
EOJ	EDGE OF JOIST	REINF	REINFORCE(D), (ING)
EOS	EDGE OF SLAB	REM	REMAINDER
EQ	EQUAL	REQ'D	REQUIRED
EQUIP	EQUIPMENT	RTU	ROOF TOP UNIT
ES	EACH SIDE		
EW	EACH WAY	S	
EXIST, EX	EXISTING	SB	SOIL BORING
EXP	EXPANSION	SC	SLIP CRITICAL
EXT	EXTERIOR	SE	SPECIALTY DESIGN ENGINEER
		SIM	SIMILAR
F		SJI	STEEL JOIST INSTITUTE
F	FOOTING MARK	SOG	SLAB ON GRADE
FD	FLOOR DRAIN	SQ	SQUARE
FDN	FOUNDATION	STD	STANDARD
FOB	FACE OF BUILDING	STL	STEEL
FOM	FACE OF MASONRY WALL	STRUCT	STRUCTURAL
FOS	FACE OF STUD	SP

30x42 SCHEDULE OF SPECIAL INSPECTIONS

		VERIFICATION AND INSPECTION	Y/N	FREQUENCY CONTINUOUS	FREQUENCY PERIODIC	REFERENCED STANDARD	IBC REFERENCE	SCOPE OF SERVICE	RESPONSIBLE PARTY
		STRUCTURAL STEEL					1705.2	-	
1.		HIGH-STRENGTH BOLTING: INSPECTION TASKS PRIOR TO BOLTING							
	A.	MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS.	Y	X	-		1705.2	-	SIER
	B.	FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	Y	-	X	AISC 360 & applicable ASTM material standards	1705.2	-	SIER
	C.	PROPER FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE).	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS ONLY	SIER
	D.	PROPER BOLTING PROCEDURE SELECTED FOR JOINT DETAIL.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS ONLY	SIER
	E.	CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION & HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS ONLY	SIER
	F.	PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED & DOCUMENTED FOR FASTENER ASSEMBLIES & METHODS USED.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS ONLY	SIER
	G.	PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS, & OTHER FASTENER COMPONENTS.	Y	-	X	AISC 360	1705.2	-	SIER
2.		HIGH-STRENGTH BOLTING: INSPECTION TASKS DURING BOLTING							
	A.	FASTENER ASSEMBLIES, OF SUITABLE CONDITION, PLACED IN ALL HOLES & WASHERS (IF REQUIRED) ARE POSITIONED AS REQUIRED.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS USING CALIBRATED WRENCH METHOD OR TURN-OF-NUT WITHOUT MATCHMARKING ONLY	SIER
	B.	JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO PRETENSIONING OPERATION.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS USING CALIBRATED WRENCH METHOD OR TURN-OF-NUT WITHOUT MATCHMARKING ONLY	SIER
	C.	FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS USING CALIBRATED WRENCH METHOD OR TURN-OF-NUT WITHOUT MATCHMARKING ONLY	SIER
	D.	FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS USING CALIBRATED WRENCH METHOD OR TURN-OF-NUT WITHOUT MATCHMARKING ONLY	SIER
3.		HIGH-STRENGTH BOLTING: INSPECTION TASKS AFTER BOLTING							
	A.	DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS.	Y	X	-	AISC 360	1705.2	-	SIER
4.		WELDING: INSPECTION TASKS PRIOR TO WELDING							
	A.	WELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE.	Y	X	X	AISC 360 & applicable AWS Documents	1705.2	ESTABLISH THE JOINT WELDING PROCEDURES ARE PREQUALIFIED OR TEST IN ACCORDANCE WITH AWS D1.1 QUALIFICATION PROCEDURES	SIER
	B.	MANUFACTURER'S CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE.	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	C.	MATERIAL IDENTIFICATION (TYPE/GRADE).	Y	-	X	AISC 360	1705.2	-	SIER
	D.	FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY):							
		1) JOINT PREPARATION	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		2) DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		3) CLEANLINESS (CONDITION OF STEEL SURFACES)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		4) TACKING (TACK WELD QUALITY & LOCATION)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		5) BACKING TYPE & FIT (IF APPLICABLE)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	E.	CONFIGURATION & FINISH OF ACCESS HOLES	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	F.	FIT-UP OF FILLET WELDS:							
		1) DIMENSIONS (ALIGNMENT, GAPS AT ROOT)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		2) CLEANLINESS (CONDITION OF STEEL SURFACES)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		3) TACKING (TACK WELD QUALITY & LOCATION)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
5.		WELDING: INSPECTION TASKS DURING WELDING							
	A.	USE OF QUALIFIED WELDERS	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	WELDERS MUST BE CURRENTLY CERTIFIED UNDER AMERICAN WELDING SOCIETY QUALIFICATION PROCEDURES.	SIER
	B.	CONTROL & HANDLING OF WELDING CONSUMABLES INCLUDING PACKAGING & EXPOSURE CONTROL	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	C.	ENVIRONMENTAL CONDITIONS: WIND SPEED WITHIN LIMITS, PRECIPITATION, & TEMPERATURE.	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	D.	WPS FOLLOWED: SETTINGS ON WELDING EQUIPMENT, TRAVEL SPEED, SELECTED WELDING MATERIALS, SHIELDING GAS TYPE/FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED (MIN./MAX.), PROPER POSITION (F,V,H,OH).	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	E.	WELDING TECHNIQUES: INTERPASS & FINAL CLEANING, EACH PASS WITHIN PROFILE LIMITATIONS, EACH PASS MEETS QUALITY REQUIREMENTS.	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
6.		WELDING: INSPECTION TASKS AFTER WELDING							
	A.	WELDS CLEANED	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	B.	SIZE, LENGTH, & LOCATION OF WELDS	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	C.	WELDS MEET VISUAL ACCEPTANCE CRITERIA: CRACK PROHIBITION, WELD/BASE-METAL FUSION, CRATER CROSS SECTION, WELD PROFILES, WELD SIZE, UNDERCUT, POROSITY	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	PERFORM ULTRASONIC TESTING OF ALL FULL PENETRATION FIELD & SHOP WELDS TO COMPLY WITH ASTM E 164 PER PROJECT SPECIFICATIONS.	SIER
	D.	ARC STRIKES	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	E.	K-AREA	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	WHEN WELDING DOUBLER PLATES, CONTINUITY PLATES, OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 IN. OF THE WELD.	SIER
	F.	BACKING REMOVED & WELD TABS REMOVED (IF REQUIRED)	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	G.	REPAIR ACTIVITIES	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	H.	DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
7.		STEEL ELEMENT OF COMPOSITE CONSTRUCTION PRIOR TO CONCRETE PLACEMENT							
	A.	PLACEMENT & INSTALLATION OF STEEL DECK	N	-	-	AWS D1.3 AISC 360	1705.2	-	-
	B.	PLACEMENT & INSTALLATION OF STEEL HEADED STUD ANCHORS	N	-	-	AWS D1.1 AISC 360	1705.2	-	-
	C.	DOCUMENT ACCEPTANCE OR REJECTION OF STEEL ELEMENTS	N	-	-	AISC 360	1705.2	-	-
8.		INSPECTION OF FABRICATORS & FABRICATION PROCEDURES	N	-	-	AISC 360	1704.2.5	-	-
		COLD-FORMED METAL DECK							
1.		VERIFY COMPLIANCE OF MATERIALS (ALL DECK AND ACCESSORIES) WITH CONSTRUCTION DOCUMENTS, INCLUDING PROFILES, MATERIAL PROPERTIES, AND BASE METAL THICKNESS AND DOCUMENT ACCEPTANCE OR REJECTION	N	-	-	SDI QA/QC	1705.2.2	-	-
2.		VERIFY COMPLIANCE OF INSTALLATION OF ALL DECK AND ACCESSORIES WITH CONSTRUCTION DOCUMENTS AND VERIFY ACCEPTANCE OR REJECTION	N	-	-	SDI QA/QC	1705.2.2	-	-
3.		VERIFY DECK MATERIALS ARE REPRESENTED BY THE MILL CERTIFICATIONS THAT COMPLY WITH THE CONSTRUCTION DOCUMENTS	N	-	-	SDI QA/QC	1705.2.2	-	-
4.		DECK WELDING							
	A.	WELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE.	N	-	-	SDI QA/QC & applicable AWS Documents		-	-
	B.	MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	C.	MATERIAL IDENTIFICATION (TYPE/GRADE).	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	D.	CHECK WELDING EQUIPMENT	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	E.	USE OF QUALIFIED WELDERS	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	F.	CONTROL & HANDLING OF WELDING CONSUMABLES	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	G.	ENVIRONMENTAL CONDITIONS: WIND SPEED WITHIN LIMITS, PRECIPITATION, & TEMPERATURE.	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	H.	WPS FOLLOWED: SETTINGS ON WELDING EQUIPMENT, TRAVEL SPEED, SELECTED WELDING MATERIALS, SHIELDING GAS TYPE/FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED (MIN./MAX.), PROPER POSITION (F,V,H,OH).	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	I.	VERIFY SIZE AND LOCATION OF WELDS, INCLUDING SUPPORT, SIDELAP, AND PERIMETER WELDS	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	J.	WELDS MEET VISUAL ACCEPTANCE CRITERIA: CRACK PROHIBITION, WELD/BASE-METAL FUSION, CRATER CROSS SECTION, WELD PROFILES, WELD SIZE, UNDERCUT, POROSITY	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	K.	VERIFY REPAIR ACTIVITIES	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	L.	DOCUMENT ACCEPTANCE OR REJECTION OF WELDS	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
5.		DECK MECHANICAL FASTENING							
	A.	MANUFACTURER INSTALLATION INSTRUCTIONS AVAILABLE FOR MECHANICAL FASTENERS	N	-	-	SDI QA/QC	1705.2.2	-	-
	B.	PROPER TOOLS AVAILABLE FOR FASTENER INSTALLATION	N	-	-	SDI QA/QC	1705.2.2	-	-
	C.	PROPER STORAGE FOR MECHANICAL FASTENERS	N	-	-	SDI QA/QC	1705.2.2	-	-
	D.	FASTENERS ARE POSITIONED AS REQUIRED	N	-	-	SDI QA/QC	1705.2.2	-	-
	E.	FASTENERS ARE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS	N	-	-	SDI QA/QC	1705.2.2	-	-
	F.	CHECK SPACING, TYPE, AND INSTALLATION OF SUPPORT FASTENERS.	N	-	-	SDI QA/QC	1705.2.2	-	-
	G.	CHECK SPACING, TYPE, AND INSTALLATION OF SIDELAP FASTENERS.	N	-	-	SDI QA/QC	1705.2.2	-	-
	H.	CHECK SPACING, TYPE, AND INSTALLATION OF PERIMETER FASTENERS.	N	-	-	SDI QA/QC	1705.2.2	-	-
	I.	VERIFY REPAIR ACTIVITIES	N	-	-	SDI QA/QC	1705.2.2	-	-
	J.	DOCUMENT ACCEPTANCE OR REJECTION OF MECHANICAL FASTENERS	N	-	-	SDI QA/QC	1705.2.2	-	-
		OPEN-WEB STEEL JOISTS AND JOIST GIRDERS							
1.		END CONNECTIONS - WELDING OR BOLTED	N	-	-	SJI Specifications	1705.2.3	-	-
2.		BRIDGING - HORIZONTAL OR DIAGONAL							
	A.	STANDARD BRIDGING	N	-	-	SJI Specifications	1705.2.3	-	-
	B.	BRIDGING THAT DIFFERS FROM THE SJI SPECIFICATIONS	N	-	-	Construction documents & approved shop drawings	1705.2.3	-	-
		CONCRETE							
1.		INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, & PLACEMENT.	Y	-	X	ACI 318	1705.3	VERIFY SIZE, LOCATION, SPACING ORIENTATION, COVER, SPLICING, & CONFORMANCE WITH THE CONTRACT DOCUMENTS, AS SUPPLEMENTED WITH APPROVED SHOP DRAWINGS OR OTHER SUBMITTALS. CONFIRM THAT THE SURFACE OF THE REINFORCING STEEL IS FREE OF FORM RELEASE OIL OR OTHE	SIER
2.		INSPECTION OF REINFORCING STEEL WELDING.							
	A.	VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706.	N	-	-	AWS D1.4 ACI 318	1705.3	-	-
	B.	INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"	N	-	-	AWS D1.4 ACI 318	1705.3	-	-
	C.	INSPECT ALL OTHER WELDS	N	-	-	AWS D1.4 ACI 318	1705.3	-	-
3.		INSPECT ANCHORS CAST-IN CONCRETE.	Y	-	X	ACI 318	1705.3	PRECISE LOCATION OF ANCHOR RODS IS NOT EXPECTED BUT VERIFY THE CONTRACTOR HAS TAKEN APPROPRIATE STEPS TO CORRECTLY POSITION THEM SUCH AS ENGAGING A SURVEYOR OR SETTING UP A SYSTEM OF STRING LINES & BATTER BOARDS & THAT CORRECT GRADE & SIZE OF ANCHORS IS	SIER

30x42 SCHEDULE OF SPECIAL INSPECTIONS

		VERIFICATION AND INSPECTION	Y/N	FREQUENCY CONTINUOUS	FREQUENCY PERIODIC	REFERENCED STANDARD	IBC REFERENCE	SCOPE OF SERVICE	RESPONSIBLE PARTY
4.		INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE.							
	A.	ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS	Y	X	-	ACI 318		0	
	B.	MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED ABOVE	Y	-	X	ACI 318		0	
5.		VERIFYING USE OF REQUIRED DESIGN MIX.	Y	-	X	ACI 318	1705.3	VERIFY APPROVED MIX DESIGN	SIER
6.		AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM, SLUMP & AIR CONTENT TESTS, & DETERMINE THE TEMPERATURE OF THE CONCRETE.	Y	X	-	ASTM C 172/ASTM C 31/ACI 318	1705.3	TEST IN ACCORDANCE WITH PROJECT SPECIFICATIONS, BUT NOT LESS THAN ONCE PER DAY PER CLASS OF CONCRETE OR ONCE PER 150 CUBIC YARDS PER DAY OR ONCE PER 5,000 SQUARE FEET OF SLAB OR WALL PER DAY. CYLINDERS MUST BE PROPERLY HANDLED & STORED ON SITE UNTIL TRANS	SIER
7.		INSPECTION OF CONCRETE & SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	Y	X	-	ACI 318	1705.3	VERIFY CONFORMANCE WITH PROJECT SPECIFICATIONS. INSPECTOR SHALL BE WHERE THE CONCRETE IS BEING PLACED RATHER THAN WHERE CONCRETE TRUCKS ARE DISCHARGING THEIR LOADS. INSPECTOR NEEDS TO BE PRESENT WHILE SLAB IS BEING FLOATED & POWER TROWELED.	SIER
8.		INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE & TECHNIQUES.	Y	-	X	ACI 318	1705.3	VERIFY CONFORMANCE WITH PROJECT SPECIFICATIONS & ACI	SIER
9.		INSPECTION OF PRESTRESSED CONCRETE:							
	A.	APPLICATION OF PRESTRESSING FORCES.	N	-	-	ACI 318	1705.3	-	-
	B.	GROUTING OF BONDED PRESTRESSING TENDONS.	N	-	-	ACI 318	1705.3	-	-
10.		ERECTION OF PRECAST CONCRETE MEMBERS.	N	-	-	ACI 318	1705.3	-	-
11.		VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POSTTENSIONED CONCRETE & PRIOR TO REMOVAL OF SHORES & FORMS FROM BEAMS & STRUCTURAL SLABS.	N	-	-	ACI 318	1705.3	-	-
12.		INSPECT FORMWORK FOR SHAPE, LOCATION & DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	N	-	-	ACI 318	1705.3	-	-
		MASONRY							
		LEVEL A QUALITY ASSURANCE: RISK CATEGORY I, II, OR III STRUCTURES DESIGNED IN ACCORDANCE WITH PART 4 OR APPENDIX A.	Y				1705.4		
		LEVEL B QUALITY ASSURANCE: RISK CATEGORY IV STRUCTURES DESIGNED IN ACCORDANCE WITH CHAPTERS 12 OR 13 & RISK CATEGORY I, II, OR III STRUCTURES DESIGNED IN ACCORDANCE WITH CHAPTERS OTHER THAN THOSE IN PART 4 OR APPENDIX A.	N				1705.4		
		LEVEL C QUALITY ASSURANCE: RISK CATEGORY IV STRUCTURES DESIGNED IN ACCORDANCE WITH CHAPTERS OTHER THAN PART 4 OR APPENDIX A.	N				1705.4		
1.		PRIOR TO CONSTRUCTION VERIFY CERTIFICATES OF COMPLIANCE USED IN MASONRY CONSTRUCTION AND DURING CONSTRUCTION COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS & THE APPROVED SUBMITTALS SHALL BE VERIFIED.	Y	-	X	ACI530.1	1705.4	VERIFY COMPLIANCE WITH APPROVED SHOP DRAWINGS.	SIER
2.		VERIFICATION OF F'M & F'AAC PRIOR TO CONSTRUCTION (AND FOR EVERY 5,000 SQUARE FEET DURING CONSTRUCTION FOR LEVEL C).	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
3.		VERIFICATION OF PROPORTIONS OF MATERIALS IN PREMIXED OR PREBLENDED MORTAR, PRESTRESSING GROUT, & GROUT OTHER THAN SELF-CONSOLIDATING GROUT AS DELIVERED TO THE SITE OR PROPORTIONS OF SITE PREPARED MORTAR.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
4.		VERIFICATION OF SLUMP FLOW & VSI AS DELIVERED TO THE SITE FOR SELF-CONSOLIDATING GROUT.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
5.		THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:							
	A.	PROPORTIONS OF SITE-PREPARED MORTAR, GROUT, & PRESTRESSING GROUT FOR BONDED TENDONS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	B.	PLACEMENT OF MASONRY UNITS & CONSTRUCTION OF MORTAR JOINTS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	C.	GRADE, TYPE, & SIZE OF REINFORCEMENT, ANCHOR BOLTS, PRESTRESSING TENDONS, & ANCHORAGES	N	-	-	TMS 402/ACI 530/ASCE 5/TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	D.	PLACEMENT OF REINFORCEMENT, CONNECTORS & PRESTRESSING TENDONS & ANCHORAGES.	N	-	-	TMS 402/ACI 530/ASCE 5/TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	E.	GROUT SPACE PRIOR TO GROUTING.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	F.	PLACEMENT OF GROUT & PRESTRESSING GROUT FOR BONDED TENDONS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	G.	SIZE & LOCATION OF STRUCTURAL ELEMENTS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	H.	TYPE, SIZE & LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION.	N	-	-	TMS 402/ACI 530/ASCE 5	1705.4	-	-
	I.	WELDING OF REINFORCEMENT.	N	-	-	TMS 402/ACI 530/ASCE 5	1705.4	-	-
	J.	PREPARATION, CONSTRUCTION & PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F).	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	K.	APPLICATION & MEASUREMENT OF PRESTRESSING FORCE.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	L.	PLACEMENT OF AAC MASONRY UNITS & CONSTRUCTION OF THIN-BED MORTAR JOINTS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	M.	PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	N.	VERIFY PRE-STRESSING TECHNIQUE IS IN COMPLIANCE AS CONSTRUCTION BEGINS	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	
6.		OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
		WOOD CONSTRUCTION							
1.		INSPECTION OF FABRICATORS & FABRICATION PROCEDURES FOR PREFABRICATED WOOD STRUCTURAL ELEMENTS.	Y	-	X		1705.5	-	SIER
2.		HIGH-LOAD DIAPHRAGMS DESIGNED IN ACCORDANCE WITH SECTION 2306.2	N	-	X		1705.5	-	SIER
3.		METAL PLATE CONNECTED WOOD TRUSSES SPANNING 60 FEET OR MORE	N	-	-		1705.5	-	-
		SOILS						-	
1.		VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	Y	-	X	Geotechnical Report	1705.6	-	SIER
2.		VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH & HAVE REACHED PROPER MATERIAL.	Y	-	X	Geotechnical Report	1705.6	-	SIER
3.		PERFORM CLASSIFICATION & TESTING OF COMPACTED FILL MATERIALS.	Y	-	X	Geotechnical Report	1705.6	-	SIER
4.		VERIFY USE OF PROPER MATERIALS, DENSITIES, & LIFT THICKNESSES DURING PLACEMENT & COMPACTION OF COMPACTED FILL.	Y	X	-	Geotechnical Report	1705.6	TEST FREQUENCY PER SPECIFICATIONS, BUT NOT LESS THAN ONE TEST EVERY 2,000 SQUARE FEET FOR EACH LAYER OF FILL OR PROOF-ROLLING	SIER
5.		PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE & VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	Y	-	X	Geotechnical Report	1705.6	-	SIER
		DRIVEN DEEP FOUNDATIONS							
1.		VERIFY ELEMENT MATERIALS, SIZES, & LENGTHS COMPLY WITH THE REQUIREMENTS.	N	-	-		1705.7	-	-
2.		DETERMINE CAPACITIES OF TEST ELEMENTS & CONDUCT ADDITIONAL LOAD TESTS, AS REQUIRED.	N	-	-		1705.7	-	-
3.		OBSERVE DRIVING OPERATIONS & MAINTAIN COMPLETE & ACCURATE RECORDS FOR EACH ELEMENT.	N	-	-		1705.7	-	-
4.		VERIFY PLACEMENT LOCATIONS & PLUMBNESS, CONFIRM TYPE & SIZE OF HAMMER, RECORD NUMBER OF BLOWS PER FOOT OF PENETRATION, DETERMINE REQUIRED PENETRATIONS TO ACHIEVE DESIGN CAPACITY, RECORD TIP & BUTT ELEVATIONS, & DOCUMENT ANY DAMAGE TO FOUNDATION ELEMENT.	N	-	-		1705.7	-	-
5.		PERFORM ADDITIONAL INSPECTIONS FOR STEEL ELEMENTS PER STEEL INSPECTION REQUIREMENTS.	N	-	-		1705.7	-	-
6.		PERFORM ADDITIONAL INSPECTIONS FOR CONCRETE & CONCRETE-FILLED ELEMENTS PER CONCRETE INSPECTION REQUIREMENTS.	N	-	-		1705.7	-	-
7.		PERFORM ADDITIONAL INSPECTIONS FOR SPECIALTY ELEMENTS AS DETERMINED BY THE REGISTERED DESIGN PROFESSIONAL IN CHARGE.	N	-	-		1705.7	-	-
		CAST-IN-PLACE DEEP FOUNDATIONS							
1.		OBSERVE DRILLING OPERATIONS & MAINTAIN COMPLETE & ACCURATE RECORDS FOR EACH ELEMENT.	N	-	-		1705.8	-	-
2.		VERIFY PLACEMENT LOCATIONS & PLUMBNESS, CONFIRM ELEMENT DIAMETERS, BELL DIAMETERS (IF APPLICABLE), LENGTHS, EMBEDMENT INTO BEDROCK (IF APPLICABLE), & ADEQUATE END-BEARING STRATA CAPACITY. RECORD CONCRETE OR GROUT VOLUMES.	N	-	-		1705.8	-	-
3.		PERFORM ADDITIONAL INSPECTIONS FOR CONCRETE ELEMENTS PER CONCRETE INSPECTION REQUIREMENTS.	N	-	-		1705.8	-	-
		HELICAL PILE FOUNDATIONS							
1.		RECORD INSTALLATION EQUIPMENT USED, PILE DIMENSIONS, TIP ELEVATIONS, FINAL DEPTH, & FINAL INSTALLATION TORQUE.	N	-	-	Geotechnical Report & Approved Shop Drawings	1705.9	-	-
		SPRAYED FIRE-RESISTANT MATERIALS						-	
1.		CONDITIONS OF SUBSTRATES	N	-	-	Approved fire-resistance design	1705.14	-	-
2.		THICKNESS OF APPLICATION	N	-	-	Approved fire-resistance design	1705.14	-	-
3.		DENSITY IN POUNDS PER CUBIC FOOT	N	-	-	Approved fire-resistance design	1705.14	-	-
4.		BOND STRENGTH ADHESION /COHESION	N	-	-	Approved fire-resistance design	1705.14	-	-
5.		CONDITION OF FINISHED APPLICATION	N	-	-	Approved fire-resistance design	1705.14	-	-
6.		FIRE-RESISTANT PENETRATIONS & JOINTS	N	-	-	Approved fire-resistance design	1705.17	-	-
		LIGHT GAGE METAL FRAMING	N	-	-	Approved shop drawings	-	-	-
		MASTIC & INTUMESCENT FIRE-RESISTANT COATINGS	N	-	-	AWCI-12B & approved fire-resistance design	1705.15	-	-
		EXTERIOR INSULATION & FINISH SYSTEMS (EIFS)	N	-	-	-	1705.16	-	-

	INSPECTION AGENTS	NAME	COMPANY	ADDRESS
1.	SPECIAL INSPECTION ENGINEER OF RECORD (SIER)	1.	1.	1.
2.	INSPECTION & TESTING AGENCY (ITA)	2.	2.	2.
3.	GEOTECHNICAL ENGINEER OF RECORD (GER)	3.	3.	3
4.	SPECIALTY ENGINEER (SE)	4.	4.	4.
5.	STRUCTURAL ENGINEER OF RECORD (SER)	5. THOMAS A. BOUFFARD, PE	5. EHLERT BRYAN	5. 8609 WESTWOOD CENTER DRIVE, SUITE 800, TYSONS, VA 22182

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06/11/2021

S1.002
SPECIAL INSPECTIONS
12/15/20

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KEYPLAN
SCALE: 1" = 50'-0"

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S1.003
KEY PLAN
03/01/21

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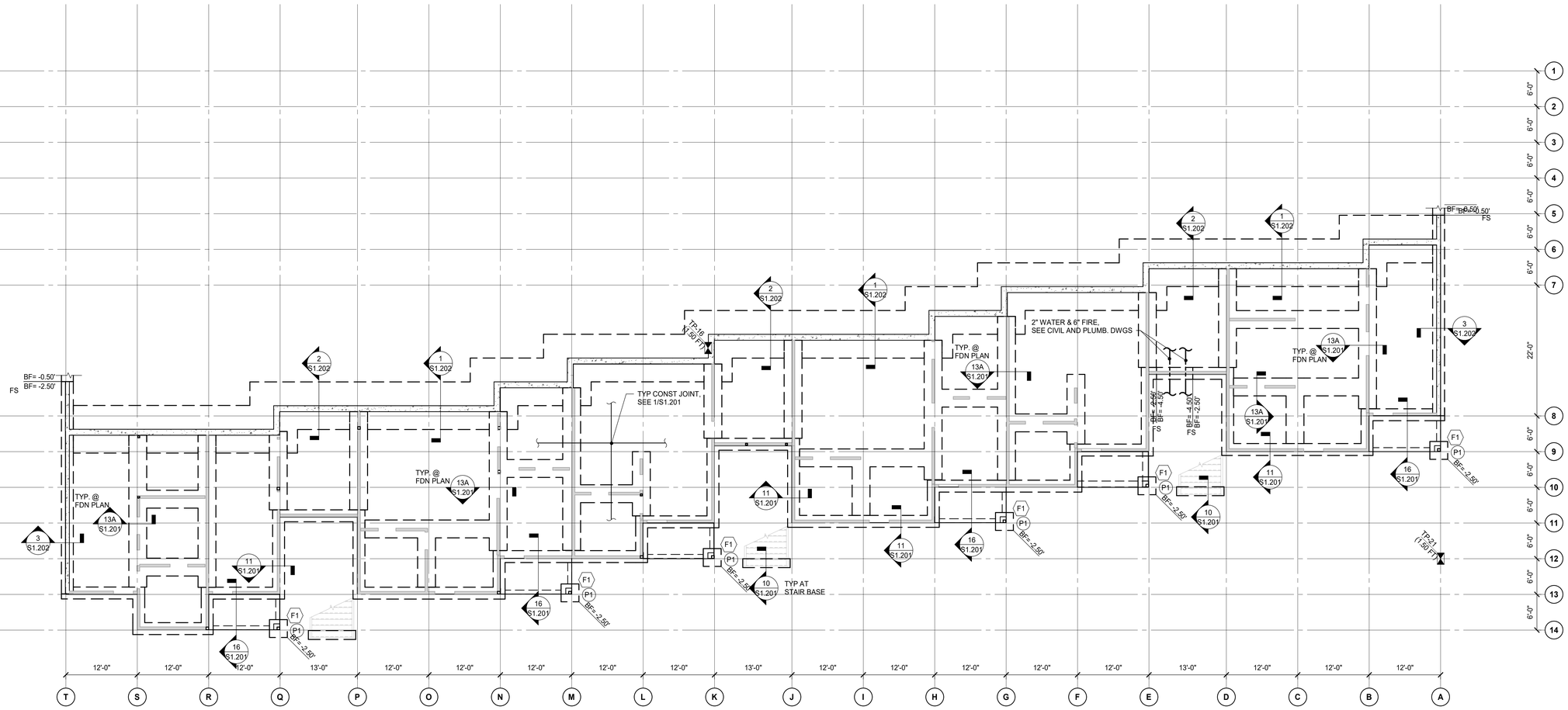

FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. FLOOR CONSTRUCTION; 4'' CONCRETE SLAB-ON-GRADE REINFORCED WITH ONE LAYER OF 6X6-W1.4 X W1.4 WWF IN THE TOP 1/3 OF SLAB PLACED OVER 10 MIL VAPOR RETARDER ON 4'' LAYER OF COMPACTED #57 STONE.
2. TOP OF CONCRETE ELEVATION = 0.00' REFERENCE (ACTUAL ELEVATION= 1016.45')
3. INTERIOR BOTTOM OF FOOTING ELEVATION = -2.50' TYP, UNO.
4. EXTERIOR BOTTOM OF FOOTING ELEVATION = -2.50' TYP, UNO.
5. TP-X DENOTES APPROXIMATE TEST PIT LOCATION.(XX') DENOTES ESTIMATED UNDERCUTS TO SUITABLE SOILS.
6. REFERENCE NOTE 2.D AND 2.E ON S1.001 FOR INFORMATION REGARDING BUILDING PAD REQUIREMENTS. SEE GEOTECHNICAL REPORT FOR MORE INFORMATION.
7. FS DENOTES FOOTING STEP, SEE 2/S1.201.

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S1.100
FOUNDATION PLAN

12/08/20

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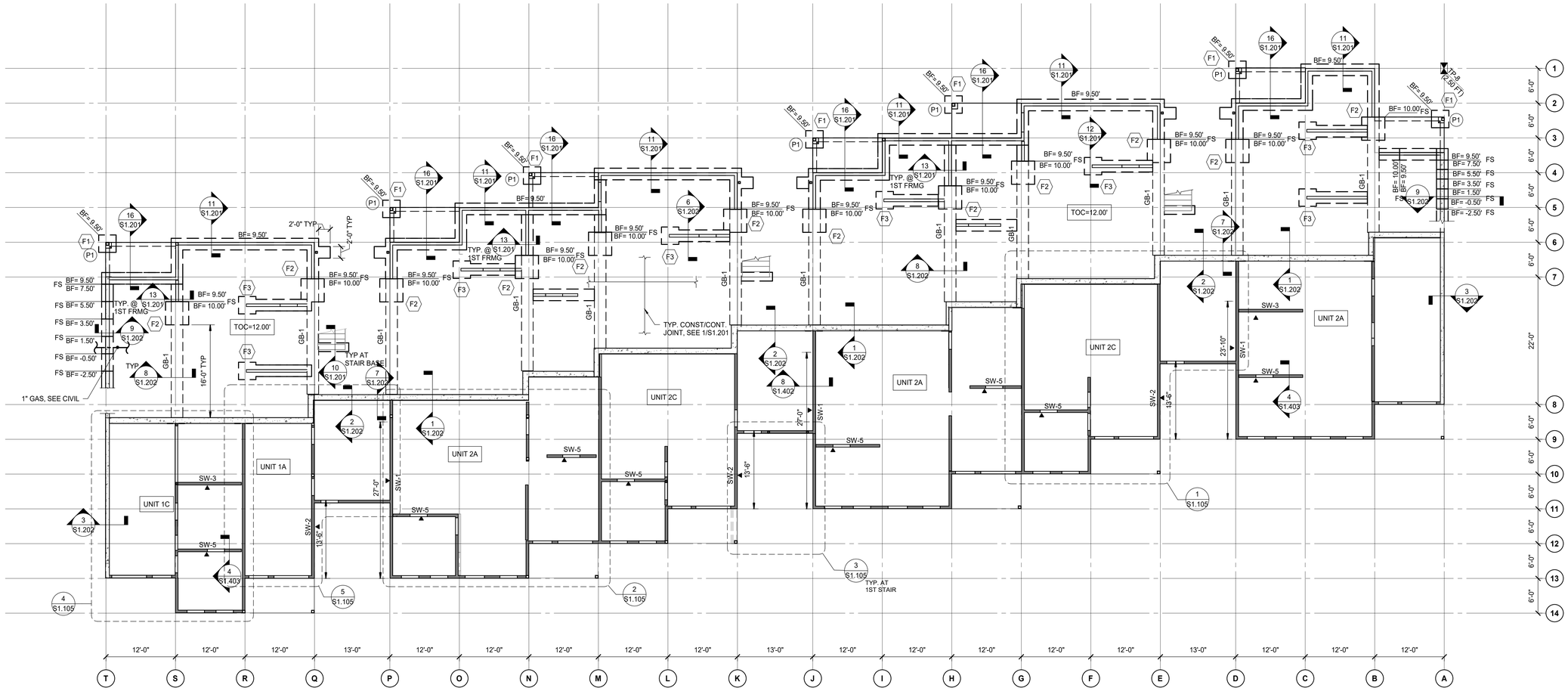

LEVEL 1 FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. FLOOR CONSTRUCTION: 4" CONCRETE SLAB-ON-GRADE REINFORCED WITH ONE LAYER OF 6X6-W1.4 X W1.4 WWF IN THE TOP 1/3 OF SLAB PLACED OVER 10 MIL VAPOR RETARDER ON 4" LAYER OF COMPACTED #57 STONE.
2. TOP OF CONCRETE ELEVATION = 12.00' (REFERENCE EL = 1028.45')
3. INTERIOR BOTTOM OF FOOTING ELEVATION VARIES, SEE PLAN.
4. EXTERIOR BOTTOM OF FOOTING ELEVATION VARIES, SEE PLAN.
5. TOP OF SHEATHING ELEVATION = 11.93'
6. FS DENOTES FOOTING STEP, SEE 2/S1.201.
7. TP-X DENOTES APPROXIMATE TEST PIT LOCATION. (XX') DENOTES ESTIMATED UNDERCUTS TO SUITABLE SOILS.
8. SEE UNIT FRAMING PLANS FOR ELEVATED FLOOR CONSTRUCTION NOTES.
9. GB-1 DENOTES 24"W x 24"D GRADE BEAM W/ 3-#6 CONTINUOUS TOP BARS, 4-#8 CONTINUOUS BOTTOM BARS, AND #3 STIRRUPS @ 10" OC.
10. REFERENCE NOTE 2.D AND 2.E ON S1.001 FOR INFORMATION REGARDING BUILDING PAD REQUIREMENTS. SEE GEOTECHNICAL REPORT FOR MORE INFORMATION.
11. SW-X DENOTES SHEAR WALL. ▶ DENOTES SIDE OF WALL TO BE SHEATHED. S1.403 FOR SHEAR WALL SCHEDULE AND TYPICAL DETAILS. ◄—— XX'-XX" ——► DENOTES SHEAR WALL EXTENTS.

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S1.101
LEVEL 1 FRAMING PLAN

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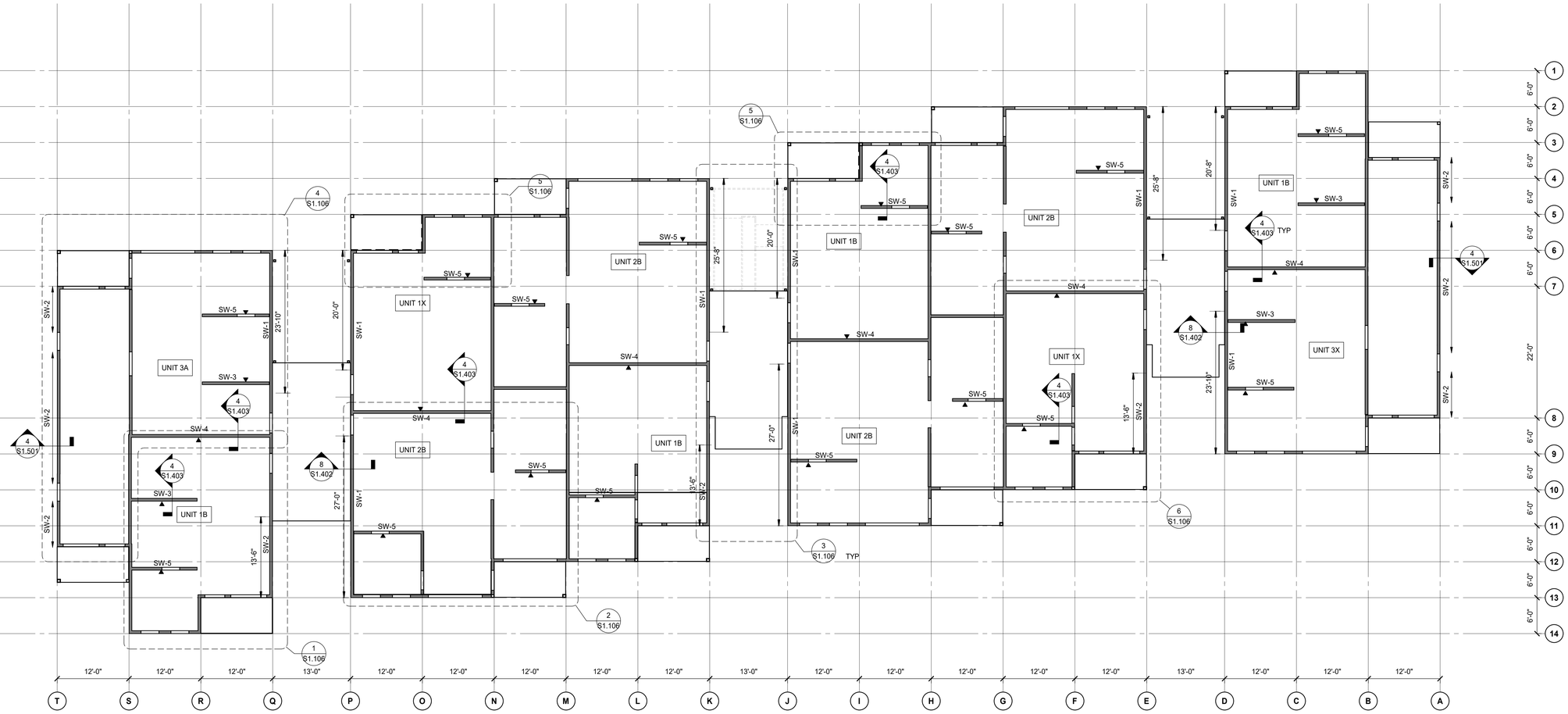

LEVEL 2 FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. SEE UNIT FRAMING PLANS FOR ELEVATED FLOOR CONSTRUCTION NOTES.
2. 2ND FLOOR TOP OF SHEATHING = 22.67'.
3. SW-X DENOTES SHEAR WALL. ▶ DENOTES SIDE OF WALL TO BE SHEATHED. S1.403 FOR SHEAR WALL SCHEDULE AND TYPICAL DETAILS. ⟵ XX-XX" ⟶ DENOTES SHEAR WALL EXTENTS

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S1.102
LEVEL 2 FRAMING PLAN
12/09/20

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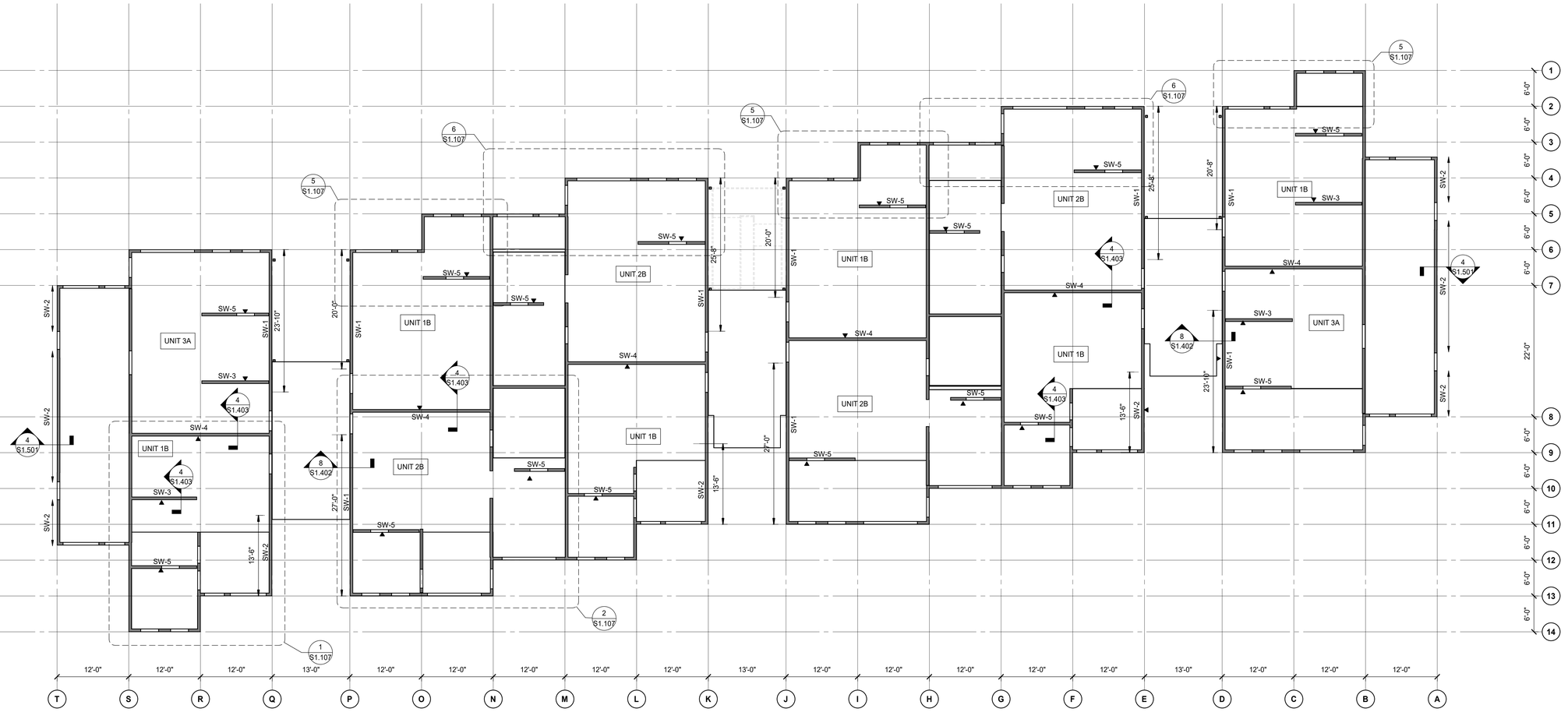

LEVEL 3 FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. SEE UNIT FRAMING PLANS FOR ELEVATED FLOOR CONSTRUCTION NOTES.
2. 2ND FLOOR TOP OF SHEATHING = 33.33'.
3. SW-X DENOTES SHEAR WALL. ▶ DENOTES SIDE OF WALL TO BE SHEATHED. S1.403 FOR SHEAR WALL SCHEDULE AND TYPICAL DETAILS. ⟵ XX'-XX" ⟶ DENOTES SHEAR WALL EXTENTS.

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S1.103

LEVEL 3 FRAMING PLAN

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ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. ROOF CONSTRUCTION 3/4'' STRUCTURAL SHEATHING ON PRE-ENGINEERED ROOF WOOD TRUSSES @ 24'' O.C.
2. ROOF TOP OF SHEATING ELEVATION VARIES, SEE ARCH.
3. TYPICAL SECTIONS SHOWN ARE APPLICABLE TO SIMILAR CONDITIONS EVEN IF MARKS ARE NOT SHOWN.
4. SEE S1.401 FOR TYPICAL HEADERS AND STUDS AT OPENINGS U.N.O.
5. SEE S1.401 FOR COLUMN BEAM & SCHEDULE.
6. TYPICAL CONDITIONS ARE APPLICABLE EVEN IF SECTIONS ARE NOT SHOWN.
3. SW-X DENOTES SHEAR WALL. ▶ DENOTES SIDE OF WALL TO BE SHEATHED. S1.403 FOR SHEAR WALL SCHEDULE AND TYPICAL DETAILS. ◄—— XX'-XX" ——► DENOTES SHEAR WALL EXTENTS.

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S1.104
ROOF FRAMING PLAN

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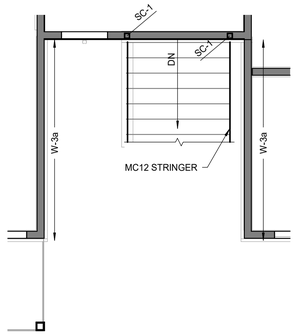
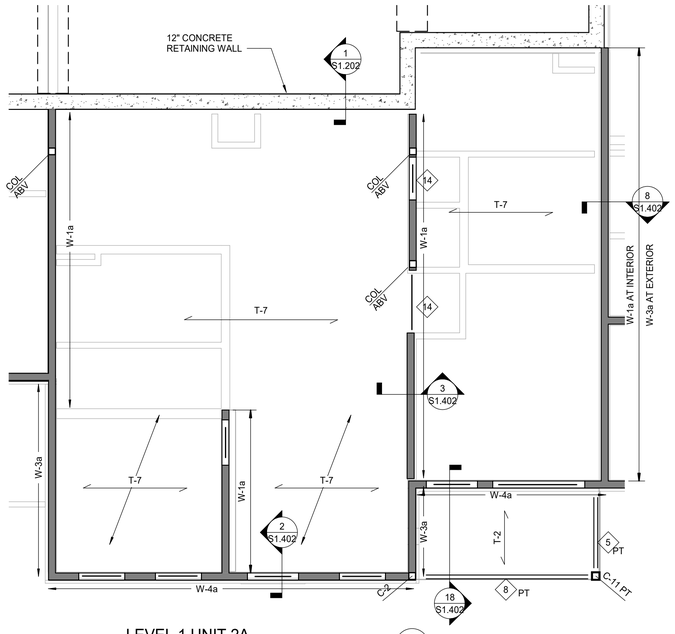
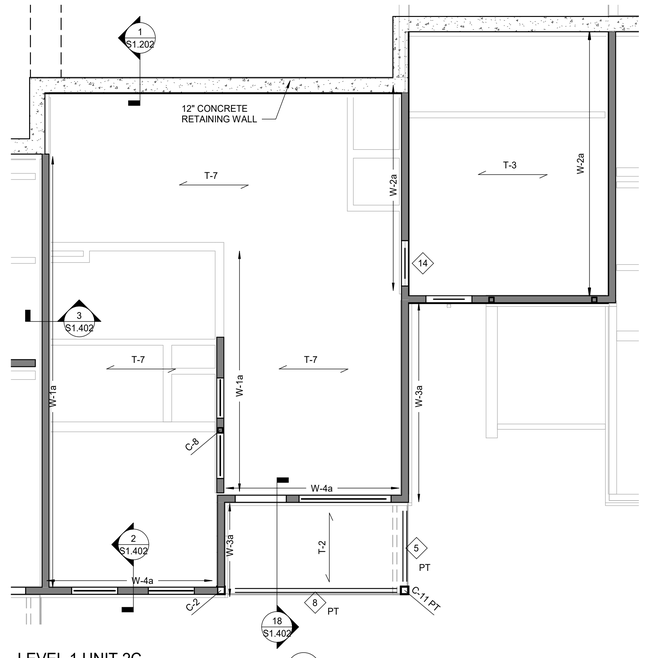


LEVEL 1 STAIR 4
SCALE: 1/4" = 1'-0"
3 S1.105



LEVEL 1 UNIT 2A
SCALE: 1/4" = 1'-0"
2 S1.105



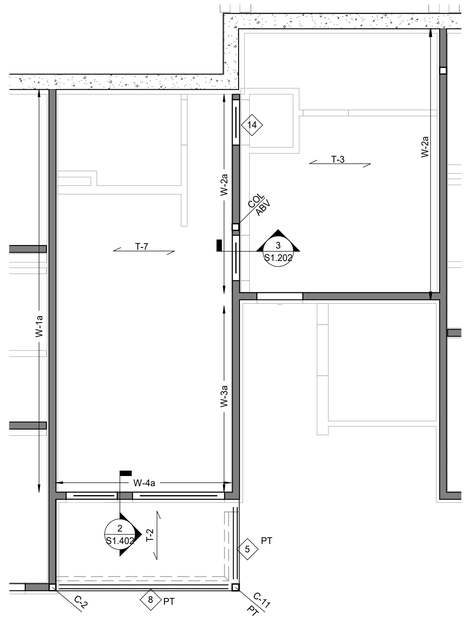
LEVEL 1 UNIT 2C
SCALE: 1/4" = 1'-0"
1 S1.105

LEVEL 1 UNIT FRAMING GENERAL NOTES:

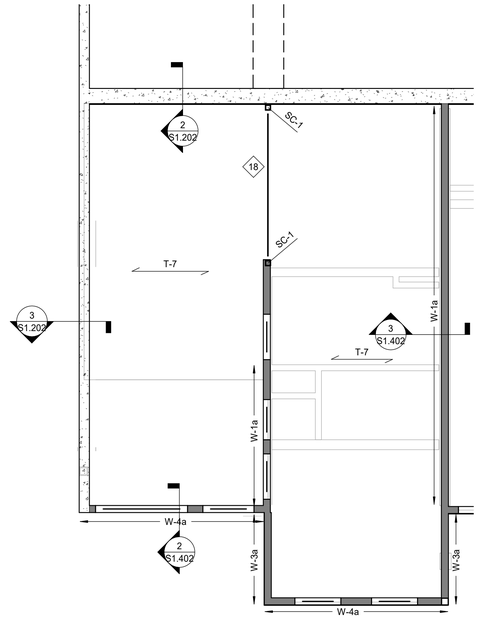
NOTES:

1. TYPICAL FLOOR CONSTRUCTION SHALL BE AS FOLLOWS: UP TO 1" THICK GYPCRETE TOPPING OVER 23/32" (3/4") APA RATED STURDI-FLOOR TONGUE AND GROOVE PANELS GLUED AND SCREWED TO 22" TRUSSES OVER 2x WOOD BEARING WALLS. SEE S1.401 FOR SCHEDULE AND TRUSS LOADING INFORMATION.
2. TOP OF SHEATHING = 11.93' @ LEVEL 1.
3. SEE S1.401 FOR TYPICAL HEADERS, STUDS AT OPENINGS, BEAM AND COLUMN SCHEDULES.
4. DENOTES BEARING WALL.
5. SEE LEVEL 1 FRAMING PLAN ON S1.101 FOR SHEAR WALL LOCATIONS AND NOTES.
6. TYPICAL CONDITIONS ARE APPLICABLE EVEN IF SECTIONS ARE NOT SHOWN.

TRUSS/JOIST SHOP DRAWING SUBMITTAL SHALL BE COORDINATED WITH AND SHALL SHOW ALL BATHTUB, SHOWER AND TOILET DRAINS AND ALL MECHANICAL SHAFTS. ADJUST JOIST SPACING AND/OR ADD JOISTS AND HEADERS TO CLEAR PLUMBING & MECHANICAL.



LEVEL 1 UNIT 1A
SCALE: 1/4" = 1'-0"
5 S1.105



LEVEL 1 UNIT 1C
SCALE: 1/4" = 1'-0"
4 S1.105

REVISIONS
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TERRACES AT HIGH MOUNTAIN
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S1.105
UNIT FRAMING PLANS
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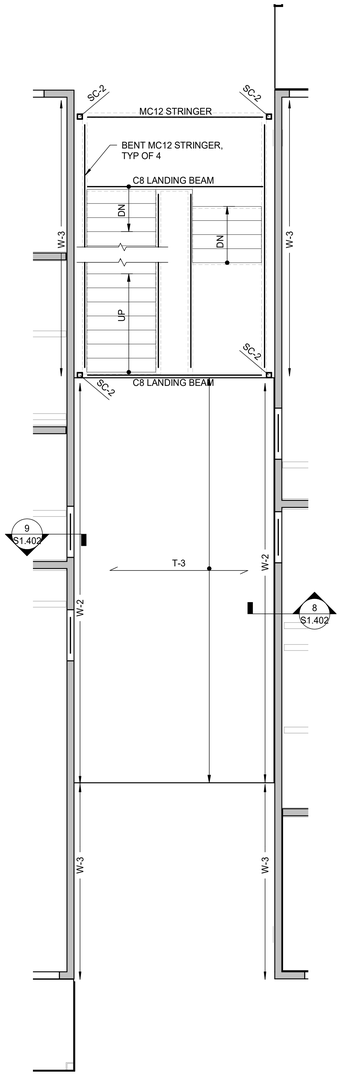
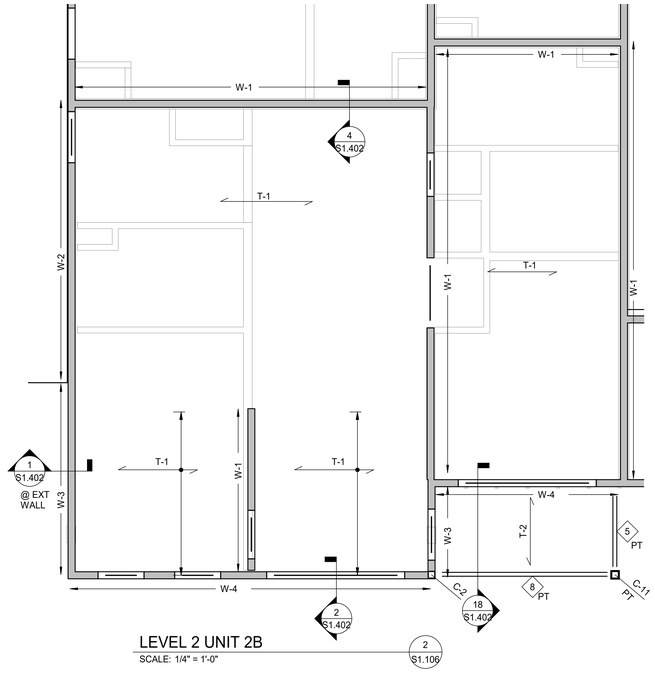


LEVEL 2 1B STAIR 4

SCALE: 1/4" = 1'-0"

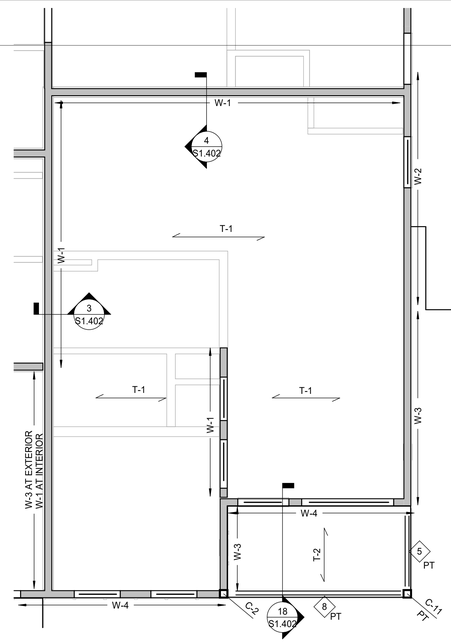
3 S1.106



LEVEL 2 UNIT 2B

SCALE: 1/4" = 1'-0"

2 S1.106



LEVEL 2 UNIT 1B

SCALE: 1/4" = 1'-0"

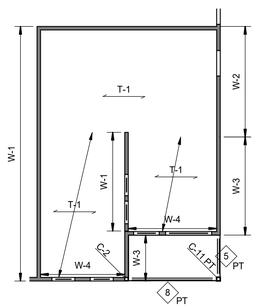
1 S1.106

LEVEL 2 UNIT FRAMING GENERAL NOTES:

NOTES:

1. TYPICAL FLOOR CONSTRUCTION SHALL BE AS FOLLOWS: UP TO 1" THICK GYPCRETE TOPPING OVER 23/32" (3/4") APA RATED STURDI-FLOOR TONGUE AND GROOVE PANELS GLUED AND SCREWED TO 18" TRUSSES OVER 2x WOOD BEARING WALLS. SEE S1.401 FOR SCHEDULE AND TRUSS LOADING INFORMATION.
2. TOP OF SHEATHING = 22.67' @ LEVEL 2.
3. SEE S1.401 FOR TYPICAL HEADERS, STUDS AT OPENINGS, BEAM AND COLUMN SCHEDULES.
4. DENOTES BEARING WALL.
5. SEE LEVEL 2 FRAMING PLAN ON S1.102 FOR SHEAR WALL LOCATIONS AND NOTES.
6. TYPICAL CONDITIONS ARE APPLICABLE EVEN IF SECTIONS ARE NOT SHOWN.

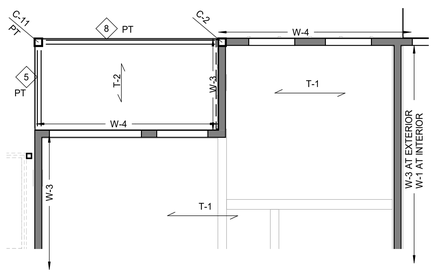
TRUSS/JOIST SHOP DRAWING SUBMITTAL SHALL BE COORDINATED WITH AND SHALL SHOW ALL BATHTUB, SHOWER AND TOILET DRAINS AND ALL MECHANICAL SHAFTS. ADJUST JOIST SPACING AND/OR ADD JOISTS AND HEADERS TO CLEAR PLUMBING & MECHANICAL.



LEVEL 2 UNIT 1X

SCALE: 1/8" = 1'-0"

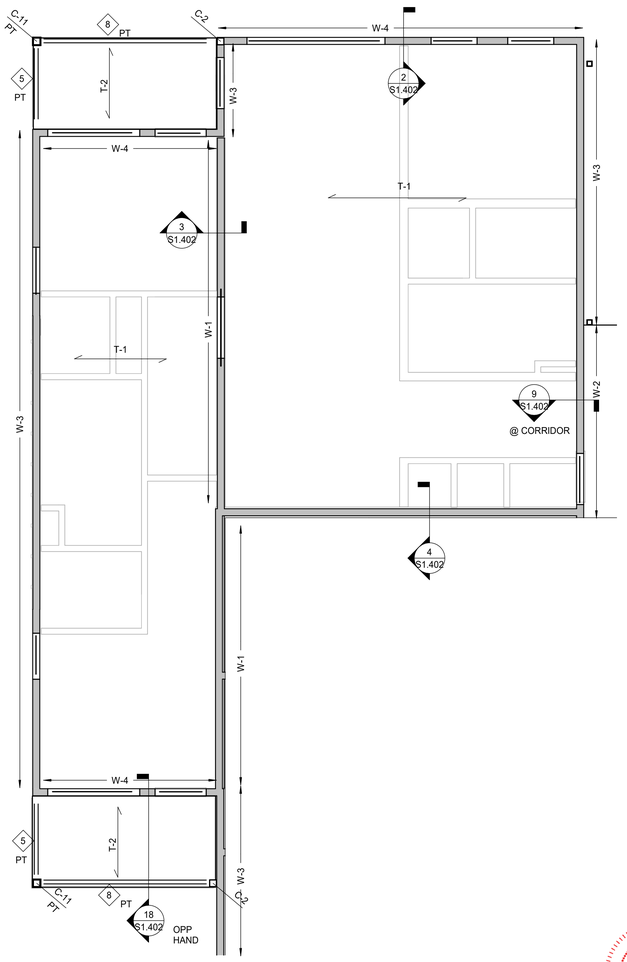
6 S1.106



LEVEL 2 UNIT 1B ALTERNATE

SCALE: 1/4" = 1'-0"

5 S1.106



LEVEL 2 UNIT 3A/3X

SCALE: 1/4" = 1'-0"

4 S1.106

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UNIT FRAMING PLANS

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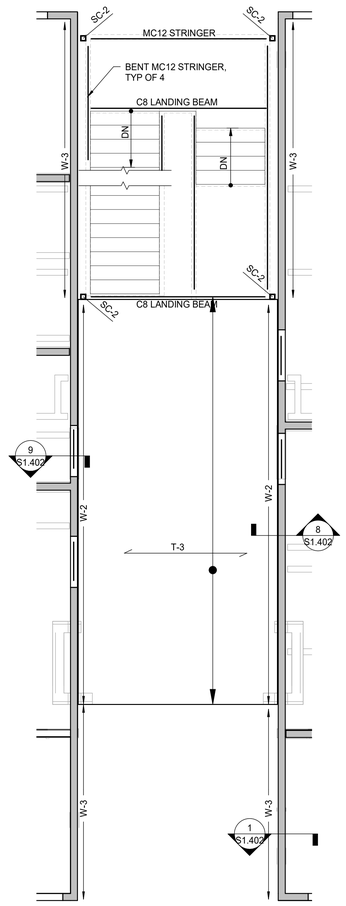
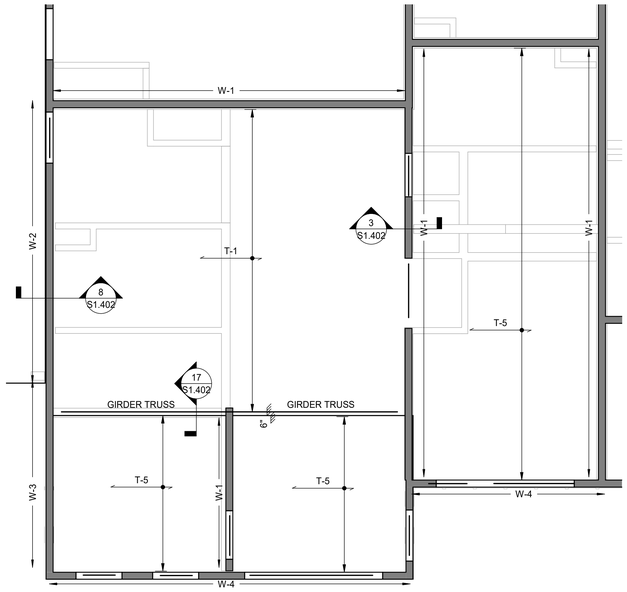
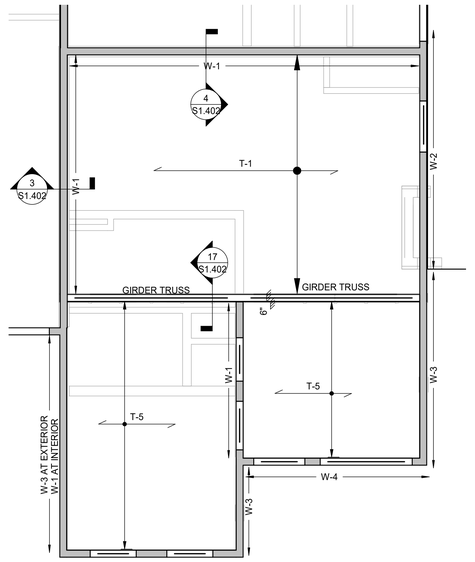


PROVIDE STAIR SHOP DRAWINGS DESIGNED BY SPECIALTY ENGINEER, SIZES ARE SHOWN FOR INTENT ONLY

LEVEL 3 STAIR
SCALE: 1/4" = 1'-0"
3 S1.107



LEVEL 3UNIT 2B
SCALE: 1/4" = 1'-0"
2 S1.107



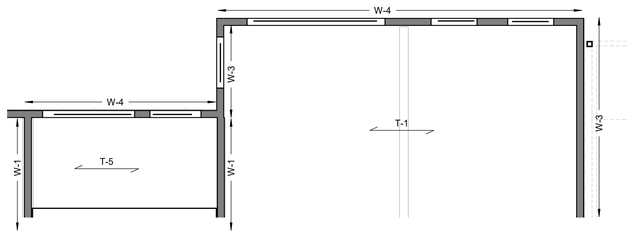
LEVEL 3 UNIT 1B
SCALE: 1/4" = 1'-0"
1 S1.107

LEVEL 3 UNIT FRAMING GENERAL NOTES:

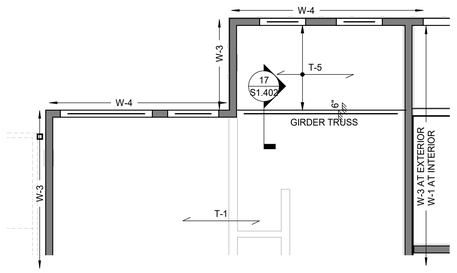
NOTES:

1. TYPICAL FLOOR CONSTRUCTION SHALL BE AS FOLLOWS: UP TO 1" THICK GYPCRETE TOPPING OVER 23/32" (3/4") APA RATED STURDI-FLOOR TONGUE AND GROOVE PANELS GLUED AND SCREWED TO 18" TRUSSES OVER 2x WOOD BEARING WALLS. SEE S1.401 FOR SCHEDULE AND TRUSS LOADING INFORMATION.
2. TOP OF SHEATHING = 33.33' @ LEVEL 3.
3. SEE S1.401 FOR TYPICAL HEADERS, STUDS AT OPENINGS, BEAM AND COLUMN SCHEDULES.
4. DENOTES BEARING WALL.
5. SEE LEVEL 3 FRAMING PLAN ON S1.103 FOR SHEAR WALL LOCATIONS AND NOTES.
6. TYPICAL CONDITIONS ARE APPLICABLE EVEN IF SECTIONS ARE NOT SHOWN.

TRUSS/JOIST SHOP DRAWING SUBMITTAL SHALL BE COORDINATED WITH AND SHALL SHOW ALL BATHTUB, SHOWER AND TOILET DRAINS AND ALL MECHANICAL SHAFTS. ADJUST JOIST SPACING AND/OR ADD JOISTS AND HEADERS TO CLEAR PLUMBING & MECHANICAL.



LEVEL 3 PARTIAL BALCONY PLAN
SCALE: 1/4" = 1'-0"
6 S1.107



LEVEL 3 PARTIAL BALCONY PLAN
SCALE: 1/4" = 1'-0"
5 S1.107

LEVEL 3 UNIT 3A
SCALE: 1/4" = 1'-0"
4 S1.107

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S1.107
UNIT FRAMING PLANS
12/14/20

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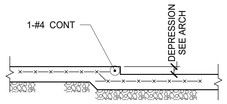
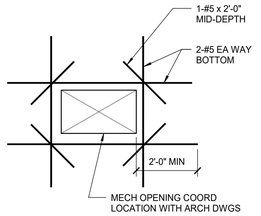


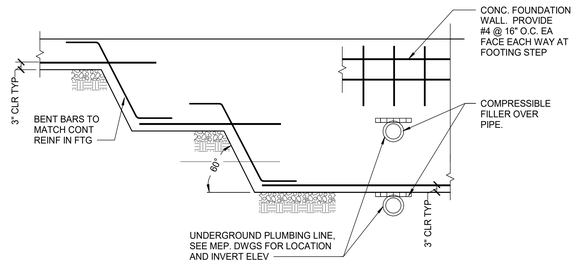
TYPICAL FLOOR DEPRESSION

SECTION 4 S1.201
SCALE: 1/2" = 1'-0"



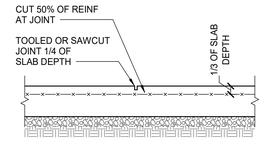
TYPICAL OPENING REINFORCEMENT @ SLAB ON GRADE BLOCK-OUTS

SECTION 3 S1.201
SCALE: 1/2" = 1'-0"

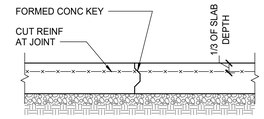


TYPICAL FOOTING STEP (F.S.)

SECTION 2 S1.201
SCALE: 1/2" = 1'-0"



TYPICAL SLAB CONTROL JOINT

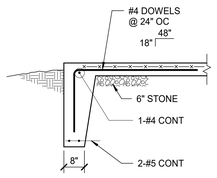


TYPICAL SLAB CONSTRUCTION JOINT

SECTION 1 S1.201
SCALE: 1/2" = 1'-0"

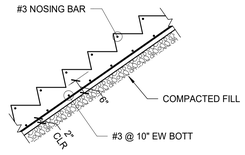
FOOTING SCHEDULE			
MARK	SIZE	REINFORCING	REMARKS
F1	3'-0"x3'-0"x12"	3-#4 BOTT. EW	
F2	4'-0"x4'-0"x12"	4-#4 BOTT. EW	
F3	3'-0"x3'-0"x24"	3-#4 BOTT. EW	

PIER SCHEDULE				
		REINFORCING		
MARK	SIZE	VERTICAL	TIES	REMARKS
P1	12"x12"	4-#5 VERT.	#3 @ 6" O.C.	CONC. PIER



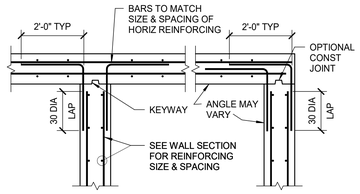
TURN DOWN SLAB EDGE

SECTION 9 S1.201
SCALE: 1/2" = 1'-0"



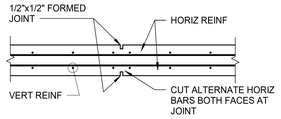
TYPICAL CONCRETE STAIRS

SECTION 8 S1.201
SCALE: 1/2" = 1'-0"

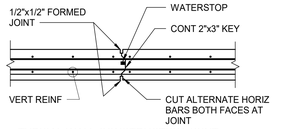


TYPICAL REINFORCING AT WALL INTERSECTIONS

SECTION 7 S1.201
SCALE: 1/2" = 1'-0"



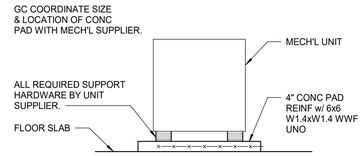
TYPICAL WALL CONTROL JOINT



TYPICAL WALL CONSTRUCTION JOINT

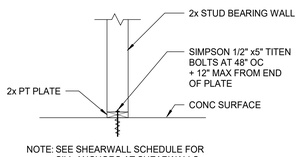
NOTES:
1. PROVIDE CONTROL OR CONSTRUCTION JOINT AT 30'-0" MAX SPACING.
2. LOCATE FIRST JOINT NO FURTHER THAN 15'-0" FROM CORNER.

SECTION 6 S1.201
SCALE: 1/2" = 1'-0"



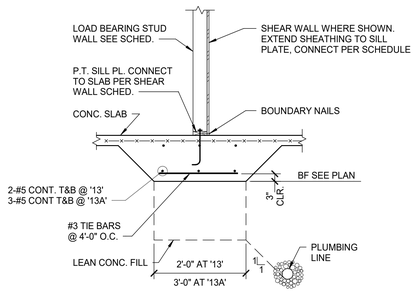
TYPICAL HOUSEKEEPING PAD

SECTION 5 S1.201
SCALE: 1/2" = 1'-0"

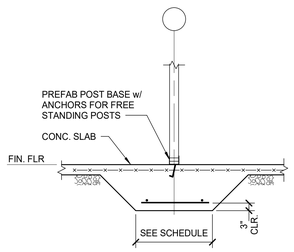


TYPICAL WOOD PLATE TO CONC ANCHORAGE
(EXCEPT SHEAR WALLS)

SECTION 14 S1.201
SCALE: 3/4" = 1'-0"

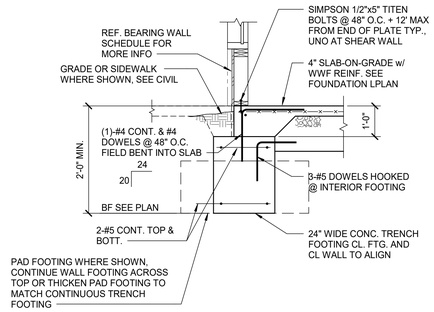


SECTION 13 13A S1.201 S1.201
SCALE: 1/2" = 1'-0"

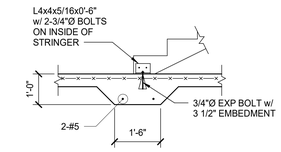


INTERIOR WOOD COLUMN FOOTING

SECTION 12 S1.201
SCALE: 1/2" = 1'-0"

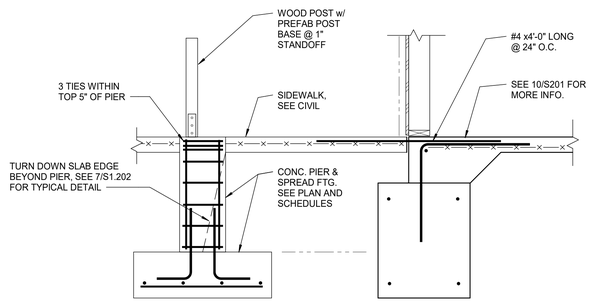


SECTION 11 S1.201
SCALE: 1/2" = 1'-0"

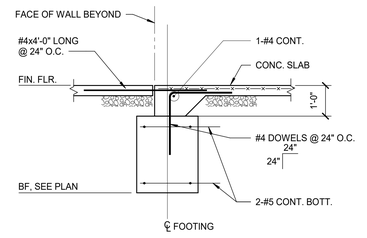


THICKENED SLAB @ STRINGER

SECTION 10 S1.201
SCALE: 1/2" = 1'-0"



SECTION 16 S1.201
SCALE: 3/4" = 1'-0"



TYPICAL SECTION @ DOOR ENTRY

SECTION 15 S1.201
SCALE: 1/2" = 1'-0"

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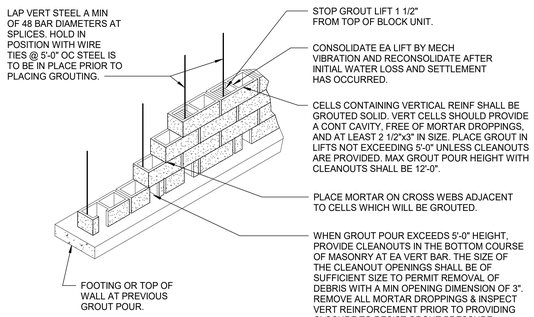
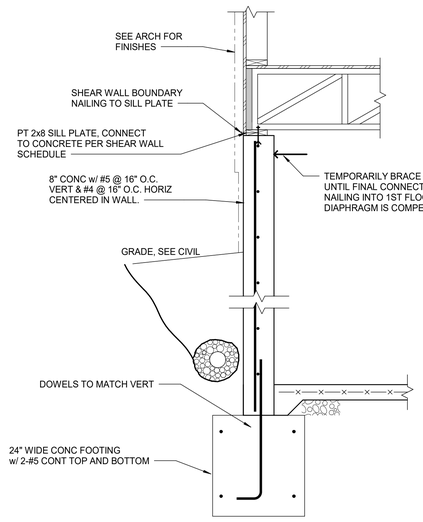
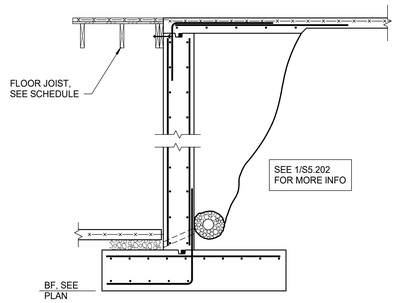


TYPICAL REINFORCED MASONRY CONSTRUCTION

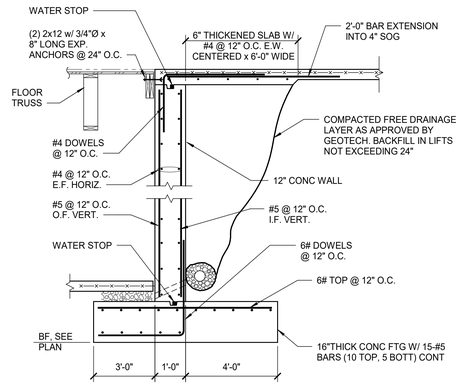
SECTION 4 S1.202
SCALE: 3/4" = 1'-0"



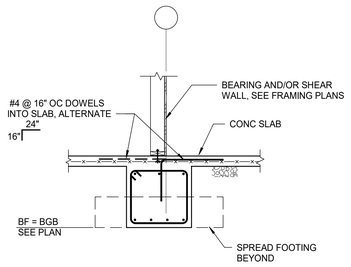
SECTION 3 S1.202
SCALE: 3/4" = 1'-0"



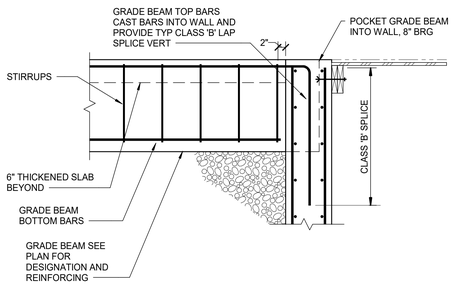
SECTION 2 S1.202
SCALE: 1/2" = 1'-0"



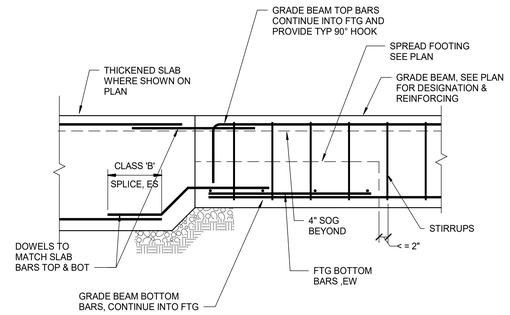
SECTION 1 S1.202
SCALE: 1/2" = 1'-0"



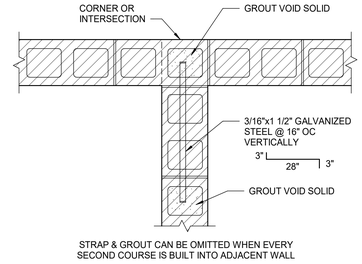
SECTION 8 S1.202
SCALE: 1/2" = 1'-0"



SECTION 7 S1.202
SCALE: 3/4" = 1'-0"

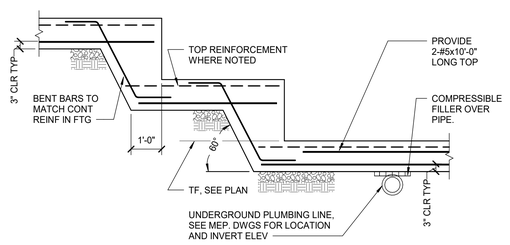


SECTION 6 S1.202
SCALE: 3/4" = 1'-0"



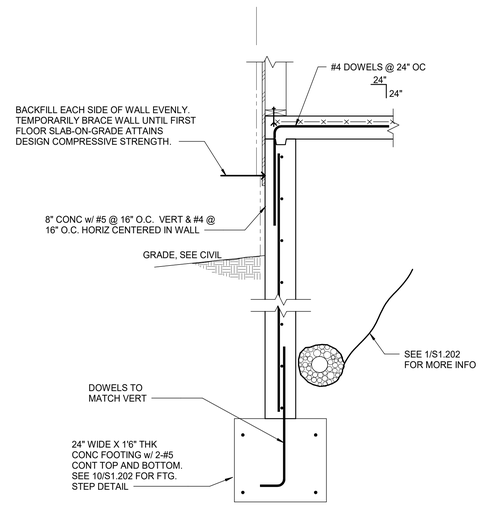
MASONRY CORNER OR INTERSECTION DETAIL

SECTION 5 S1.202
SCALE: 3/4" = 1'-0"



TYPICAL FOOTING STEP (F.S.)

SECTION 10 S1.202
SCALE: 1/2" = 1'-0"



SECTION 9 S1.202
SCALE: 3/4" = 1'-0"

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FLOOR JOIST/TRUSS SCHEDULE

MARK	SIZE	UNIFORM LOADS			DEFLECTION LIMITS			REMARKS
		DL PSF (TOP CHORD)	LL PSF (TOP CHORD)	DL PSF (BOT. CHORD)	LL	TL	△MAX. (INCHES)	
T-1	18" DEEP WOOD TRUSS @ 24" O.C. MAX.	15	40	7	L/480	L/240	1"	FLOOR/UNIT
T-2	PT 2x10 @ 16" OC	-	-	-	-	-	-	EXTERIOR PRIVATE BALCONY
T-3	PT 2x12 @ 12" OC	-	-	-	-	-	-	BREEZEWAY
T-4	METAL PLATED OPEN WEB WOOD TRUSSES @ 24" OC	13	20 MIN.	7	L/360	L/240	1"	PITCHED ROOF AREAS
T-5	12" DEEP WOOD TRUSS @ 24" O.C. MAX.	13	60	7	L/360	L/240	1"	LOW ROOF BALCONY/MECH AREAS
T-6	12" DEEP WOOD TRUSS @ 24" O.C. MAX.	13	20 MIN.	7	L/360	L/240	1"	FLAT ROOF
T-7	22" DEEP WOOD TRUSS @ 24" O.C. MAX.	15	40	7	L/480	L/240	1"	FLOOR/ UNIT

1. TRUSS SPACING SHALL BE DETERMINED BY TRUSS MANUF. 24" OC MECH TO MEET THE DESIGNATED LOADING AND DEFLECTION CRITERIA.
2. REFER TO FRAMING PLANS AND SECTIONS FOR CONCENTRATED OR OTHER ADDITIONAL LOADS TO TRUSSES.

HEADER SCHEDULE

1	2-2x6
2	3-2x6
3	2-2x8
4	3-2x8
5	2-2x10
6	3-2x10
7	2-2x12
8	3-2x12
9	2-1 3/4"x9 1/2" LVL
10	2-1 3/4"x11 7/8" LVL
11	2-1 3/4"x14" LVL
12	2-1 3/4"x16" LVL
13	2-1 3/4"x18" LVL
14	3-1 3/4"x9 1/2" LVL
15	3-1 3/4"x11 7/8" LVL
16	3-1 3/4"x14" LVL
17	3-1 3/4"x16" LVL
18	3-1 3/4"x11 7/8" LVL w/ (2)-3/8" THK. STL. PL.

NOTE:
PROVIDE DOUBLE STUD @ END OF EACH HEADER UNO

COLUMN SCHEDULE

C-1	2-2x4
C-2	2-2x6
C-3	3-2x4
C-4	3-2x6
C-5	4-2x4
C-6	4-2x6
C-7	5-2x4
C-8	5-2x6
C-9	4x4 POST
C-10	4x6 POST
C-11	6x6 SP #2 POST

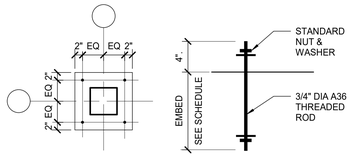
BEARING WALL SCHEDULE

		EXTERIOR NON-LOAD BEARING		EXTERIOR		BREEZEWAY		UNIT SEPARATION/ UNIT INTERIOR		MARK / FLOOR
		W-4a	W-4	W-3a	W-3	W-2a	W-2	W-1a	W-1	
										ROOF
		(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 24" OC	(1)-2x6 @ 24" OC	LEVEL 3
		(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 24" OC	(1)-2x6 @ 24" OC	LEVEL 2
						(2)-2x6 @ 24" OC	(2)-2x6 @ 24" OC	(2)-2x6 @ 24" OC	(2)-2x6 @ 24" OC	LEVEL 1
						(2)-2x6 @ 24" OC		(3)-2x6 @ 24" OC		LOWER LEVEL

BEARING WALL SCHEDULE NOTES:
1. PROVIDE CRIPPLE BLOCKING AS PER TYPICAL DETAILS.
2. SEE TYPICAL SHEARWALL DETAILS FOR ADD'L REQUIREMENTS.
3. REFER TO ARCHITECTURAL DRAWINGS FOR WALL SECTIONS AND LOCATION OF DRAFT STOP.
4. ALL BEARING WALLS SHALL HAVE MID-HEIGHT BLOCKING.

STEEL COLUMN SCHEDULE

				SC-1	MARK / FLOOR
					ROOF
			HSS4X4X1/4"		LEVEL 3
					LEVEL 2
					LEVEL 1
				HSS3.1/2X3.1/2X5/16"	FOUNDATION
			12X12X3/4"	12X12X3/4"	BASE PLATE
			4-3/4" DIA. X 8" EMBED	4-3/4" DIA. X 8" EMBED	ANCHOR BOLTS
					REMARKS



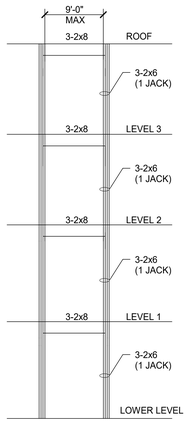
TYP BASE PLATE

TYPICAL ANCHOR BOLT

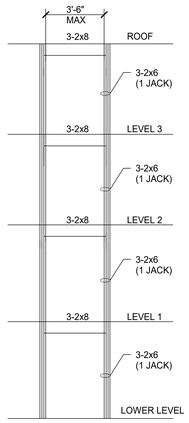
ANCHOR BOLT ARRANGEMENT

WOOD POST SCHEDULE

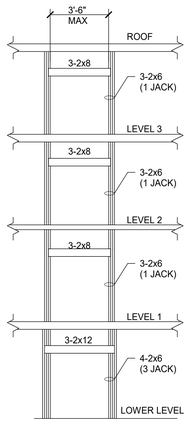
	P2	P1	MARK / FLOOR
			ROOF
			LEVEL 3
			LEVEL 2
			LEVEL 1
			LOWER LEVEL



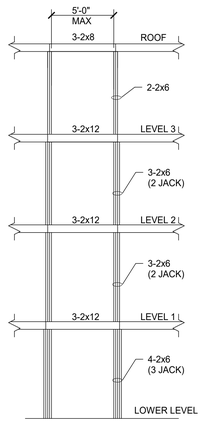
TYPICAL HEADERS/COLUMNS W-4/ W-4a



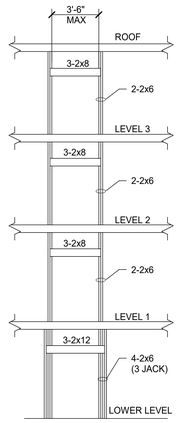
TYPICAL HEADERS/COLUMNS W-3/ W-3a



TYPICAL HEADERS/COLUMNS W-2/ W-2a



TYPICAL FLUSH HEADERS W-1/W-1a



TYPICAL DROPPED HEADERS/COLUMNS W-1/ W-1a

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TERRACES AT HIGH MOUNTAIN
4130 HIGH MOUNTAIN ROAD NE
HUNTSVILLE, AL 35811
06/11/2021

S1.401
WOOD FRAMING SCHEDULES
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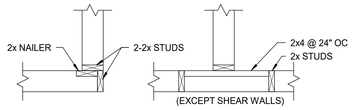
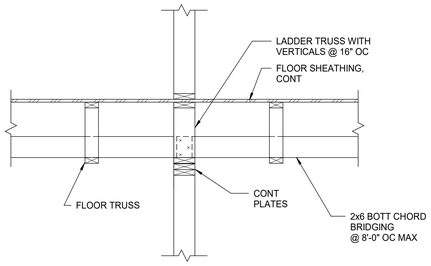


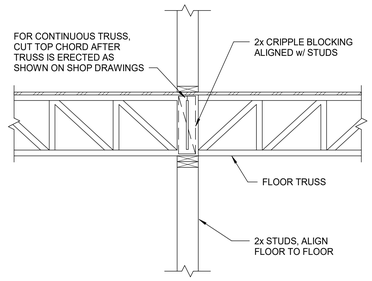
TYPICAL WALL CORNER AND INTERSECTION
(INTERIOR AND EXTERIOR - TYPE V)

SECTION 5 S1.402
SCALE: 3/4" = 1'-0"



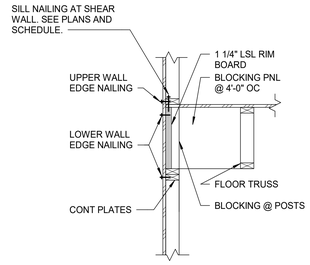
TYPICAL UNIT SEPARATION WALL
PARALLEL TRUSS - SINGLE WALL

SECTION 4 S1.402
SCALE: 3/4" = 1'-0"



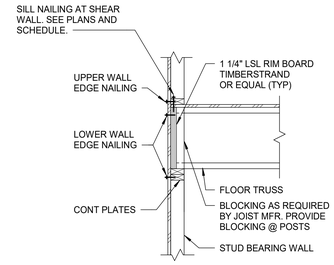
TYPICAL INTERIOR BEARING WALL

SECTION 3 S1.402
SCALE: 3/4" = 1'-0"



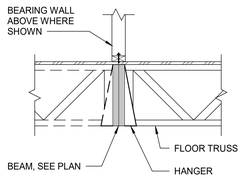
WALL PARALLEL TO JOIST

SECTION 2 S1.402
SCALE: 3/4" = 1'-0"



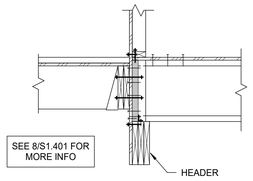
TYPICAL EXTERIOR BRG. WALL

SECTION 1 S1.402
SCALE: 3/4" = 1'-0"

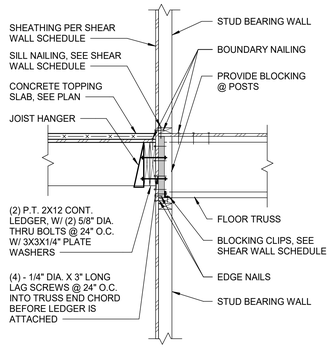


TYPICAL JOIST BRG. ON BEAM

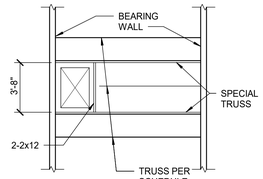
SECTION 10 S1.402
SCALE: 3/4" = 1'-0"



SECTION 9 S1.402
SCALE: 3/4" = 1'-0"



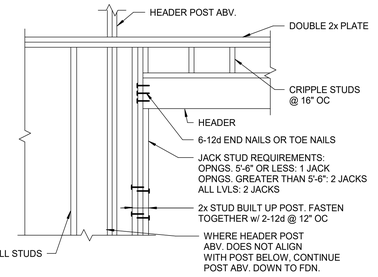
SECTION 8 S1.402
SCALE: 3/4" = 1'-0"



NOTE: DETAIL APPLIES FOR SMALL OPENINGS UP TO 3'-8" WIDE

TYPICAL SHAFT FRAMING AT TRUSS FLOOR

SECTION 7 S1.402
SCALE N.T.S.



TYPICAL DROPPED HEADER AT OPENING

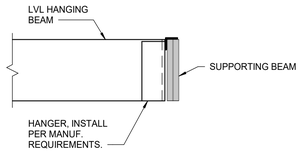
SECTION 6 S1.402
SCALE: 3/4" = 1'-0"

HANGING BEAM SIZE	MAX. LOAD (LB)	REQUIRED * HANGER
2-12"	7,900	SIMPSON HGLT 3.511
2-14"	9,310	SIMPSON HGLT 3.514
2-16"	10,600	SIMPSON HGLT 3.516
2-18"	10,950	SIMPSON HGLT 3.518

* OR EQUAL

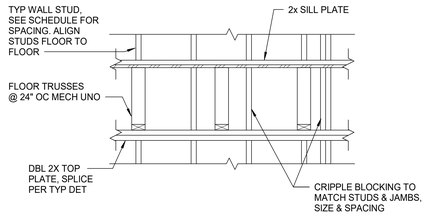
HANGING BEAM SIZE	MAX. LOAD (LB)	REQUIRED * HANGER
3-12"	11,850	SIMPSON HGLT 5.511
3-14"	12,750	SIMPSON HGLT 5.514
3-16"	12,750	SIMPSON HGLT 5.516
3-18"	12,750	SIMPSON HGLT 5.518

* OR EQUAL



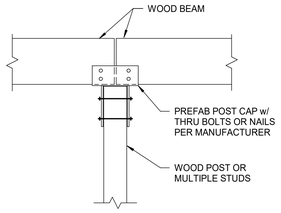
TYPICAL LVL BEAM TO BEAM CONNECTION

SECTION 15 S1.402
SCALE: 3/4" = 1'-0"



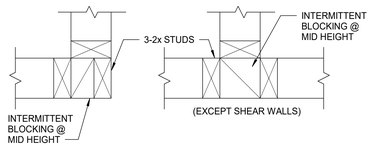
TYPICAL WALL CRIPPLE BLOCKING
AT BEARING WALLS

SECTION 14 S1.402
SCALE: 3/4" = 1'-0"



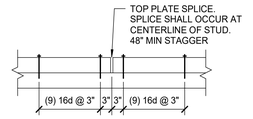
TYPICAL WOOD BEAM TO WOOD POST CONNECTION

SECTION 13 S1.402
SCALE: 3/4" = 1'-0"



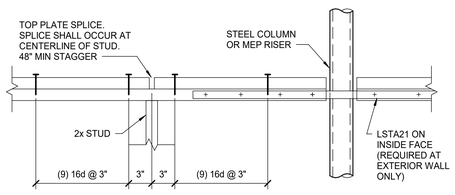
TYP CORNER AND INTERSECTION STUD WALL

SECTION 12 S1.402
SCALE: 3/4" = 1'-0"



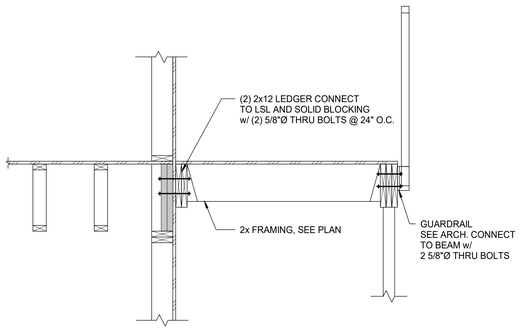
TYPICAL TOP PLATE SPLICE

SECTION 11 S1.402
SCALE: 3/4" = 1'-0"

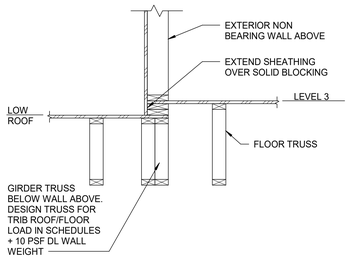


TYPICAL TOP PLATE SPLICE AT ALL WALLS AND
PLATE INTERRUPTION STRAP AT EXTERIOR WALL

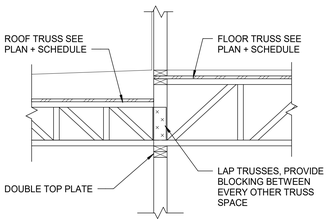
SECTION 19 S1.402
SCALE: 1 1/2" = 1'-0"



SECTION 18 S1.402
SCALE: 3/4" = 1'-0"



SECTION 17 S1.402
SCALE: 3/4" = 1'-0"



SECTION 16 S1.402
SCALE: 3/4" = 1'-0"

ALABAMA LICENSED No. 22545 PROFESSIONAL ENGINEER

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TERRACES AT HIGH MOUNTAIN
4130 HIGH MOUNTAIN ROAD NE
HUNTSVILLE, AL 35811
06/11/2021

S1.402
WOOD FRAMING SECTIONS
12/15/20

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SHEARWALL SCHEDULE

MARK (WALL TYPE)	LEVEL	SHEATHING TYPE	NAIL SIZE / PENETRATION	NAIL SPACING AT EDGES	BLOCKING	MIN END CHORD	SILL ANCHORS	CHORD ANCHORS	REMARKS
SW-1	3RD TO ROOF	7/16" OSB (EXT. FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	2-2X6	(1) ROWS 10d NAILS @ 6" O.C. / (1) ROWS 10d NAILS @ 6" O.C.	HDU2-SDS2.5 w/5/8" ØANCHOR	
SW-1	2ND TO 3RD	7/16" OSB (EXT. FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	2-2X6	(1) ROWS 10d NAILS @ 6" O.C. / (1) ROWS 10d NAILS @ 6" O.C.	NA	
SW-1	1ST TO 2ND	7/16" OSB (EXT. FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	3-2X6	(1) ROWS 10d NAILS @ 4" O.C. / (1) ROWS 10d NAILS @ 4" O.C.	NA	NOTE 1 & NOTE 3
SW-1	LOWER LEVEL	7/16" OSB (EXT. FACE)	8d COMMON / 1 3/8"	4"	BLOCKED	4-2X6	(1) ROWS 1/4" DIA WOOD SCREWS @ 4" O.C. / 1/2" DIA. A.B. @ 24" O.C. THRU PT SILL	NA	

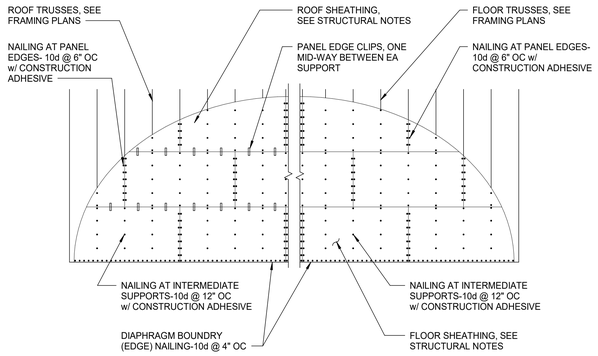
MARK (WALL TYPE)	LEVEL	SHEATHING TYPE	NAIL SIZE / PENETRATION	NAIL SPACING AT EDGES	BLOCKING	END CHORD	SILL ANCHORS	CHORD ANCHORS	REMARKS
SW-2	3RD TO ROOF								
SW-2	2ND TO 3RD	7/16" OSB (EXT. FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	2-2X6	(1) ROWS 10d NAILS @ 6" O.C. / (1) ROWS 10d NAILS @ 6" O.C.	HDU2-SDS2.5 w/5/8" ØANCHOR	
SW-2	1ST TO 2ND	7/16" OSB (EXT. FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	3-2X6	(1) ROWS 10d NAILS @ 4" O.C. / (1) ROWS 10d NAILS @ 4" O.C.	HDU2-SDS2.5 w/5/8" ØANCHOR	NOTE 1 & NOTE 3
SW-2	LOWER LEVEL	7/16" OSB (EXT. FACE)	8d COMMON / 1 3/8"	4"	BLOCKED	4-2X6	(1) ROWS 1/4" DIA WOOD SCREWS @ 4" O.C. / 1/2" DIA. A.B. @ 24" O.C. THRU PT SILL	HDU2-SDS2.5 w/SB5/8x24	

MARK (WALL TYPE)	LEVEL	SHEATHING TYPE	NAIL SIZE / PENETRATION	NAIL SPACING AT EDGES	BLOCKING	END CHORD	SILL ANCHORS	CHORD ANCHORS	REMARKS
SW-3	3RD TO ROOF								
SW-3	2ND TO 3RD	7/16" OSB (1 FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	2-2X6	(1) ROWS 10d NAILS @ 6" O.C. / (1) ROWS 10d NAILS @ 6" O.C.	HDU2-SDS2.5 w/5/8" ØANCHOR	
SW-3	1ST TO 2ND	7/16" OSB (1 FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	2-2X6	(1) ROWS 10d NAILS @ 4" O.C. / (1) ROWS 10d NAILS @ 4" O.C.	HDU5-SDS2.5 w/5/8" ØANCHOR	NOTE 1 & NOTE 3 - SB5/8X24 WHERE TERMINATES @ 1ST
SW-3	LOWER LEVEL	7/16" OSB (1 FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	3-2X6	(1) ROWS 10d NAILS @ 4" O.C. / 1/2" DIA. A.B. @ 32" O.C. THRU PT SILL	HDU8-SDS2.5 w/SB7/8x24	

MARK (WALL TYPE)	LEVEL	SHEATHING TYPE	NAIL SIZE / PENETRATION	NAIL SPACING AT EDGES	BLOCKING	END CHORD	SILL ANCHORS	CHORD ANCHORS	REMARKS
SW-4	3RD TO ROOF								
SW-4	2ND TO 3RD	7/16" OSB (1 FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	2-2X6	(1) ROWS 10d NAILS @ 6" O.C. / (1) ROWS 10d NAILS @ 6" O.C.	HDU2-SDS2.5 w/5/8" ØANCHOR	
SW-4	1ST TO 2ND	7/16" OSB (1 FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	2-2X6	(1) ROWS 10d NAILS @ 4" O.C. / 1/2" DIA. A.B. @ 32" O.C. THRU PT SILL	HDU2-SDS2.5 w/SB5/8x24	
SW-4	LOWER LEVEL								

MARK (WALL TYPE)	LEVEL	SHEATHING TYPE	NAIL SIZE / PENETRATION	NAIL SPACING AT EDGES	BLOCKING	END CHORD	SILL ANCHORS	CHORD ANCHORS	REMARKS
SW-5	3RD TO ROOF								
SW-5	2ND TO 3RD	7/16" OSB (1 FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	2-2X6	(2) ROWS 16d NAILS @ 6" O.C. / (2) ROWS 16d NAILS @ 6" O.C.	HDU4-SDS2.5 w/5/8" ØANCHOR	NOTE 4
SW-5	1ST TO 2ND	7/16" OSB (1 FACE)	8d COMMON / 1 3/8"	4"	BLOCKED	2-2X6	(2) ROWS 16d NAILS @ 6" O.C. / (2) ROWS 16d NAILS @ 6" O.C.	HDU8-SDS2.5 w/7/8" ØANCHOR	NOTE 1, 3 AND 4 - SB7/8X24 WHERE TERMINATES @ 1ST
SW-5	LOWER LEVEL	7/16" OSB (1 FACE)	8d COMMON / 1 3/8"	4"	BLOCKED	6X6 POST	(2) ROWS 16d NAILS @ 4" O.C. / 1/2" DIA. A.B. w/ 4.5" SQ PL WASHERS @ 24" O.C. THRU PT SILL	HDU11-SDS2.5 w/SB1x30	NOTE 4

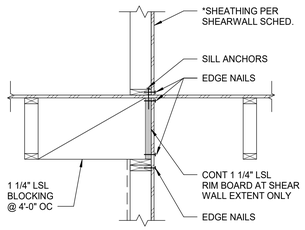
NOTES:
1. ALL SHEAR WALLS END WHERE SHOWN ON FRAMING PLANS, SEE S1.101-S1.104.
2. WHERE SHEAR WALLS TERMINATE ABOVE FOUNDATION, PROVIDE SILL NAILING PER SHEAR WALL SCHEDULE.
3. WHERE SHEAR WALLS TERMINATE AT FOUNDATIONS, PROVIDE ANCHOR BOLTS THRU PT SILL AS SCHEDULED ON LOWEST LEVEL.
4. SHEAR WALL END CHORDS AND HOLDDOWNS ONLY REQUIRED AT FAR SHEAR WALL ENDS.



ROOF DIAPHRAGM NAILING PATTERN

FLOOR DIAPHRAGM NAILING PATTERN (WALL NAILING PATTER SIMILIAR - SEE SHEAR WALL SCHEDULE)

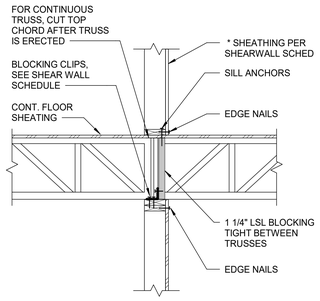
SECTION 1 S1.403
SCALE: 3/4" = 1'-0"



NOTES:
1. "*" -WHERE SHEATHING IS INDICATED ON 1 FACE IN SCHEDULE, PLACE ON SAME FACE AS LSL RIM.
2. SEE DETAILS ON S1.402 FOR ADD'L INFORMATION.

TYPICAL SHEARWALL AT UNIT INTERIOR
WALL PARALLEL TO TRUSS

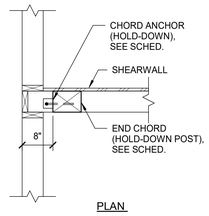
SECTION 4 S1.403
SCALE: 3/4" = 1'-0"



NOTES:
1. "*" WHERE SHEATHING IS INDICATED ON 1 FACE IN SCHEDULE, PLACE ON SAME FACE AS LSL BLOCKING

TYPICAL SHEARWALL AT UNIT INTERIOR
PERPENDIULAR TRUSS

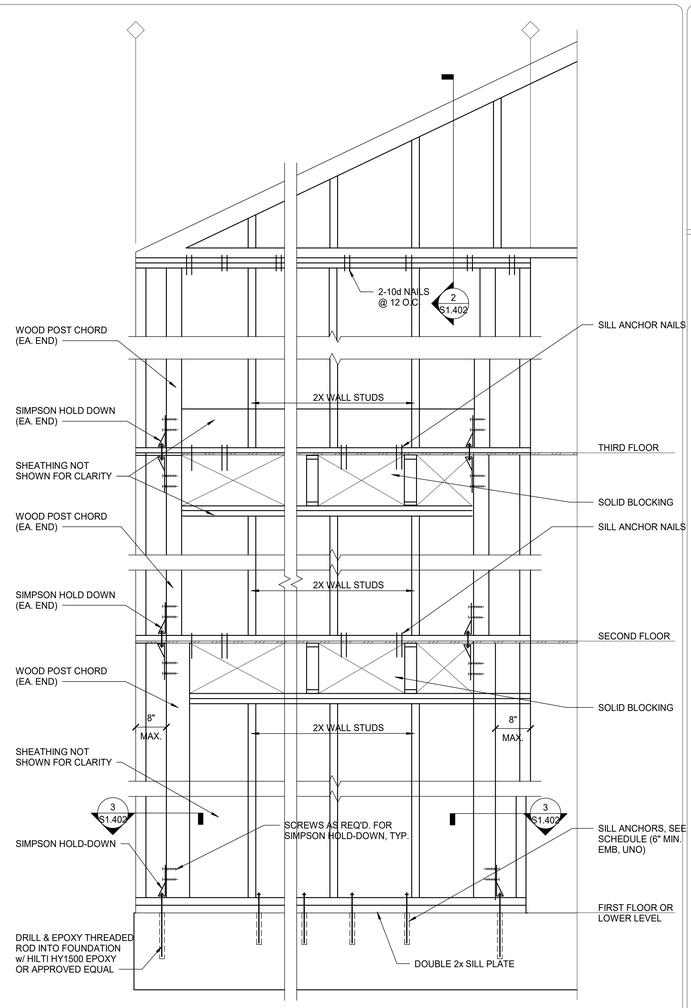
SECTION 3 S1.403
SCALE: 3/4" = 1'-0"



PLAN

TYPICAL DETAIL AT SHEARWALL

SECTION 2 S1.403
SCALE: 3/4" = 1'-0"



TYPICAL SHEAR WALL DETAIL

TYPICAL SHEARWALL ELEVATION AND SCHEDULE

NOTES:

1. SHEAR WALLS ARE INDICATED THUS ON PLAN. 'SW-#'.
2. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE DIAGONAL BRACING TO ENSURE LATERAL STABILITY OF STUD WALLS, BY USE OF DIAGONAL METAL STRAPS OR OTHER MEANS, PRIOR TO INSTALLATION OF SHEAR WALL SHEATHING.
3. INTERIOR SHEAR WALL SHEATHING SHALL BE INSTALLED AND FASTENED AT SILL PLATE PRIOR TO PLACING OF GYPCRETE.
4. ALL SHEAR WALLS SHALL HAVE A MINIMUM END CHORD AS NOTED IN THE SHEAR WALL SCHEDULE.
5. SCHEDULED SHEARWALL SHEATHING SHALL BE FASTENED DIRECTLY IN CONTACT WITH THE WALL STUDS. IT SHALL NOT BE PERMITTED TO PLACE RESILIENT CHANNELS NOR ANY OTHER MATRIAL IN BETWEEN THE SCHEDULED SHEATHING AND THE STUDS.
6. APPLY PANELS WITH LONG DIMENSION ACROSS STUDS.

SECTION A S1.403
SCALE: 3/4" = 1'-0"

REVISIONS
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TERRACES AT HIGH MOUNTAIN
4130 HIGH MOUNTAIN ROAD NE
HUNTSVILLE, AL 35811
06/11/2021

S1.403
WOOD SHEARWALL SCHEDULE & DETAILS
12/15/20

3 NORTH

ALABAMA LICENSED No. 22545 PROFESSIONAL ENGINEER THOMAS A. BOURGEOIS

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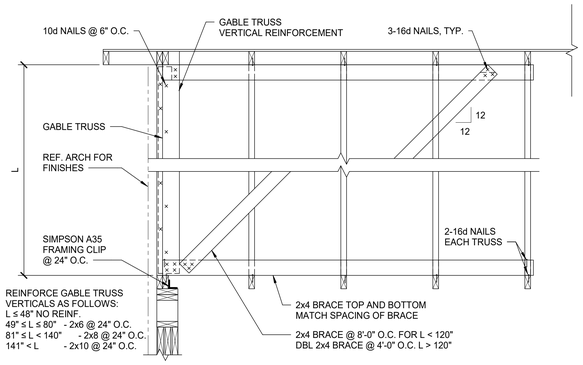
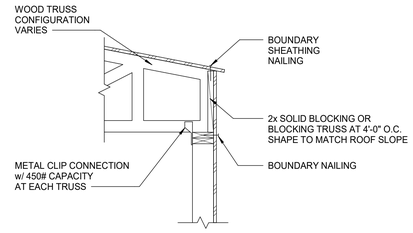
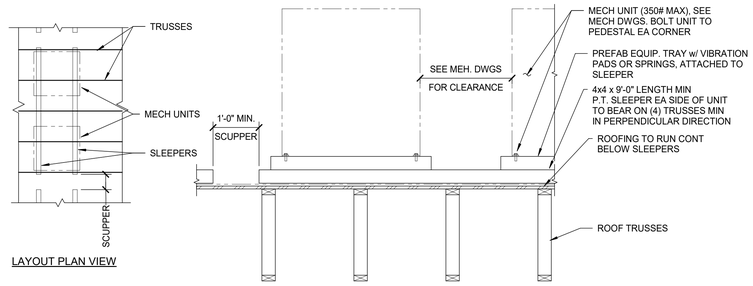


TYPICAL GABLE TRUSS REINFORCEMENT

SECTION 3 S1.501
SCALE: 3/4" = 1'-0"



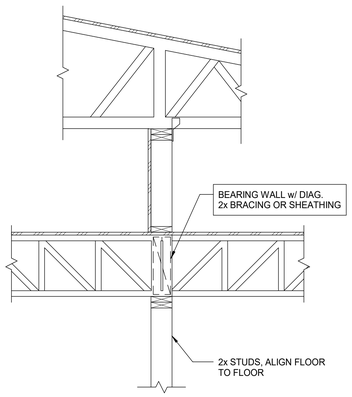
SECTION 2 S1.501
SCALE: 3/4" = 1'-0"



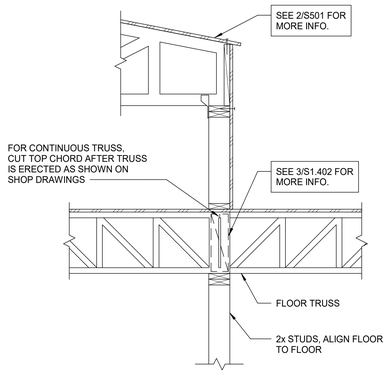
LAYOUT PLAN VIEW

TYPICAL AC UNIT SUPPORT

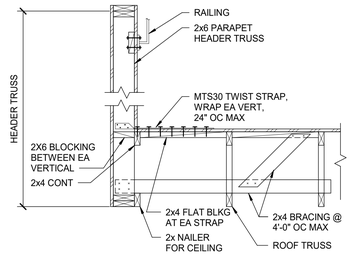
SECTION 1 S1.501
SCALE: 3/4" = 1'-0"



SECTION 7 S1.501
SCALE: 3/4" = 1'-0"

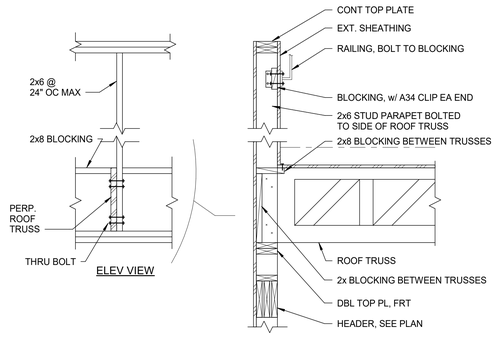


SECTION 6 S1.501
SCALE: 3/4" = 1'-0"



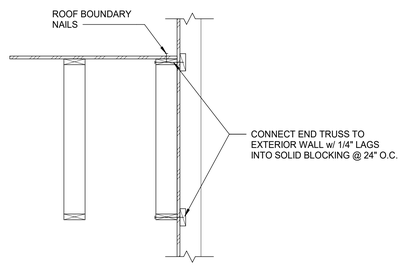
TYPICAL PARAPET DETAIL AT WINDOW HEADER
PARALLEL TRUSS

SECTION 5 S1.501
SCALE: 3/4" = 1'-0"

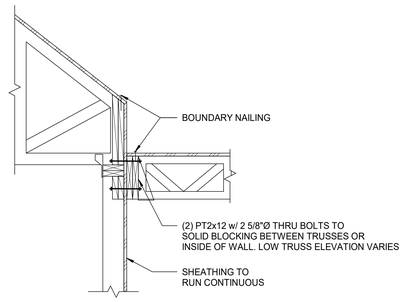


TYPICAL PARAPET DETAIL
PERPENDICULAR TRUSS

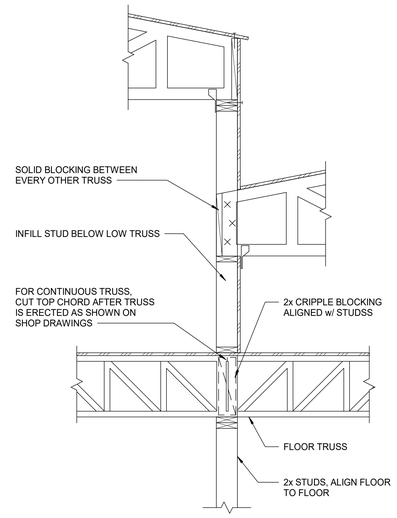
SECTION 4 S1.501
SCALE: 3/4" = 1'-0"



SECTION 10 S1.501
SCALE: 3/4" = 1'-0"



SECTION 9 S1.501
SCALE: 3/4" = 1'-0"



SECTION 8 S1.501
SCALE: 3/4" = 1'-0"

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TERRACES AT HIGH MOUNTAIN
4130 HIGH MOUNTAIN ROAD NE
HUNTSVILLE, AL 35811
06/11/2021

S1.501
WOOD ROOF FRAMING SECTIONS
12/15/20

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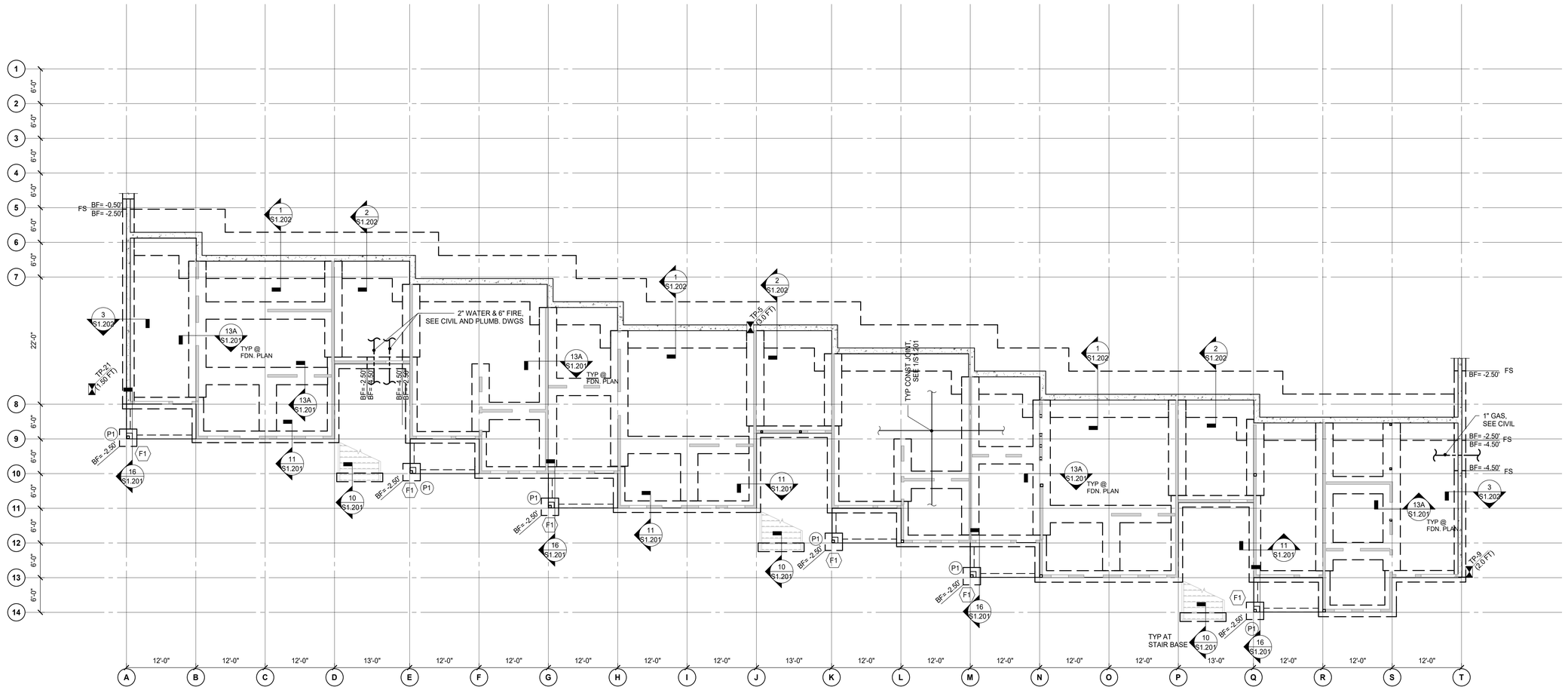

FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. REFERENCE BUILDING A1 DRAWINGS FOR BUILDING STRUCTURAL NOTES, SPECIAL INSPECTIONS, TYPICAL CONSTRUCTION, SECTIONS, DETAILS AND SCHEDULES.
2. TOP OF CONCRETE ELEVATION = 0.00' REFERENCE (ACTUAL ELEVATION= 1031.10')
3. REFER TO S1.100 ON BUILDING A1 FOR BALANCE OF PLAN NOTES NOT SHOWN.

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S2.100
FOUNDATION PLAN

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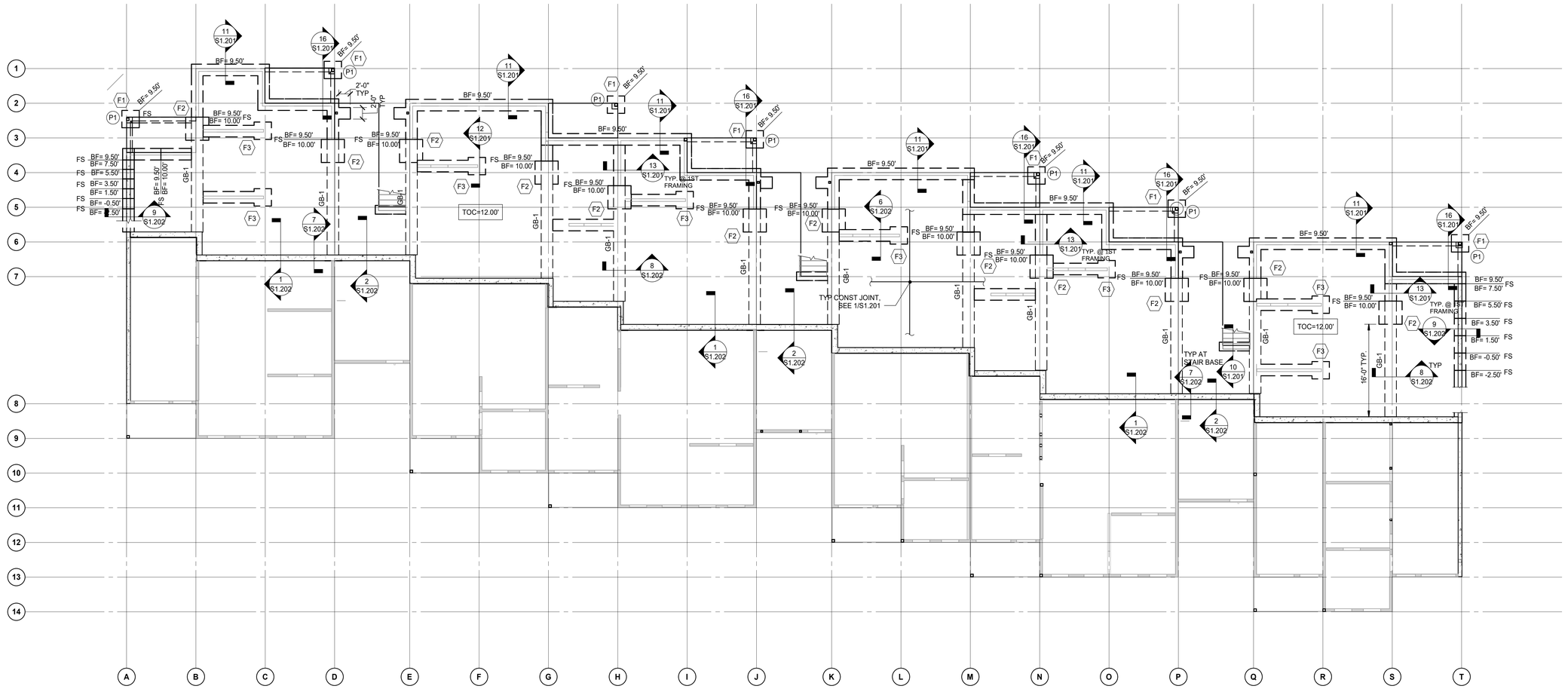

LEVEL 1 FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. SEE S2.100 FOR TYPICAL FOUNDATION NOTES.
2. TOP OF CONCRETE ELEVATION = 12.00' (REFERENCE EL = 1043.10')

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S2.101
LEVEL 1 FRAMING PLAN

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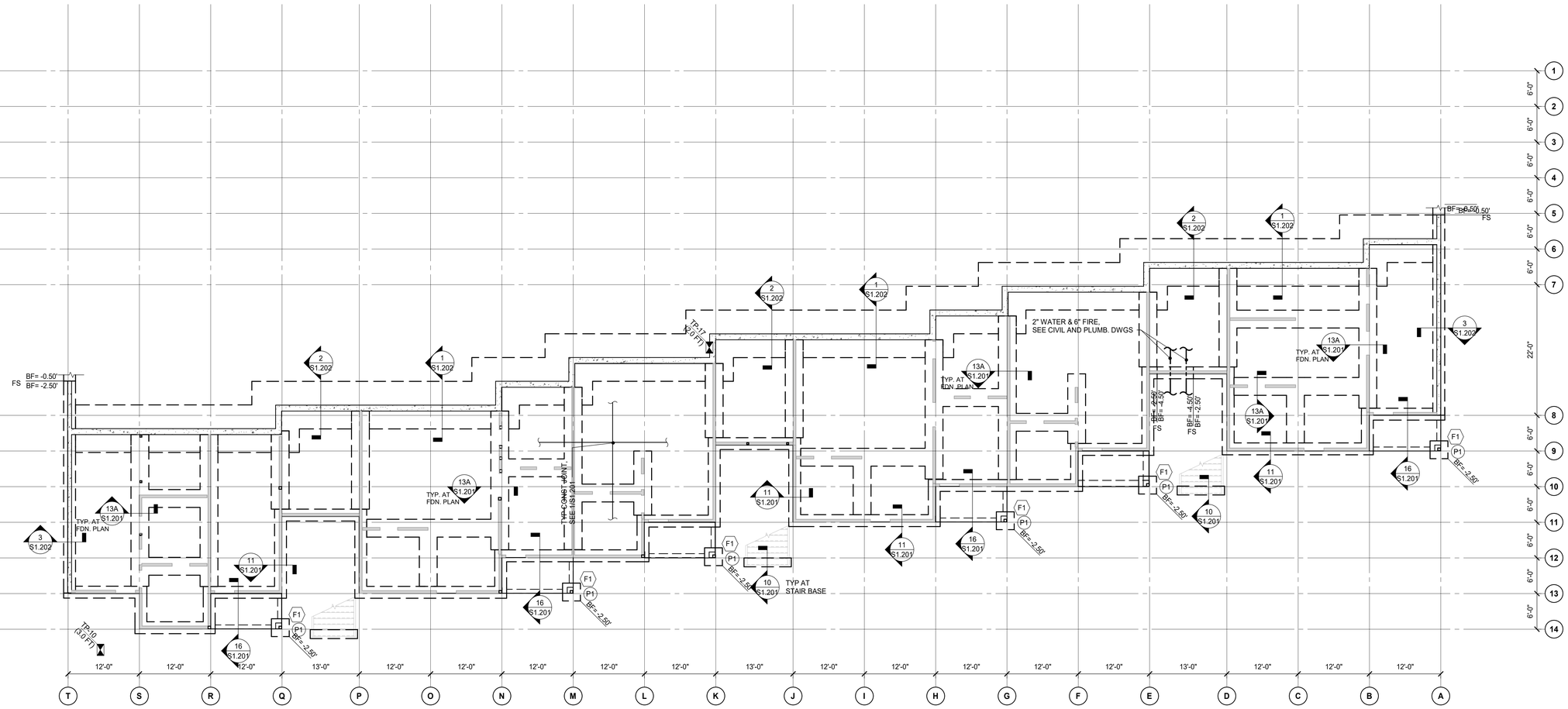

FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. REFERENCE BUILDING A1 DRAWINGS FOR BUILDING STRUCTURAL NOTES, SPECIAL INSPECTIONS, TYPICAL BUILDING CONSTRUCTION, SECTIONS, AND SCHEDULE.S.
2. TOP OF CONCRETE ELEVATION = 0.00' REFERENCE (ACTUAL ELEVATION= 1032.75')
3. REFER TO S1.100 ON BUILDING A1 FOR BALANCE OF PLAN NOTES NOT SHOWN.

ALABAMA LICENSED No. 22545 PROFESSIONAL ENGINEER THOMAS A. SOUTHARD

REVISIONS
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TERRACES AT HIGH MOUNTAIN
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S3.100
FOUNDATION PLAN
12/08/20

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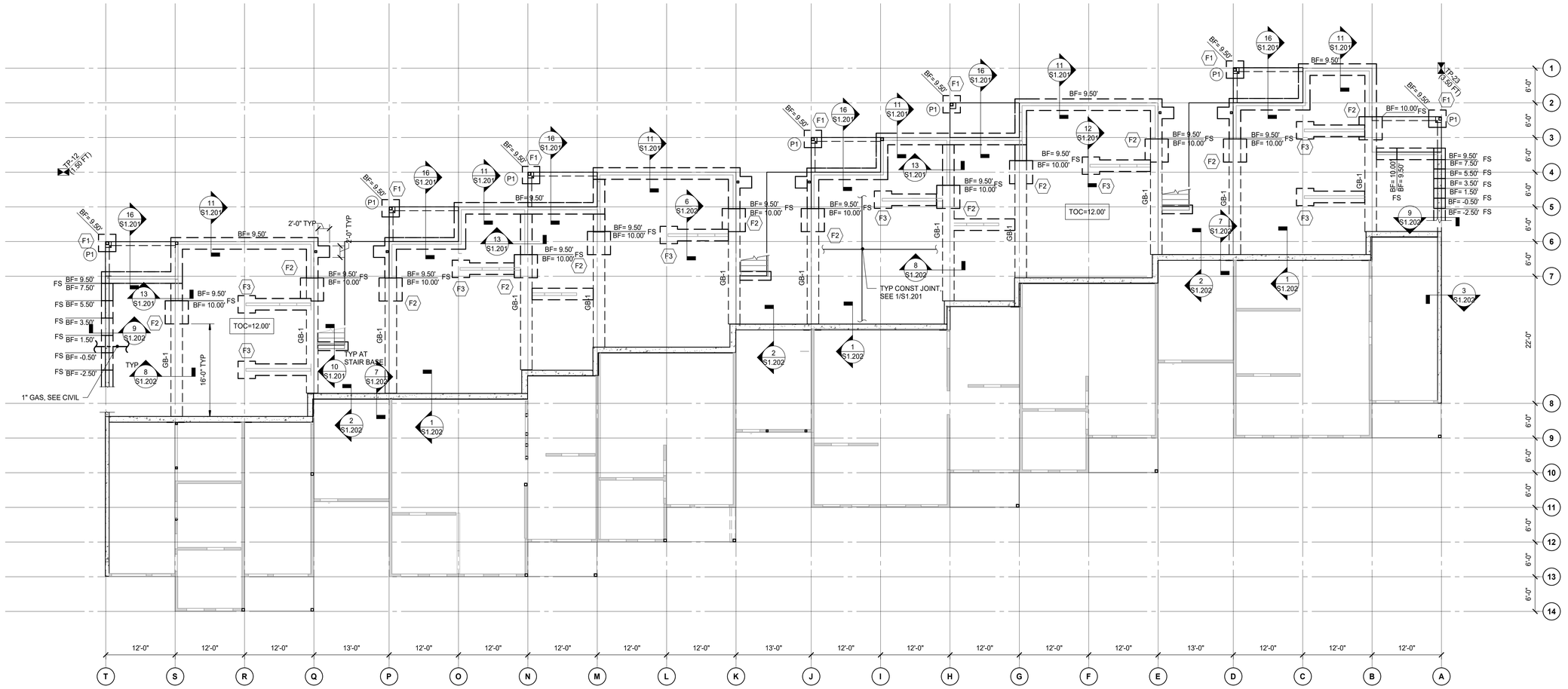

LEVEL 1 FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. SEE S3.100 FOR TYPICAL FOUNDATION PLAN NOTES..
2. TOP OF CONCRETE ELEVATION = 12.00' (REFERENCE EL = 1044.75')

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TERRACES AT HIGH MOUNTAIN

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S3.101
LEVEL 1 FRAMING PLAN

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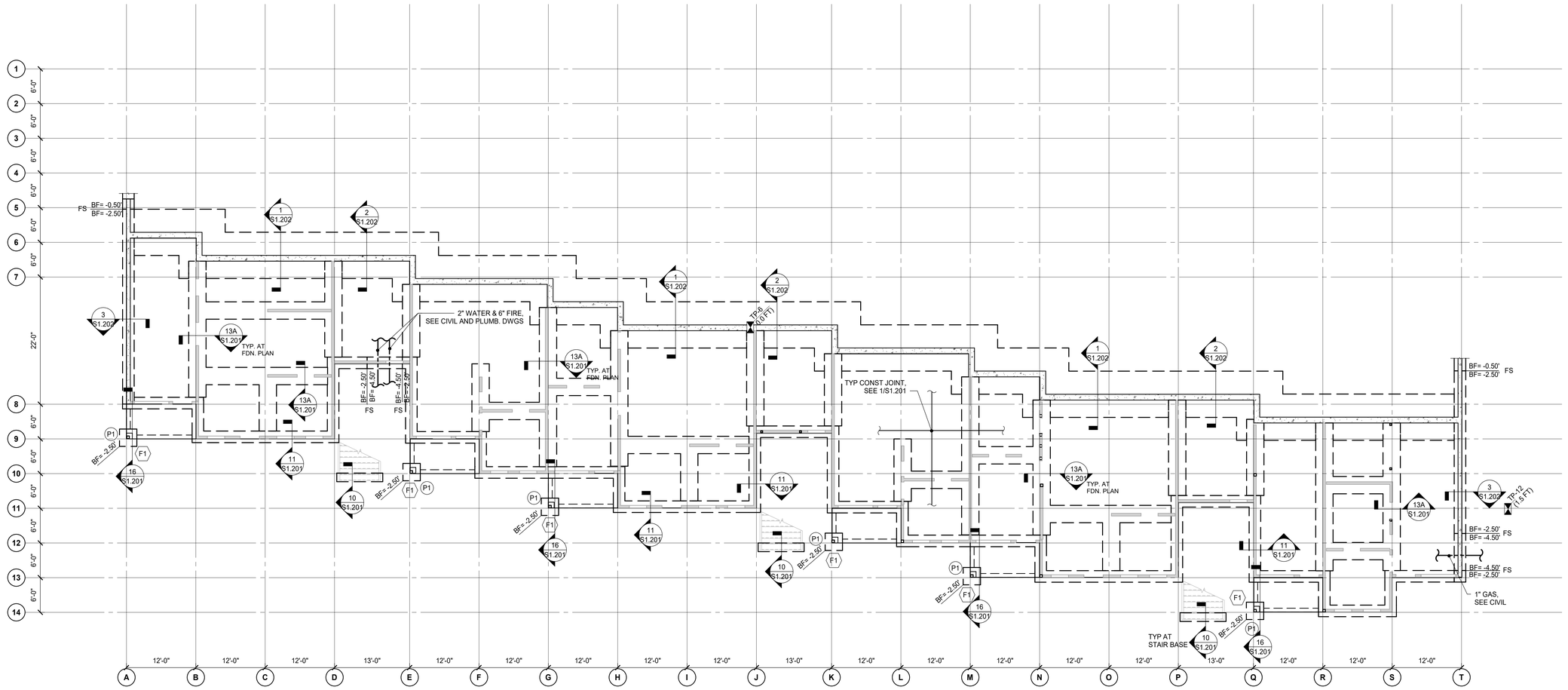

FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. REFERENCE BUILDING A1 DRAWINGS FOR BUILDING STRUCTURAL NOTES, SPECIAL INSPECTIONS, TYPICAL CONSTRUCTION, SECTIONS, DETAILS AND SCHEDULES.
2. TOP OF CONCRETE ELEVATION = 0.00' REFERENCE (ACTUAL ELEVATION= 1042.75')
3. REFER TO S1.100 ON BUILDING A1 FOR BALANCE OF PLAN NOTES NOT SHOWN.

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HUNTSVILLE, AL 35811
06/11/2021

S4.100
FOUNDATION PLAN

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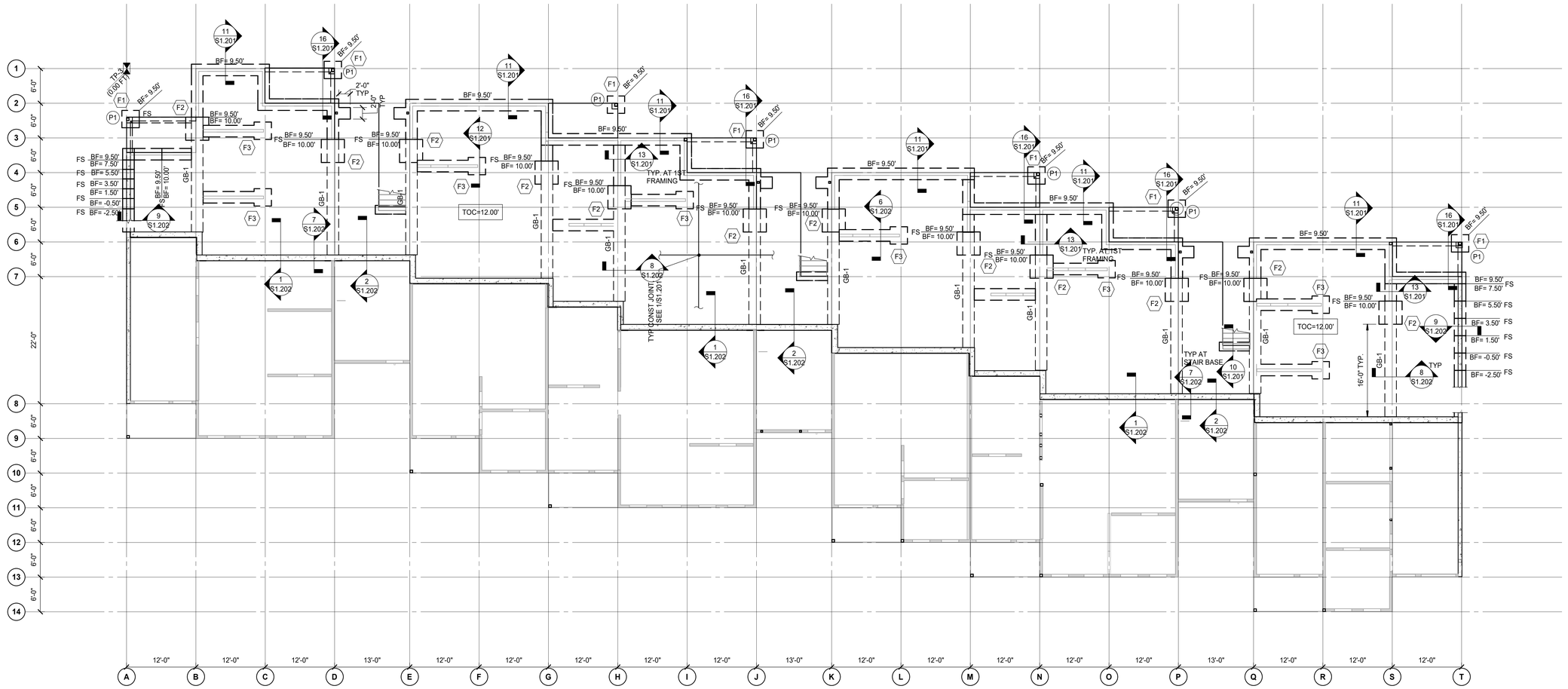

LEVEL 1 FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. SEE S4.100 FOR TYPICAL FOUNDATION NOTES.
2. TOP OF CONCRETE ELEVATION = 12.00' (REFERENCE EL = 1054.75')

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06/11/2021

S4.101
LEVEL 1 FRAMING PLAN

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STRUCTURAL NOTES

1. GENERAL

A. THE BUILDING IS DESIGNED UNDER THE PROVISIONS OF THE 2015 INTERNATIONAL BUILDING CODE AND ASCE 7-10.

B. THE FOLLOWING LOADS WERE USED IN THE DESIGN:

BUILDING RISK CATEGORY		II
FLOOR LIVE LOAD		
RESIDENTIAL UNITS		40 PSF
UNIT INTERNAL STAIRS		40 PSF / 300 LB PT LOAD
EXTERNAL EGRESS STAIRS		100 PSF / 300 LB PT LOAD
PRIVATE BALCONIES		60 PSF
ELEVATED CORRIDORS SERVING RESIDENTIAL UNITS		40 PSF
PUBLIC AREAS		100 PSF
NOTE: LIVE LOAD REDUCTION IS UTILIZED AS ALLOWED BY CODE		
ROOF LIVE LOAD		
MINIMUM UNIFORM ROOF LIVE LOAD		20 PSF / 300 PT LOAD
ROOF SNOW LOAD		
GROUND SNOW LOAD (Pg)		10 PSF
FLAT-ROOF SNOW LOAD (Pf)		7 PSF
RAIN-ON-SNOW SURCHARGE		5 PSF
SNOW EXPOSURE FACTOR (Ce)		1.0
SNOW LOAD IMPORTANCE FACTOR (Is)		1.0
THERMAL FACTOR (Ct)		1.0
WIND LOAD		
WIND SPEED (3-SECOND GUST)		
Vult		115 MPH
Vasd: (0.77*VULT)		90 MPH
WIND EXPOSURE		B
INTERNAL PRESSURE COEFFICIENT		+0.18, -0.18
COMPONENTS AND CLADDING WIND LOAD		
EFFECTIVE WIND AREA ZONE	PRESSURE	
10 FT.^2	4	+24.1 PSF
10 FT.^2	4	-26.1 PSF
10 FT.^2	5	+24.1 PSF
10 FT.^2	5	-32.1 PSF
EARTHQUAKE DESIGN		
SEISMIC IMPORTANCE FACTOR (Ie)		1.00
SEISMIC DESIGN CATEGORY		C
SITE CLASSIFICATION		D
SEISMIC RESPONSE COEFFICIENTS		
Ss		0.258
S1		0.120
Sds		0.274
Sd1		0.186
DESIGN BASE SHEAR		30 KIPS
SEISMIC-FORCE RESISTING SYSTEM PER ASCE 7-10 TABLE 12.2-1		TYPE A15
R		6.5
Cd		4
Cs		0.042
TL		12
ANALYSIS METHOD		ELFP
HANDRAIL AND GUARD LOADS		
HANDRAIL AND GUARD		50 PLF / 200 LB PT LOAD
INTERMEDIATE RAIL		50 LB PT LOAD

C. SEE ARCHITECTURAL DRAWINGS FOR ANGLES, CLIPS, PLATES, ETC., AND OTHER MISCELLANEOUS ITEMS. VERIFY AND COORDINATE ALL FRAMES, OPENINGS, ETC. WITH THE MECHANICAL AND ELECTRICAL CONTRACTORS.

D. SUBMIT SHOP DRAWINGS FOR THE FOLLOWING ITEMS. SUBMITTALS INCLUDE BUT MAY NOT BE LIMITED TO:

--CONCRETE MIX DESIGN
--REINFORCING STEEL
--STRUCTURAL STEEL
--PRE-ENGINEERED BUILDING COMPONENTS
--WOOD TRUSSES
--STEEL STAIRS

DO NOT USE CONTRACT DRAWINGS AS A BASE FOR SHOPS. REVIEW IS LIMITED TO DESIGN CONFORMANCE. CONTRACTOR IS RESPONSIBLE FOR DIMENSIONS.

E. CONTRACTOR SHALL COORDINATE WITH THE QUALIFIED AGENCY RETAINED BY THE OWNER TO PERFORM INSPECTION AND TESTING. INSPECTIONS REQUIRED INCLUDE, BUT MAY NOT BE LIMITED TO:

--SOILS AND FOUNDATIONS
--CONCRETE
--STRUCTURAL STEEL
--MASONRY

2. EARTHWORK

A. FOUNDATIONS ARE DESIGNED TO BEAR ON ENGINEERED FILL OR NATURAL SOIL WITH A CAPACITY OF 2,000 PSF, BASED ON RECOMMENDATIONS IN THE GEOTECHNICAL REPORT PREPARED BY OMI, INC. DATED 12/31/2020. THIS VALUE IS TO BE VERIFIED IN THE FIELD BY THE BUILDING INSPECTOR OR A QUALIFIED TESTING AGENCY.

B. BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 2 FOOT-0 INCH BELOW FINISH EXTERIOR GRADE. WHERE REQUIRED, STEP FOOTINGS IN RATIO OF 2 HORIZONTAL TO 1 VERTICAL.

C. COMPACTED BACKFILL BELOW BUILDING SLABS AND FOOTINGS: ALL SOIL FILL MATERIAL MUST BE APPROVED BY SOILS ENGINEER PRIOR TO PLACEMENT. PROOFROLL SUBGRADE REMOVING AND REPLACING SOFT OR COMPRESSIVE MATERIALS. FILL MATERIAL SHALL BE PLACED IN LAYERS NOT TO EXCEED 8 INCHES AND COMPACTED TO MIN. 95 PERCENT OF THE DRY MAXIMUM DENSITY AS DETERMINED BY ASTM D698.

D. AT ROCKY AREAS AROUND AND BELOW EL 1055 FT MSL: ROCK LEDGES, PINNACLES OR BOULDERS, IF ENCOUNTERED AND CONFLICTING WITH THE PROPOSED FOUNDATION SYSTEM, SHALL BE REMOVED BY BLASTING, RIPPING OR HOE RAMMING PER THE GEOTECHNICAL REPORT. ONCE THE AREA HAS BEEN EXCAVATED, A MINIMUM 1 FOOT LAYER OF #2 STONE SHALL BE PLACED ACROSS THE AREA. THE #2 STONE SHALL BE CAPPED WITH A 6" LAYER OF "CRUSHER RUN" (1.5" TO DUST, STONE). SUBSEQUENT LAYERS OF ENGINEERED FILL SHALL THEN BE PLACED TO BUILDING PAD ELEVATIONS, BELOW THE 4" #57 UNDERSLAB BASE.

E. AT SOIL AREAS AROUND AND ABOVE EL 1062 FT MSL: AREAS APPROXIMATLEY AT BUILDING PAD EL AND AREAS THAT WILL RECEIVE ENGINEERED FILL SHALL BE OBSERVED BY THE GER PRIOR TO PLACING THE 1 FOOT LAYER OF #2 STONE AND 6" CAP OF CRUSHER RUN. SUBSEQUENT LAYERS OF ENGINEERED FILL SHALL THEN BE PLACED TO BUILDING PAD ELEVATIONS, BELOW THE 4" #57 UNDERSLAB BASE.

3. CONCRETE

A. CONCRETE CONSTRUCTION SHALL BE PER THE APPLICABLE BUILDING CODE, ACI 318 AND ACI 301, LATEST EDITIONS.

B. CONCRETE SHALL ATTAIN THE FOLLOWING 28 DAY COMPRESSIVE STRENGTHS PER ASTM A39.

--FOOTINGS, PIERS	3,000 PSI
--SLAB-ON-GRADE	3,500 PSI
--RETAINING WALLS	4,000 PSI

C. VERIFY CONCRETE STRENGTHS WITH A MINIMUM OF ONE SET OF NINE 4X8-INCH COMPRESSION CYLINDERS, (3 @ 7 DAYS, 3 @ 28, 3 SPARE).

D. EXTERIOR CONCRETE SHALL BE AIR-ENTRAINED TO PROVIDE AN AIR CONTENT OF 6+/-1.5 PERCENT BY VOLUME.

E. PROVIDE CLEAR DISTANCE TO OUTERMOST REINFORCING AS FOLLOWS:

CONCRETE CAST AGAINST EARTH	3 INCHES
CONCRETE EXPOSED TO EARTH OR WEATHER:	
#5 OR SMALLER	1-1/2 INCHES
#6 OR LARGER	2 INCHES

F. NON-SHRINK GROUT FOR COLUMNS BASE PLATES SHALL ATTAIN A 28 DAY COMPRESSIVE STRENGTH: F'c = 5,000 PSI.

G. REINFORCING STEEL SHALL CONFORM TO A615-GR60; MESH SHALL CONFORM TO ASTM A185 WITH MINIMUM LAPS OF 8 INCHES. PLACING PLANS AND SHOP FABRICATION DETAILS SHALL BE IN ACCORDANCE WITH "THE MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES". FURNISH SUPPORT BARS AND A CCESSORIES IN ACCORDANCE WITH C.R.S.I. STANDARDS.

H. PROVIDE CORNER BARS TO MATCH HORIZONTAL REINFORCING IN WALLS AND FOOTINGS. SPLICE LAPS SHALL BE A MINIMUM OF 36 BAR DIAMETERS, UNLESS NOTED OTHERWISE. PROVIDE DOWELS BETWEEN FOOTINGS AND WALLS OR PIERS TO MATCH SIZE AND SPACING OF VERTICAL REINFORCING.

I. WALLS WITH LATERAL EARTH PRESSURES SHALL BE ADEQUATELY SHORED OR FLOOR/ROOF CONSTRUCTION SHALL BE IN PLACE AND SECURED PRIOR TO BACKFILLING.

J. INSTALLATION OF ELECTRICAL CONDUIT WITHIN THE CONCRETE SLAB-ON-GRADE IS PROHIBITED.

4. MASONRY

A. MASONRY CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE APPLICABLE BUILDING CODE AND THE "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" TMS 402/ACI-530/ASCE 5 AND THE "SPECIFICATIONS FOR MASONRY STRUCTURES" TMS 602/ACI-530.1/ASCE 6, LATEST EDITIONS.

B. MASONRY TO CONFORM TO THE FOLLOWING SPECIFICATIONS:

HOLLOW LOAD-BEARING C.M.U	ASTM C90
CONCRETE BUILDING BRICK	ASTM C55, GRADE A
MORTAR	ASTM C270, TYPE M OR S
GROUT	ASTM C476

C. MASONRY ASSEMBLIES SHALL HAVE COMPRESSIVE STRENGTH (F'M) GREATER THAN OR EQUAL TO 2000 PSI.

D. WALLS SHALL BE CONSTRUCTED USING A FULL BED OF MORTAR. VERTICAL REINFORCING SHALL BE GROUTED IN PLACE WITH 2500 PSI GROUT (GROUT SLUMP SHALL FALL BETWEEN 8 AND 11 INCHES) POUR HEIGHT AND LIFT HEIGHT SHALL NOT EXCEED 5 FEET - 0 INCHES.

E. PROVIDE CONTINUOUS HORIZONTAL JOINT REINFORCING IN MASONRY WALLS AT 16 INCHES O.C. PROVIDE AT 8 INCHES O.C. AT PARAPETS.

F. CAVITY WALLS OF BRICK AND BLOCK SHALL BE CONSTRUCTED WITH JOINT REINFORCING IN MASONRY AND ADJUSTABLE METAL ANCHORS TO BRICK.

G. UNLESS NOTED OTHERWISE, PROVIDE 16 INCH LONG BY 24 INCHES HIGH SOLID OR GROUTED BLOCK UNDER BEARING ENDS OF BEAMS.

H. PROVIDE 48 INCH REINFORCEMENT LAP AT CONTINUOUS BOND BEAM STEPS.

I. COMPOSITE WALLS SHALL HAVE THE COLLAR JOINT BETWEEN BRICK AND BLOCK GROUTED SOLID AND THE WALLS SHALL BE BUILT WITH BOTH WYTHES SIMULTANEOUSLY.

J. MASONRY WALLS SHALL HAVE CONTROL JOINTS AT 30 FEET ON CENTER UNLESS NOTED OTHERWISE.

K. REINFORCING STEEL SHALL CONFORM TO ASTM A615-GR60. LAP BARS A MINIMUM OF 48 BAR DIAMETERS. GROUT ALL REINFORCED CORES SOLID.

L. UNLESS SHOWN ON PLAN, LINTELS FOR MASONRY WALLS SHALL BE AS FOLLOWS:

OPENINGS TO 3 FT, 0 IN	3-1/2 X 3-1/2 X 1/4
3 FT, 1 IN TO 5 FT, 0 IN	4 X 3-1/2 X 5/16 - 3-1/2 HORIZONTAL
5 FT, 1 IN TO 6 FT, 6 IN	5 X 3-1/2 X 5/16 - 3-1/2 HORIZONTAL
OVER 6 FT, 6 IN	CONSULT ARCHITECT/ENGINEER

PROVIDE 1 ANGLE FOR EACH 4 INCHES OF WALL THICKNESS. LINTELS SHALL BEAR 6 INCHES MINIMUM EACH END U.N.O.

M. PROVIDE TWO-PIECE ADJUSTABLE ANCHORS TO MASONRY AT A MAXIMUM SPACING OF 24 INCHES O.C. AT ALL VERTICAL AND HORIZONTAL STRUCTURAL STEEL MEMBERS.

N. CAVITY WALLS OF BRICK WITH STUD BACKUP SHALL BE CONSTRUCTED WITH TWO-PIECE ADJUSTABLE METAL ANCHORS AT A MAXIMUM SPACING OF 16 INCHES O.C. HORIZONTAL (INTO STUDS) AND 24 INCHES O.C. VERICAL. AT BRICK WALLS OVER 30 FEET HIGH, PROVIDE ANCHORS AT 16 INCHES O.C. HORIZONTAL AND VERTICAL.

O. ALL NON-LOADBEARING MASONRY WALLS SHALL BE PROVIDED WITH VERTICAL SLIP CONNECTIONS AT THE TOP OF THE WALL, U.N.O.

5. STEEL

A. STEEL CONSTRUCTION SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE APPLICABLE BUILDING CODE AND SHALL CONFORM TO AISC 360. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING ASTM STANDARDS:

WIDE FLANGE SHAPES	A992 – GR50
STEEL PLATES, CHANNELS AND ANGLES	A36
STRUCTURAL PIPES	A53 – GR B
STRUCTURAL RECT/ROUND (HSS)	A500 - GR B
ANCHOR RODS (3/4" DIAM. OR LESS)	F1554 – 36 KSI
ANCHOR RODS (7/8" DIAM. OR GREATER)	F1554 – 55 KSI
HIGH-STRENGTH BOLTS	A325
HEADED STUDS	A108

B. BOLTED CONNECTIONS TO USE A325-TYPE N, HIGH STRENGTH BOLTS IN BEARING TYPE CONNECTIONS TIGHTENED TO A SNUG TIGHT CONDITION IN ACCORDANCE WITH RCSC SPECIFICATIONS.

C. SHOP CONNECTIONS TO BE WELDED OR BOLTED. FIELD CONNECTIONS TO BE BOLTED UNLESS OTHERWISE SHOWN. BOLT HOLES TO BE STANDARD ROUND HOLES (d+1/16 INCHES) UNLESS OTHERWISE NOTED. SHORT SLOTS SHALL BE PERMITTED NORMAL TO THE LOAD DIRECTION IN SLIP CRITICAL AND BEARING TYPE CONNECTIONS AS PER AISC REQUIREMENTS.

D. IF BEAM REACTIONS ARE DENOTED ON THE DRAWINGS, BEAM-WEB CONNECTIONS FOR SHEAR AT EACH END SHALL BE DETAILED TO SUPPORT THE LOADS SHOWN OR PROVIDE THE FOLLOWING MINIMUM NUMBER OF BOLTS, WHICHEVER IS GREATER. STIFFENED SEATS SHALL BE DETAILED TO SUPPORT THE LOADS SHOWN ON THE DRAWINGS OR THE MINIMUM FACTORED LOADS INDICATED BELOW, WHICHEVER IS GREATER.

	BEAM/WEB	STIFF. SEAT
W8 OR W10	2 BOLTS	20K
W12 OR W14	3 BOLTS	30K
W16 OR W18	4 BOLTS	40K
W21 OR W24	5 BOLTS	60K

E. STRUCTURAL STEEL SHALL BE GIVEN ONE SHOP COAT OF APPROVED SHOP PRIMER APPLIED TO CLEAN AND DRY SURFACES. DO NOT PAINT STEEL THAT WILL BE FIREPROOFED OR EMBEDDED IN CONCRETE.

F. STEEL BEAMS SHALL BE WELDED TO STEEL BEARING PLATES WITH 3 INCH LONG BY 1/4-INCH FILLET WELD EACH SIDE OF FLANGE (MINIMUM).

G. WELDING OF STRUCTURAL STEEL SHALL BE WITH E70XX ELECTRODES.

H. STEEL FLITCH BEAMS SHALL BE CONNECTED WITH 1/2" DIAMETER THROUGH BOLTS @ 16" O.C. WITH THE FIRST ROW OF BOLTS 6" FROM EACH END. SEE SCHEDULE ON S5.401.

6. WOOD FRAMING (CONVENTIONAL/TYPE V)

A. FRAMING LUMBER FOR STUDS, HEADERS AND JOISTS SHALL BE HEM FIR #2, SPRUCE-PINE-FIR (SPF) #2, OR BETTER, WITH A MAXIMUM MOISTURE CONTENT OF 19-PERCENT, HAVING THE FOLLOWING MINIMUM PROPERTIES (BASED ON 2X12 MEMBERS):

BENDING STRESS "Fb"	= 850 PSI FOR SINGLE MEMBER USE
HORIZONTAL SHEAR "Fv"	= 135 PSI
COMPRESSION PERPENDICULAR TO GRAIN "Fc"	= 405 PSI
COMPRESSION PARALLEL TO GRAIN "Fc11"	= 1150 PSI
MODULUS OF ELASTICITY "E"	= 1,300,000 PSI

NOTE: SPF (SOUTH) IS NOT ACCEPTABLE.

B. ALL EXPOSED EXTERIOR FRAMING AND FRAMING IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE-TREATED (PT). FRAMING SHALL BE PRESSURE-TREATED WITH ALAKALINE COPPER QUAT (ACQ) OR COPPER AZOLE (CBA-A AND CA-B), NOT SODIUM BORATE (SBX). PT LUMBER SHALL NOT BE INCISED.

C. STRUCTURAL POSTS AND TREATED LUMBER (PT) SHALL BE SOUTHERN PINE (SP) #2 OR BETTER, HAVING THE FOLLOWING MINIMUM PROPERTIES (BASED ON 2X12 MEMBERS):

	STRENGTH
BENDING STRESS "Fb"	= 750 PSI SINGLE MEMBER USE
HORIZONTAL SHEAR "Fv"	= 175 PSI
COMPRESSION PERPENDICULAR TO GRAIN "Fc"	= 565 PSI
COMPRESSION PARALLEL TO GRAIN "Fc11"	= 1,250 PSI
MODULUS OF ELASTICITY "E"	= 1,400,000 PSI

D. LAMINATED VENEER LUMBER (LVL OR MICROLAM) BEAMS SHALL CONFORM TO ASTM D 5456 AND SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES (BASED ON 1-3/4 X 11-7/8 MEMBERS):

BENDING STRESS "Fb"	= 2600 PSI
HORIZONTAL SHEAR "Fv"	= 285 PSI
MODULUS OF ELASTICITY "E"	= 2,000,000 PSI
BEARING STRESS "FPERP"	= 750 PSI
EQUIV SPECIFIC GRAVITY FOR CONNECTION DESIGN= 0.50	

E. PARALLEL STRAND LUMBER (PSL) COLUMNS SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES:

BENDING STRESS "Fb"	= 2,400 PSI
HORIZONTAL SHEAR "Fv"	= 190 PSI
COMPRESSION PARALLEL TO GRAIN "Fc11"	= 2,500 PSI
MODULUS OF ELASTICITY "E"	= 1,800,000 PSI

F. AT EXTERIOR WALLS, PROVIDE SOLID BLOCKING AT 4 FEET ON CENTER BETWEEN BAND JOIST AND FIRST INTERIOR PARALLEL JOIST.

G. PREFABRICATED JOIST HANGERS, BEAM HANGERS, POST CAPS, AND POST BASES SHALL BE SIZED AND ATTACHED PER MANUFACTURER'S RECOMMENDATIONS, TO ACHIEVE AT LEAST THE MINIMUM MANUFACTURER LISTED CAPACITIES, UNO ON THE DRAWINGS. FASTENERS AND CONNECTORS UTILIZED WITH TREATED LUMBER (PT OR FRT) SHALL MEET G185 HOT-DIPPED GALVANIZING.

H. ANCHOR BOLTS CONNECTING PRESSURE-TREATED WOOD PLATES TO FOUNDATIONS, MASONRY WALLS, OR CONCRETE SLABS SHALL BE HOT-DIPPED GALVANIZED.

I. BUILT-UP STUD COLUMNS SHALL HAVE ONE JACK STUD AND THE REMAINING STUDS SHALL BE KING STUDS. MULTIPLE STUDS SHALL BE NAILED WITH 10D NAILS AT 8 INCHES O.C. PROVIDE SOLID BLOCKING OR CRIPPLE STUDS IN FLOOR SYSTEM AT ALL POINT LOADS ABOVE.

J. FREESTANDING POSTS SHALL HAVE PREFAB POSTCAP AND BASE. POSTS WITHIN WALL NEED ONLY HAVE PREFAB CAP ATTACHED TO BEAM, UNO. POSTS WITHIN WALL BEARING ON MASONRY OR CONCRETE SHALL HAVE PREFAB BASE, UNO.

K. STANDARD MEMBER CONNECTIONS SHALL BE PER FASTENING SCHEDULE IN SECTION 23 OF THE INTERNATIONAL BUILDING CODE (IBC), UNO.

L. STUD BEARING WALLS TO BE PROVIDED WITH 2 CONTINUOUS TOP PLATES AND 1 CONTINUOUS BOTTOM PLATE WITH A MINIMUM OF ONE ROW OF HORIZONTAL BRIDGING AT MID-HEIGHT OF WALL UNLESS NOTED OTHERWISE. SPLICES OF TOP PLATES SHALL OCCUR OVER STUD AND SHALL BE STAGGERED A MINIMUM OF FOUR FEET.

M. NAILS FOR FRAMING AND SHEATHING CONNECTIONS SPECIFIED IN THE DRAWINGS AND ASSOCIATED NOTES SHALL CONFORM TO ASTM F1667 AND SHALL MEET THE FOLLOWING MINIMUM SIZE REQUIREMENTS:

TYPE	DIAMETER x LENGTH
8d	0.131" x 2-1/2"
10d	0.148" x 3"
12d	0.148" x 3-1/4"
16d	0.162" x 3-1/2"
20d	0.192" x 4"

SHANK DIAMETER MINIMUM STRENGTH	
0.099" TO 0.142"	100 KSI
0.143" TO 0.177"	90 KSI
0.178" TO 0.254"	80 KSI

NOTE: NAILS USED IN STANDARD CONNECTIONS SHALL BE SIZED PER THE REQUIREMENTS OF THE BUILDING CODE.

N. ROOF MEMBERS SHALL BE CONNECTED AT EACH BEARING POINT WITH ONE PREFABRICATED GALVANIZED METAL ANCHOR. ANCHORS SHALL BE 18 GAGE MINIMUM AND SHALL BE ATTACHED TO HAVE A CAPACITY TO RESIST A 450# UPLIFT LOADING, UNLESS SHOWN OTHERWISE ON DRAWINGS.

P. THE MINIMUM DEPTH AND MAXIMUM SPACING OF WOOD TRUSSES IS SHOWN ON DRAWINGS. THE SUPPLIER SHALL ADJUST SPACING AS REQUIRED TO MEET THE LOADINGS DESIGNATED ON THE DRAWING.

Q. PROVIDE LSL BAND BOARD IN WOOD TRUSS SYSTEMS AT ALL PERIMETER BEARING WALLS. ALTERNATIVELY, PROVIDE 2-3/4 INCH PLYWOOD BANDS GLUED AND SCREWED TOGETHER. PROVIDE SQUASH BLOCKS AND STIFFENERS AS REQUIRED TO DISTRIBUTE LOADINGS AND AS REQUIRED BY MANUFACTURER. PROVIDE SOLID BLOCKING AT INTERIOR JOIST SUPPORTS WITH BEARING WALLS ABOVE.

R. DO NOT SPLICE STRUCTURAL MEMBERS BETWEEN SUPPORTS.

S. PREFABRICATED TRUSSES SHALL BE DESIGNED FOR THE LOADS SCHEDULED ON THE DRAWINGS. SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR REVIEW. THE DESIGN OF THE BRACING REQUIRED TO LATERALLY STABILIZE THE TRUSSES AND TRUSS MEMBERS SHALL BE THE RESPONSIBILITY OF THE SPECIALTY TRUSS ENGINEER. AFFIX SEAL OF ENGINEER REGISTERED IN THE STATE OF THE PROPOSED PROJECT. TEMPORARY BRACING DURING ERECTION IS THE RESPONSIBILITY OF THE CONTRACTOR.

7. SHEATHING

A. FLOOR SHEATHING SHALL BE 23/32 (3/4) INCH APA RATED STURD-I (COMBINATION SUBFLOOR-UNDERLAYMENT) WOOD STRUCTURAL PANEL, TONGUE AND GROOVE, WITH SPAN RATING OF 48/24. PANELS SHALL HAVE LONG DIMENSION ORIENTED ACROSS THREE OR MORE JOISTS AND SHALL BE FASTENED WITH CONSTRUCTION ADHESIVE AND NAILS AT PANEL EDGES AND INTERMEDIATE SUPPORTS AS SCHEDULED ON THE DRAWINGS. UNLESS NOTED OTHERWISE, PANEL EDGES NEED NOT BE BLOCKED. INSTALL PER MANUFACTURER'S INSTALLATION INSTRUCTIONS.

B. EXTERIOR SHEATHING SHALL BE 7/16 (1/2) INCH THICK APA RATED WOOD STRUCTURAL PANELS U.N.O. AS SHEAR WALL. FASTEN PANELS TO STUDS WITH 8D NAILS AT 6 INCHES ON CENTER AT PANEL EDGES AND 12 INCHES ON CENTER AT INTERMEDIATE SUPPORTS. UNLESS NOTED OTHERWISE, PANEL EDGES NEED NOT BE BLOCKED. IF EXTERIOR WALLS ARE DENOTED AS SHEAR WALLS, THEY SHALL BE SHEATHED, FASTENED AND BLOCKED AS SCHEDULED ON THE DRAWINGS.

C. SHEARWALLS SHALL BE SHEATHED, FASTENED AND BLOCKED AS SCHEDULED ON THE DRAWINGS.

D. ROOF SHEATHING SHALL BE 23/32 (3/4) INCH APA RATED WOOD STRUCTURAL PANEL, TONGUE AND GROOVE, WITH SPAN RATING OF 48/24. PANELS SHALL HAVE LONG DIMENSION ORIENTED ACROSS THREE OR MORE JOISTS AND SHALL BE FASTENED WITH CONSTRUCTION ADHESIVE AND NAILS AT PANEL EDGES AND INTERMEDIATE SUPPORTS AS SCHEDULED ON THE DRAWINGS. UNLESS NOTED OTHERWISE, PANEL EDGES NEED NOT BE BLOCKED.

8. POST-INSTALLED ANCHORS IN CONCRETE AND MASONRY

A. GENERAL

INSTALL ANCHORS IN STRICT CONFORMANCE WITH THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS AND PROCEDURES. ALL POST-INSTALLED ANCHORS IN CONCRETE SHALL HAVE ICC APPROVAL FOR USE IN CRACKED CONCRETE.

SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE SUBMITTED FOR APPROVAL PRIOR TO USE. CONTRACTOR SHALL PROVIDE LOAD CAPACITIES DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT.

PROVIDE STAINLESS STEEL FASTENERS FOR EXTERIOR USE OR WHEN PERMANENTLY EXPOSED TO WEATHER. PROVIDE GALVANIZED CARBON STEEL ANCHORS AT OTHER LOCATIONS, UNLESS OTHERWISE NOTED.

B. PRODUCTS

ANCHORS IN CONCRETE:
--EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT TZ.
--UNDERCUT ANCHORS SHALL BE HILTI HDA.
--SCREW ANCHORS SHALL BE HILTI KWIK HUS.
--ADHESIVE ANCHORS SHALL BE HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HIT-Z ROD OR WITH HILTI HOLLOW DRILL BIT SYSTEM WITH HAS-E THREADED ROD.

ANCHORS IN MASONRY:
--EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT TZ. GROUT MASONRY CELLS SOLID WITH 2000 PSI GROUT AT ANCHOR LOCATIONS.
--SCREW ANCHORS SHALL BE HILTI KWIK HUS. GROUT MASONRY CELLS SOLID WITH 2000 PSI GROUT AT ANCHOR LOCATIONS.
--ADHESIVE ANCHORS IN SOLID MASONRY SHALL BE HILTI HIT-HY 270 ADHESIVE ANCHORING SYSTEM. STEEL ANCHOR ELEMENT SHALL BE HILTI HAS-E CONTINUOUSLY THREADED ROD OR HILTI HIS-N INTERNALLY THREADED INSERT.
--ADHESIVE ANCHORS IN HOLLOW OR MULTI-WYTHE MASONRY SHALL BE HILTI HIT-HY 270 ADHESIVE ANCHORING SYSTEM. STEEL ANCHOR ELEMENT SHALL BE HILTI HAS-E CONTINUOUSLY THREADED ROD OR HILTI HIT-IC INTERNALLY THREADED INSERT. THE APPROPRIATE SIZE SCREEN TUBE SHALL BE USED PER THE ADHESIVE MANUFACTURER'S RECOMMENDATION.

C. INSTALLATION

ALL INSTALLATION PROCEDURES SHALL BE PER MANUFACTURERS RECOMMENDATIONS. COORDINATE AND/OR PROVIDE FOR THIRD PARTY INSPECTION AS REQUIRED BY BUILDING CODE OR LOCAL JURISDICTION.

ANCHOR CAPACITY IS DEPENDENT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHOR TO EDGE OF CONCRETE OR MASONRY. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE DISTANCE INDICATED ON THE DRAWINGS; IF NOT SHOWN, COMPLY WITH MINIMUM SPACING AND EDGE DISTANCE FOR FULL ANCHOR CAPACITY, AS SPECIFIED BY MANUFACTURER.

EXISTING REINFORCING BARS IN THE CONCRETE OR MASONRY STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. DO NOT CUT OR DAMAGE REINFORCING BARS UNLESS SPECIFICALLY PERMITTED IN THE DRAWINGS.

PRIOR TO DRILLING, THE CONTRACTOR SHALL LOCATE REINFORCING BAR POSITIONS IN THE IMMEDIATE VICINITY OF PROPOSED POST-INSTALLED ANCHORS USING GPR, X-RAY, OR OTHER NON-DESTRUCTIVE MEANS.

WHEN CONFLICTS BETWEEN PROPOSED ANCHORS AND EXISTING REINFORCING BARS EXIST, SUBMIT RESULTS OF BAR LOCATIONS TO ARCHITECT / ENGINEER FOR REVIEW AND FURTHER DIRECTION.

WARNING: THE STRUCTURAL INTEGRITY OF THE BUILDING SHOWN ON THESE PLANS IS DEPENDENT UPON COMPLETION ACCORDING TO PLANS AND SPECIFICATIONS. STRUCTURAL MEMBERS ARE NOT SELF-BRACING UNTIL PERMANENTLY AFFIXED TO THE STRUCTURE. THE STRUCTURAL ENGINEERS ASSUME NO LIABILITY FOR THE STRUCTURE DURING CONSTRUCTION.
09/18

ABBREVIATIONS & LEGEND

A		K	
AB	ANCHOR BOLT	K	KIP
ADDL	ADDITIONAL	KO	KNOCK-OUT
ADJ	ADJACENT	KSI	KIPS PER SQ. INCH
AFF	ABOVE FINISH FLOOR		
ALT	ALTERNATE	L	
APPROX	APPROXIMATE(LY)	L	LINTEL MARK
ARCH	ARCHITECT(URAL)	LLH	LONG LEG HORIZONTAL
		LLV	LONG LEG VERTICAL
B		LL	LIVE LOAD
B	BEAM MARK	LP	LOW POINT
BF	BOTTOM OF FOOTING ELEVATION BRACED FRAME MARK	LVL	LAMINATED VENEER LUMBER
BLKG	BLOCKING	M	
BLDG	BUILDING	MANUF	MANUFACTURER(ED)
BM	BEAM	MAS	MASONRY
BOD	BOTTOM OF DECK	MAX	MAXIMUM
BOS	BOTTOM OF STEEL	MIN	MINIMUM
BOTT	BOTTOM	MISC	MISCELLANEOUS
BP	BEARING PLATE MARK	MO	MASONRY OPENING
BRG	BEARING	MATL	MATERIAL
BSMT	BASEMENT	MTL	METAL
BTWN	BETWEEN		
		N	
C		NTS	NOT TO SCALE
C	COLUMN MARK	NS	NEAR SIDE
CIP	CAST IN PLACE	NIC	NOT IN CONTRACT
CJ	CONTROL/CONSTRUCTION JOINT		
CLR	CLEAR(ANCE)	O	
CMU	CONCRETE MASONRY UNIT	OC	ON CENTER(S)
COL	COLUMN	OPNG	OPENING
COM	CENTER OF MASONRY WALL	OPP	OPPOSITE
COMP	COMPOSITE	OF	OUTSIDE FACE
CONC	CONCRETE		
CONN	CONNECTION	P	
CONST	CONSTRUCTION	P	PIER MARK
CONT	CONTINUOUS	PC	PRECAST CONCRETE
COORD	COORDINATE(TION)	PDF	POWER DRIVEN FASTENER
COS	CENTER OF STUD	PEB	PRE-ENGINEERED BUILDING
		PERIM	PERIMETER
D		PL	PLATE
DBA	DEFORMED BAR ANCHORS	PLF	POUNDS PER LINEAR FOOT
DTL	DETAIL	PP	PRECAST PLANK MARK
DIAM	DIAMETER	PROJ	PROJECTION
DIAG	DIAGONAL	PSF	POUNDS PER SQ. FOOT
DN	DOWN	PSI	POUNDS PER SQ. INCH
DWG	DRAWING	PSL	PARALLEL STRAND LUMBER COLUMN
DBL	DOUBLE	PT	POST TENSION(ED)/PRESSURE TREATED
DL	DEAD LOAD		
		Q	
E		QTY	QUANTITY
EA	EACH		
EE	EACH END	R	
EF	EACH FACE	RAD	RADIUS
EL	ELEVATION	RD	ROOF DRAIN
ELEV	ELEVATOR	REV	REVISION, REVISE(D)
EOD	EDGE OF DECK	REINF	REINFORCE(D), (ING)
EOJ	EDGE OF JOIST	REM	REMAINDER
EOS	EDGE OF SLAB	REQ'D	REQUIRED
EQ	EQUAL	RTU	ROOF TOP UNIT
EQUIP	EQUIPMENT		
ES	EACH SIDE	S	
EW	EACH WAY	SB	SOIL BORING
EXIST, EX	EXISTING	SC	SLIP CRITICAL
EXP	EXPANSION	SE	SPECIALTY DESIGN ENGINEER
EXT	EXTERIOR	SIM	SIMILAR
		SJI	STEEL JOIST INSTITUTE
F		SOG	SLAB ON GRADE
F	FOOTING MARK	SQ	SQUARE
FD	FLOOR DRAIN	STD	STANDARD
FDN	FOUNDATION	STL	STEEL
FOB	FACE OF BUILDING	STRUCT	STRUCTURAL

30x42 SCHEDULE OF SPECIAL INSPECTIONS

		VERIFICATION AND INSPECTION	Y/N	FREQUENCY CONTINUOUS	FREQUENCY PERIODIC	REFERENCED STANDARD	IBC REFERENCE	SCOPE OF SERVICE	RESPONSIBLE PARTY
		STRUCTURAL STEEL					1705.2	-	
1		HIGH-STRENGTH BOLTING: INSPECTION TASKS PRIOR TO BOLTING							
	A.	MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS.	Y	X	-	-	1705.2	-	SIER
	B.	FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	Y	-	X	AISC 360 & applicable ASTM material standards	1705.2	-	SIER
	C.	PROPER FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE).	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS ONLY	SIER
	D.	PROPER BOLTING PROCEDURE SELECTED FOR JOINT DETAIL.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS ONLY	SIER
	E.	CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION & HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS ONLY	SIER
	F.	PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED & DOCUMENTED FOR FASTENER ASSEMBLIES & METHODS USED.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS ONLY	SIER
	G.	PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS, & OTHER FASTENER COMPONENTS.	Y	-	X	AISC 360	1705.2	-	SIER
2		HIGH-STRENGTH BOLTING: INSPECTION TASKS DURING BOLTING							
	A.	FASTENER ASSEMBLIES, OF SUITABLE CONDITION, PLACED IN ALL HOLES & WASHERS (IF REQUIRED) ARE POSITIONED AS REQUIRED.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS USING CALIBRATED WRENCH METHOD OR TURN-OF-NUT WITHOUT MATCHMARKING ONLY	SIER
	B.	JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO PRETENSIONING OPERATION.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS USING CALIBRATED WRENCH METHOD OR TURN-OF-NUT WITHOUT MATCHMARKING ONLY	SIER
	C.	FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS USING CALIBRATED WRENCH METHOD OR TURN-OF-NUT WITHOUT MATCHMARKING ONLY	SIER
	D.	FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS USING CALIBRATED WRENCH METHOD OR TURN-OF-NUT WITHOUT MATCHMARKING ONLY	SIER
3		HIGH-STRENGTH BOLTING: INSPECTION TASKS AFTER BOLTING							
	A.	DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS.	Y	X	-	AISC 360	1705.2	-	SIER
4		WELDING: INSPECTION TASKS PRIOR TO WELDING							
	A.	WELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE.	Y	X	X	AISC 360 & applicable AWS Documents	1705.2	ESTABLISH THE JOINT WELDING PROCEDURES ARE PREQUALIFIED OR TEST IN ACCORDANCE WITH AWS D1.1 QUALIFICATION PROCEDURES	SIER
	B.	MANUFACTURER'S CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE.	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	C.	MATERIAL IDENTIFICATION (TYPE/GRADE).	Y	-	X	AISC 360	1705.2	-	SIER
	D.	FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY):							
		1) JOINT PREPARATION	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		2) DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		3) CLEANLINESS (CONDITION OF STEEL SURFACES)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		4) TACKING (TACK WELD QUALITY & LOCATION)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		5) BACKING TYPE & FIT (IF APPLICABLE)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	E.	CONFIGURATION & FINISH OF ACCESS HOLES	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	F.	FIT-UP OF FILLET WELDS:							
		1) DIMENSIONS (ALIGNMENT, GAPS AT ROOT)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		2) CLEANLINESS (CONDITION OF STEEL SURFACES)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		3) TACKING (TACK WELD QUALITY & LOCATION)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
5		WELDING: INSPECTION TASKS DURING WELDING							
	A.	USE OF QUALIFIED WELDERS	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	WELDERS MUST BE CURRENTLY CERTIFIED UNDER AMERICAN WELDING SOCIETY QUALIFICATION PROCEDURES.	SIER
	B.	CONTROL & HANDLING OF WELDING CONSUMABLES INCLUDING PACKAGING & EXPOSURE CONTROL	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	C.	ENVIRONMENTAL CONDITIONS: WIND SPEED WITHIN LIMITS, PRECIPITATION, & TEMPERATURE.	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	D.	WPS FOLLOWED: SETTINGS ON WELDING EQUIPMENT, TRAVEL SPEED, SELECTED WELDING MATERIALS, SHIELDING GAS TYPE/FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED (MIN./MAX.), PROPER POSITION (F,V,H,OH).	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	E.	WELDING TECHNIQUES: INTERPASS & FINAL CLEANING, EACH PASS WITHIN PROFILE LIMITATIONS, EACH PASS MEETS QUALITY REQUIREMENTS.	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
6		WELDING: INSPECTION TASKS AFTER WELDING							
	A.	WELDS CLEANED	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	B.	SIZE, LENGTH, & LOCATION OF WELDS	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	C.	WELDS MEET VISUAL ACCEPTANCE CRITERIA: CRACK PROHIBITION, WELD/BASE-METAL FUSION, CRATER CROSS SECTION, WELD PROFILES, WELD SIZE, UNDERCUT, POROSITY	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	PERFORM ULTRASONIC TESTING OF ALL FULL PENETRATION FIELD & SHOP WELDS TO COMPLY WITH ASTM E 164 PER PROJECT SPECIFICATIONS.	SIER
	D.	ARC STRIKES	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	E.	K-AREA	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	WHEN WELDING DOUBLER PLATES, CONTINUITY PLATES, OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 IN. OF THE WELD.	SIER
	F.	BACKING REMOVED & WELD TABS REMOVED (IF REQUIRED)	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	G.	REPAIR ACTIVITIES	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	H.	DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
7		STEEL ELEMENT OF COMPOSITE CONSTRUCTION PRIOR TO CONCRETE PLACEMENT							
	A.	PLACEMENT & INSTALLATION OF STEEL DECK	N	-	-	AWS D1.3 AISC 360	1705.2	-	-
	B.	PLACEMENT & INSTALLATION OF STEEL HEADED STUD ANCHORS	N	-	-	AWS D1.1 AISC 360	1705.2	-	-
	C.	DOCUMENT ACCEPTANCE OR REJECTION OF STEEL ELEMENTS	N	-	-	AISC 360	1705.2	-	-
8		INSPECTION OF FABRICATORS & FABRICATION PROCEDURES	N	-	-	AISC 360	1704.2.5	-	-
		COLD-FORMED METAL DECK							
1		VERIFY COMPLIANCE OF MATERIALS (ALL DECK AND ACCESSORIES) WITH CONSTRUCTION DOCUMENTS, INCLUDING PROFILES, MATERIAL PROPERTIES, AND BASE METAL THICKNESS AND DOCUMENT ACCEPTANCE OR REJECTION	N	-	-	SDI QA/QC	1705.2.2	-	-
2		VERIFY COMPLIANCE OF INSTALLATION OF ALL DECK AND ACCESSORIES WITH CONSTRUCTION DOCUMENTS AND VERIFY ACCEPTANCE OR REJECTION	N	-	-	SDI QA/QC	1705.2.2	-	-
3		VERIFY DECK MATERIALS ARE REPRESENTED BY THE MILL CERTIFICATIONS THAT COMPLY WITH THE CONSTRUCTION DOCUMENTS	N	-	-	SDI QA/QC	1705.2.2	-	-
4		DECK WELDING							
	A.	WELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE.	N	-	-	SDI QA/QC & applicable AWS Documents		-	-
	B.	MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	C.	MATERIAL IDENTIFICATION (TYPE/GRADE).	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	D.	CHECK WELDING EQUIPMENT	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	E.	USE OF QUALIFIED WELDERS	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	F.	CONTROL & HANDLING OF WELDING CONSUMABLES	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	G.	ENVIRONMENTAL CONDITIONS: WIND SPEED WITHIN LIMITS, PRECIPITATION, & TEMPERATURE.	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	H.	WPS FOLLOWED: SETTINGS ON WELDING EQUIPMENT, TRAVEL SPEED, SELECTED WELDING MATERIALS, SHIELDING GAS TYPE/FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED (MIN./MAX.), PROPER POSITION (F,V,H,OH).	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	I.	VERIFY SIZE AND LOCATION OF WELDS, INCLUDING SUPPORT, SIDELAP, AND PERIMETER WELDS	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	J.	WELDS MEET VISUAL ACCEPTANCE CRITERIA: CRACK PROHIBITION, WELD/BASE-METAL FUSION, CRATER CROSS SECTION, WELD PROFILES, WELD SIZE, UNDERCUT, POROSITY	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	K.	VERIFY REPAIR ACTIVITIES	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	L.	DOCUMENT ACCEPTANCE OR REJECTION OF WELDS	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
5		DECK MECHANICAL FASTENING							
	A.	MANUFACTURER INSTALLATION INSTRUCTIONS AVAILABLE FOR MECHANICAL FASTENERS	N	-	-	SDI QA/QC	1705.2.2	-	-
	B.	PROPER TOOLS AVAILABLE FOR FASTENER INSTALLATION	N	-	-	SDI QA/QC	1705.2.2	-	-
	C.	PROPER STORAGE FOR MECHANICAL FASTENERS	N	-	-	SDI QA/QC	1705.2.2	-	-
	D.	FASTENERS ARE POSITIONED AS REQUIRED	N	-	-	SDI QA/QC	1705.2.2	-	-
	E.	FASTENERS ARE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS	N	-	-	SDI QA/QC	1705.2.2	-	-
	F.	CHECK SPACING, TYPE, AND INSTALLATION OF SUPPORT FASTENERS.	N	-	-	SDI QA/QC	1705.2.2	-	-
	G.	CHECK SPACING, TYPE, AND INSTALLATION OF SIDELAP FASTENERS.	N	-	-	SDI QA/QC	1705.2.2	-	-
	H.	CHECK SPACING, TYPE, AND INSTALLATION OF PERIMETER FASTENERS.	N	-	-	SDI QA/QC	1705.2.2	-	-
	I.	VERIFY REPAIR ACTIVITIES	N	-	-	SDI QA/QC	1705.2.2	-	-
	J.	DOCUMENT ACCEPTANCE OR REJECTION OF MECHANICAL FASTENERS	N	-	-	SDI QA/QC	1705.2.2	-	-
		OPEN-WEB STEEL JOISTS AND JOIST GIRDERS							
1		END CONNECTIONS - WELDING OR BOLTED	N	-	-	SJI Specifications	1705.2.3	-	-
2		BRIDGING - HORIZONTAL OR DIAGONAL							
	A.	STANDARD BRIDGING	N	-	-	SJI Specifications	1705.2.3	-	-
	B.	BRIDGING THAT DIFFERS FROM THE SJI SPECIFICATIONS	N	-	-	Construction documents & approved shop drawings	1705.2.3	-	-
		CONCRETE							
1		INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, & PLACEMENT.	Y	-	X	ACI 318	1705.3	VERIFY SIZE, LOCATION, SPACING ORIENTATION, COVER, SPLICING, & CONFORMANCE WITH THE CONTRACT DOCUMENTS, AS SUPPLEMENTED WITH APPROVED SHOP DRAWINGS OR OTHER SUBMITTALS. CONFIRM THAT THE SURFACE OF THE REINFORCING STEEL IS FREE OF FORM RELEASE OIL OR OTHE	SIER
2		INSPECTION OF REINFORCING STEEL WELDING.							
	A.	VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706.	N	-	-	AWS D1.4 ACI 318	1705.3	-	-
	B.	INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"	N	-	-	AWS D1.4 ACI 318	1705.3	-	-
	C.	INSPECT ALL OTHER WELDS	N	-	-	AWS D1.4 ACI 318	1705.3	-	-
3		INSPECT ANCHORS CAST-IN CONCRETE.	Y	-	X	ACI 318	1705.3	PRECISE LOCATION OF ANCHOR RODS IS NOT EXPECTED BUT VERIFY THE CONTRACTOR HAS TAKEN APPROPRIATE STEPS TO CORRECTLY POSITION THEM SUCH AS ENGAGING A SURVEYOR OR SETTING UP A SYSTEM OF STRING LINES & BATTER BOARDS & THAT CORRECT GRADE & SIZE OF ANCHORS IS	SIER

30x42 SCHEDULE OF SPECIAL INSPECTIONS

		VERIFICATION AND INSPECTION	Y/N	FREQUENCY CONTINUOUS	FREQUENCY PERIODIC	REFERENCED STANDARD	IBC REFERENCE	SCOPE OF SERVICE	RESPONSIBLE PARTY
4		INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE.							
	A.	ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS	Y	X	-	ACI 318		0	
	B.	MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED ABOVE	Y	-	X	ACI 318		0	
5		VERIFYING USE OF REQUIRED DESIGN MIX.	Y	-	X	ACI 318	1705.3	VERIFY APPROVED MIX DESIGN	SIER
6		AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM, SLUMP & AIR CONTENT TESTS, & DETERMINE THE TEMPERATURE OF THE CONCRETE.	Y	X	-	ASTM C 172/ASTM C 31/ACI 318	1705.3	TEST IN ACCORDANCE WITH PROJECT SPECIFICATIONS, BUT NOT LESS THAN ONCE PER DAY PER CLASS OF CONCRETE OR ONCE PER 150 CUBIC YARDS PER DAY OR ONCE PER 5,000 SQUARE FEET OF SLAB OR WALL PER DAY. CYLINDERS MUST BE PROPERLY HANDLED & STORED ON SITE UNTIL TRANS	SIER
7		INSPECTION OF CONCRETE & SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	Y	X	-	ACI 318	1705.3	VERIFY CONFORMANCE WITH PROJECT SPECIFICATIONS. INSPECTOR SHALL BE WHERE THE CONCRETE IS BEING PLACED RATHER THAN WHERE CONCRETE TRUCKS ARE DISCHARGING THEIR LOADS. INSPECTOR NEEDS TO BE PRESENT WHILE SLAB IS BEING FLOATED & POWER TROWELED.	SIER
8		INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE & TECHNIQUES.	Y	-	X	ACI 318	1705.3	VERIFY CONFORMANCE WITH PROJECT SPECIFICATIONS & ACI	SIER
9		INSPECTION OF PRESTRESSED CONCRETE:							
	A.	APPLICATION OF PRESTRESSING FORCES.	N	-	-	ACI 318	1705.3	-	-
	B.	GROUTING OF BONDED PRESTRESSING TENDONS.	N	-	-	ACI 318	1705.3	-	-
10		ERECTION OF PRECAST CONCRETE MEMBERS.	N	-	-	ACI 318	1705.3	-	-
11		VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POSTTENSIONED CONCRETE & PRIOR TO REMOVAL OF SHORES & FORMS FROM BEAMS & STRUCTURAL SLABS.	N	-	-	ACI 318	1705.3	-	-
12		INSPECT FORMWORK FOR SHAPE, LOCATION & DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	N	-	-	ACI 318	1705.3	-	-
		MASONRY							
		LEVEL A QUALITY ASSURANCE: RISK CATEGORY I, II, OR III STRUCTURES DESIGNED IN ACCORDANCE WITH PART 4 OR APPENDIX A.	Y				1705.4		
		LEVEL B QUALITY ASSURANCE: RISK CATEGORY IV STRUCTURES DESIGNED IN ACCORDANCE WITH CHAPTERS 12 OR 13 & RISK CATEGORY I, II, OR III STRUCTURES DESIGNED IN ACCORDANCE WITH CHAPTERS OTHER THAN THOSE IN PART 4 OR APPENDIX A.	N				1705.4		
		LEVEL C QUALITY ASSURANCE: RISK CATEGORY IV STRUCTURES DESIGNED IN ACCORDANCE WITH CHAPTERS OTHER THAN PART 4 OR APPENDIX A.	N				1705.4		
1		PRIOR TO CONSTRUCTION VERIFY CERTIFICATES OF COMPLIANCE USED IN MASONRY CONSTRUCTION AND DURING CONSTRUCTION COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS & THE APPROVED SUBMITTALS SHALL BE VERIFIED.	Y	-	X	ACI530.1	1705.4	VERIFY COMPLIANCE WITH APPROVED SHOP DRAWINGS.	SIER
2		VERIFICATION OF F'M & F'AAC PRIOR TO CONSTRUCTION (AND FOR EVERY 5,000 SQUARE FEET DURING CONSTRUCTION FOR LEVEL C).	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
3		VERIFICATION OF PROPORTIONS OF MATERIALS IN PREMIXED OR PREBLENDED MORTAR, PRESTRESSING GROUT, & GROUT OTHER THAN SELF-CONSOLIDATING GROUT AS DELIVERED TO THE SITE OR PROPORTIONS OF SITE PREPARED MORTAR.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
4		VERIFICATION OF SLUMP FLOW & VSI AS DELIVERED TO THE SITE FOR SELF-CONSOLIDATING GROUT.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
5		THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:							
	A.	PROPORTIONS OF SITE-PREPARED MORTAR, GROUT, & PRESTRESSING GROUT FOR BONDED TENDONS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	B.	PLACEMENT OF MASONRY UNITS & CONSTRUCTION OF MORTAR JOINTS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	C.	GRADE, TYPE, & SIZE OF REINFORCEMENT, ANCHOR BOLTS, PRESTRESSING TENDONS, & ANCHORAGES	N	-	-	TMS 402/ACI 530/ASCE 5/TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	D.	PLACEMENT OF REINFORCEMENT, CONNECTORS & PRESTRESSING TENDONS & ANCHORAGES.	N	-	-	TMS 402/ACI 530/ASCE 5/TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	E.	GROUT SPACE PRIOR TO GROUTING.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	F.	PLACEMENT OF GROUT & PRESTRESSING GROUT FOR BONDED TENDONS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	G.	SIZE & LOCATION OF STRUCTURAL ELEMENTS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	H.	TYPE, SIZE & LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION.	N	-	-	TMS 402/ACI 530/ASCE 5	1705.4	-	-
	I.	WELDING OF REINFORCEMENT.	N	-	-	TMS 402/ACI 530/ASCE 5	1705.4	-	-
	J.	PREPARATION, CONSTRUCTION & PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F).	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	K.	APPLICATION & MEASUREMENT OF PRESTRESSING FORCE.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	L.	PLACEMENT OF AAC MASONRY UNITS & CONSTRUCTION OF THIN-BED MORTAR JOINTS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	M.	PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	N.	VERIFY PRE-STRESSING TECHNIQUE IS IN COMPLIANCE AS CONSTRUCTION BEGINS	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	
6		OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
		WOOD CONSTRUCTION							
1		INSPECTION OF FABRICATORS & FABRICATION PROCEDURES FOR PREFABRICATED WOOD STRUCTURAL ELEMENTS.	Y	-	X		1705.5	-	SIER
2		HIGH-LOAD DIAPHRAGMS DESIGNED IN ACCORDANCE WITH SECTION 2306.2	N	-	X		1705.5	-	SIER
3		METAL PLATE CONNECTED WOOD TRUSSES SPANNING 60 FEET OR MORE	N	-	-		1705.5	-	-
		SOILS						-	
1		VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	Y	-	X	Geotechnical Report	1705.6	-	SIER
2		VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH & HAVE REACHED PROPER MATERIAL	Y	-	X	Geotechnical Report	1705.6	-	SIER
3		PERFORM CLASSIFICATION & TESTING OF COMPACTED FILL MATERIALS.	Y	-	X	Geotechnical Report	1705.6	-	SIER
4		VERIFY USE OF PROPER MATERIALS, DENSITIES, & LIFT THICKNESSES DURING PLACEMENT & COMPACTION OF COMPACTED FILL.	Y	X	-	Geotechnical Report	1705.6	TEST FREQUENCY PER SPECIFICATIONS, BUT NOT LESS THAN ONE TEST EVERY 2,000 SQUARE FEET FOR EACH LAYER OF FILL OR PROOF-ROLLING	SIER
5		PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE & VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	Y	-	X	Geotechnical Report	1705.6	-	SIER
		DRIVEN DEEP FOUNDATIONS							
1		VERIFY ELEMENT MATERIALS, SIZES, & LENGTHS COMPLY WITH THE REQUIREMENTS.	N	-	-		1705.7	-	-
2		DETERMINE CAPACITIES OF TEST ELEMENTS & CONDUCT ADDITIONAL LOAD TESTS, AS REQUIRED.	N	-	-		1705.7	-	-
3		OBSERVE DRIVING OPERATIONS & MAINTAIN COMPLETE & ACCURATE RECORDS FOR EACH ELEMENT.	N	-	-		1705.7	-	-
4		VERIFY PLACEMENT LOCATIONS & PLUMBNESS, CONFIRM TYPE & SIZE OF HAMMER, RECORD NUMBER OF BLOWS PER FOOT OF PENETRATION, DETERMINE REQUIRED PENETRATIONS TO ACHIEVE DESIGN CAPACITY, RECORD TIP & BUTT ELEVATIONS, & DOCUMENT ANY DAMAGE TO FOUNDATION ELEMENT.	N	-	-		1705.7	-	-
5		PERFORM ADDITIONAL INSPECTIONS FOR STEEL ELEMENTS PER STEEL INSPECTION REQUIREMENTS.	N	-	-		1705.7	-	-
6		PERFORM ADDITIONAL INSPECTIONS FOR CONCRETE & CONCRETE-FILLED ELEMENTS PER CONCRETE INSPECTION REQUIREMENTS.	N	-	-		1705.7	-	-
7		PERFORM ADDITIONAL INSPECTIONS FOR SPECIALTY ELEMENTS AS DETERMINED BY THE REGISTERED DESIGN PROFESSIONAL IN CHARGE.	N	-	-		1705.7	-	-
		CAST-IN-PLACE DEEP FOUNDATIONS							
1		OBSERVE DRILLING OPERATIONS & MAINTAIN COMPLETE & ACCURATE RECORDS FOR EACH ELEMENT.	N	-	-		1705.8	-	-
2		VERIFY PLACEMENT LOCATIONS & PLUMBNESS, CONFIRM ELEMENT DIAMETERS, BELL DIAMETERS (IF APPLICABLE), LENGTHS, EMBEDMENT INTO BEDROCK (IF APPLICABLE), & ADEQUATE END-BEARING STRATA CAPACITY. RECORD CONCRETE OR GROUT VOLUMES.	N	-	-		1705.8	-	-
3		PERFORM ADDITIONAL INSPECTIONS FOR CONCRETE ELEMENTS PER CONCRETE INSPECTION REQUIREMENTS.	N	-	-		1705.8	-	-
		HELICAL PILE FOUNDATIONS							
1		RECORD INSTALLATION EQUIPMENT USED, PILE DIMENSIONS, TIP ELEVATIONS, FINAL DEPTH, & FINAL INSTALLATION TORQUE.	N	-	-	Geotechnical Report & Approved Shop Drawings	1705.9	-	-
		SPRAYED FIRE-RESISTANT MATERIALS						-	
1		CONDITIONS OF SUBSTRATES	N	-	-	Approved fire-resistance design	1705.14	-	-
2		THICKNESS OF APPLICATION	N	-	-	Approved fire-resistance design	1705.14	-	-
3		DENSITY IN POUNDS PER CUBIC FOOT	N	-	-	Approved fire-resistance design	1705.14	-	-
4		BOND STRENGTH ADHESION /COHESION	N	-	-	Approved fire-resistance design	1705.14	-	-
5		CONDITION OF FINISHED APPLICATION	N	-	-	Approved fire-resistance design	1705.14	-	-
6		FIRE-RESISTANT PENETRATIONS & JOINTS	N	-	-	Approved fire-resistance design	1705.17	-	-
		LIGHT GAGE METAL FRAMING	N	-	-	Approved shop drawings	-	-	-
		MASTIC & INTUMESCENT FIRE-RESISTANT COATINGS	N	-	-	AWCI-12B & approved fire-resistance design	1705.15	-	-
		EXTERIOR INSULATION & FINISH SYSTEMS (EIFS)	N	-	-	-	1705.16	-	-

*	INSPECTION AGENTS	NAME	COMPANY	ADDRESS
	1. SPECIAL INSPECTION ENGINEER OF RECORD (SIER)	1.	1.	1.
	2. INSPECTION & TESTING AGENCY (ITA)	2.	2.	2.
	3. GEOTECHNICAL ENGINEER OF RECORD (GER)	3.	3.	3.
	4. SPECIALTY ENGINEER (SE)	4.	4.	4.
	5. STRUCTURAL ENGINEER OF RECORD (SER)	5. THOMAS A. BOUFFARD, PE	5. EHLERT BRYAN	5. 8609 WESTWOOD CENTER DRIVE, SUITE 800, TYSONS, VA 22182

ALABAMA LICENSED No. 22545 PROFESSIONAL ENGINEER THOMAS A. BOUFFARD

REVISIONS
PERMIT 06/11/2021

PERMIT SET

TERRACES AT HIGH MOUNTAIN
4130 HIGH MOUNTAIN ROAD NE
HUNTSVILLE, AL 35811
06/11/2021

S5.002
SPECIAL INSPECTIONS
01/08/21

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KEY PLAN
SCALE: 1" = 50'-0"

ALABAMA LICENSED No. 22545 PROFESSIONAL ENGINEER THOMAS A. BOUTWELL

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TERRACES AT HIGH MOUNTAIN
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S5.003
KEY PLAN
03/02/21

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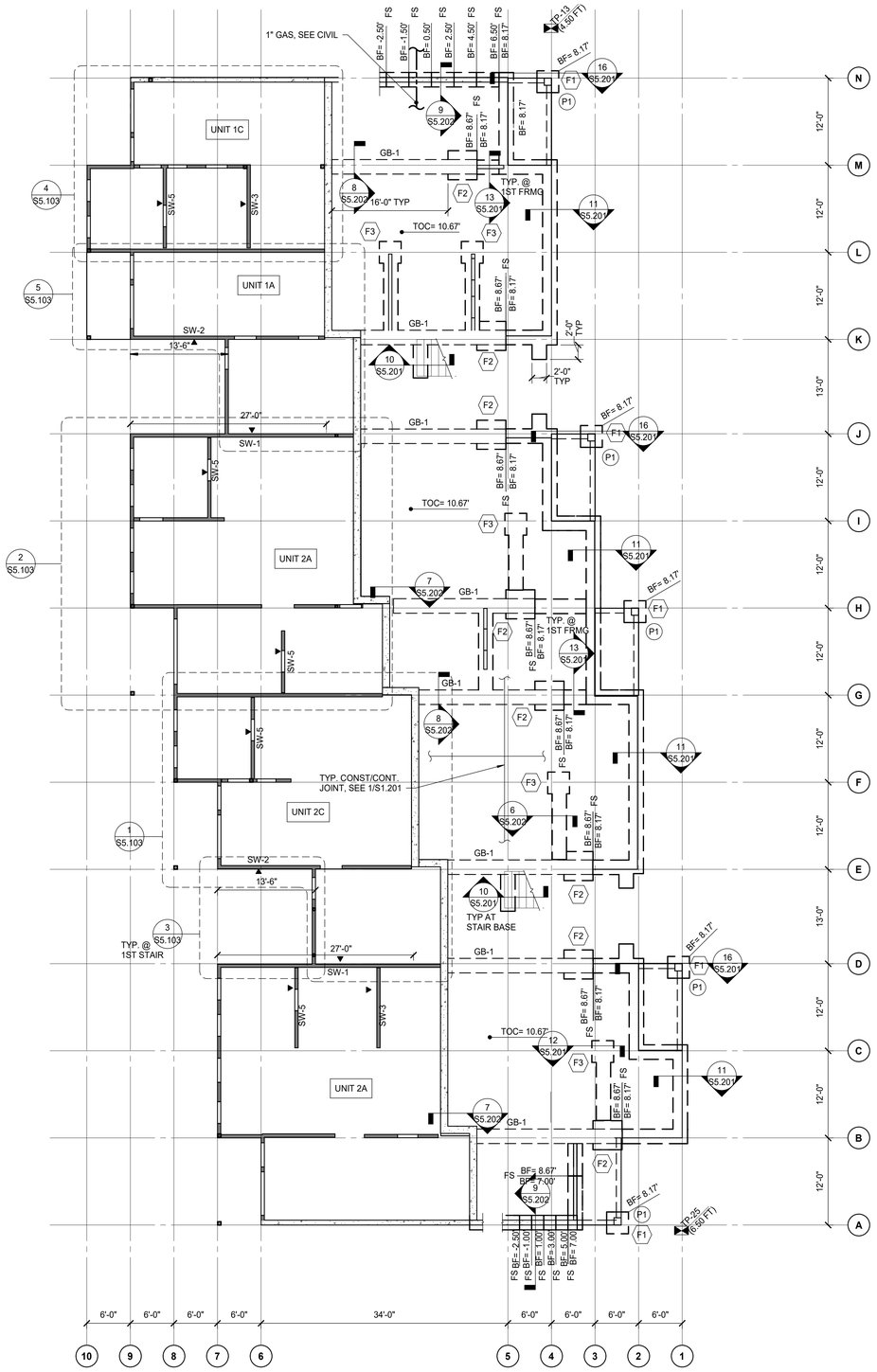
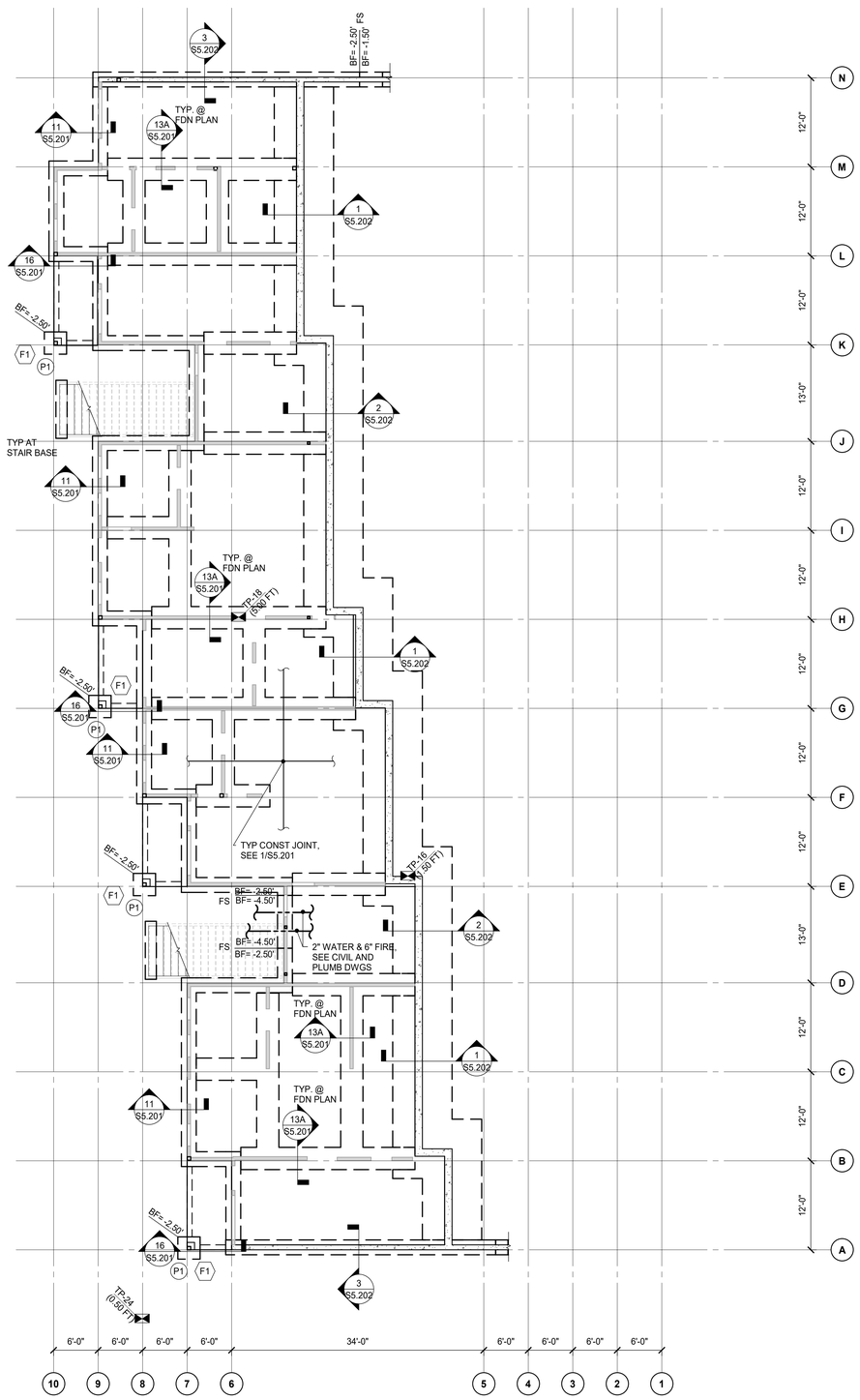


LEVEL 1 FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. FLOOR CONSTRUCTION; 4" CONCRETE SLAB-ON-GRADE REINFORCED WITH ONE LAYER OF 6X6-W1.4 X W1.4 WWF IN THE TOP 1/3 OF SLAB PLACED OVER 10 MIL VAPOR RETARDER ON 4" LAYER OF COMPACTED #57 STONE.
2. TOP OF CONCRETE ELEVATION = 10.67' (ACTUAL EL = 1060.07')
3. INTERIOR BOTTOM OF FOOTING ELEVATION VARIES, SEE PLAN.
4. EXTERIOR BOTTOM OF FOOTING ELEVATION VARIES, SEE PLAN.
5. TOP OF SHEATHING ELEVATION = 10.60'
6. FS DENOTES FOOTING STEP, SEE 2/S5.201.
7. TP-X DENOTES APPROXIMATE TEST PIT LOCATION
8. SEE UNIT FRAMING PLANS FOR ELEVATED FLOOR CONSTRUCTION NOTES.
9. GB-1 DENOTES 24"W x 24"D GRADE BEAM W/ 3-#6 CONTINUOUS TOP BARS, 4-#8 CONTINUOUS BOTTOM BARS, AND #3 STIRRUPS @ 10" OC.
10. REFERENCE NOTE 2.D AND 2.E ON S5.001 FOR INFORMATION REGARDING BUILDING PAD REQUIREMENTS. SEE GEOTECHNICAL REPORT FOR MORE INFORMATION.
11. SW-X DENOTES SHEAR WALL. ▶ DENOTES SIDE OF WALL TO BE SHEATHED. S5.403 FOR SHEAR WALL SCHEDULE AND TYPICAL DETAILS. ⟵XX'XX"⟶ DENOTES SHEAR WALL EXTENTS.



FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. FLOOR CONSTRUCTION; 4" CONCRETE SLAB-ON-GRADE REINFORCED WITH ONE LAYER OF 6X6-W1.4 X W1.4 WWF IN THE TOP 1/3 OF SLAB PLACED OVER 10 MIL VAPOR RETARDER ON 4" LAYER OF COMPACTED #57 STONE.
2. TOP OF CONCRETE ELEVATION = 0.00' REFERENCE (ACTUAL ELEVATION= 1049.40')
3. INTERIOR BOTTOM OF FOOTING ELEVATION = -2.50' TYP, UNO.
4. EXTERIOR BOTTOM OF FOOTING ELEVATION = -2.50' TYP, UNO.
5. TP-X DENOTES APPROXIMATE TEST PIT LOCATION.(XX') DENOTES ESTIMATED UNDERCUTS TO SI
6. REFERENCE NOTE 2.D AND 2.E ON S5.001 FOR INFORMATION REGARDING BUILDING PAD REQUIREMENTS. SEE GEOTECHNICAL REPORT FOR MORE INFORMATION.
7. FS DENOTES FOOTING STEP, SEE 2/S5.201.

ALABAMA LICENSED PROFESSIONAL No. 22545 ENGINEER THOMAS A. BOUFFARD

REVISIONS
PERMIT 06/11/2021

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TERRACES AT HIGH MOUNTAIN

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06/11/2021

S5.100

FOUNDATION PLAN AND LEVEL 1 FRAMING PLANS

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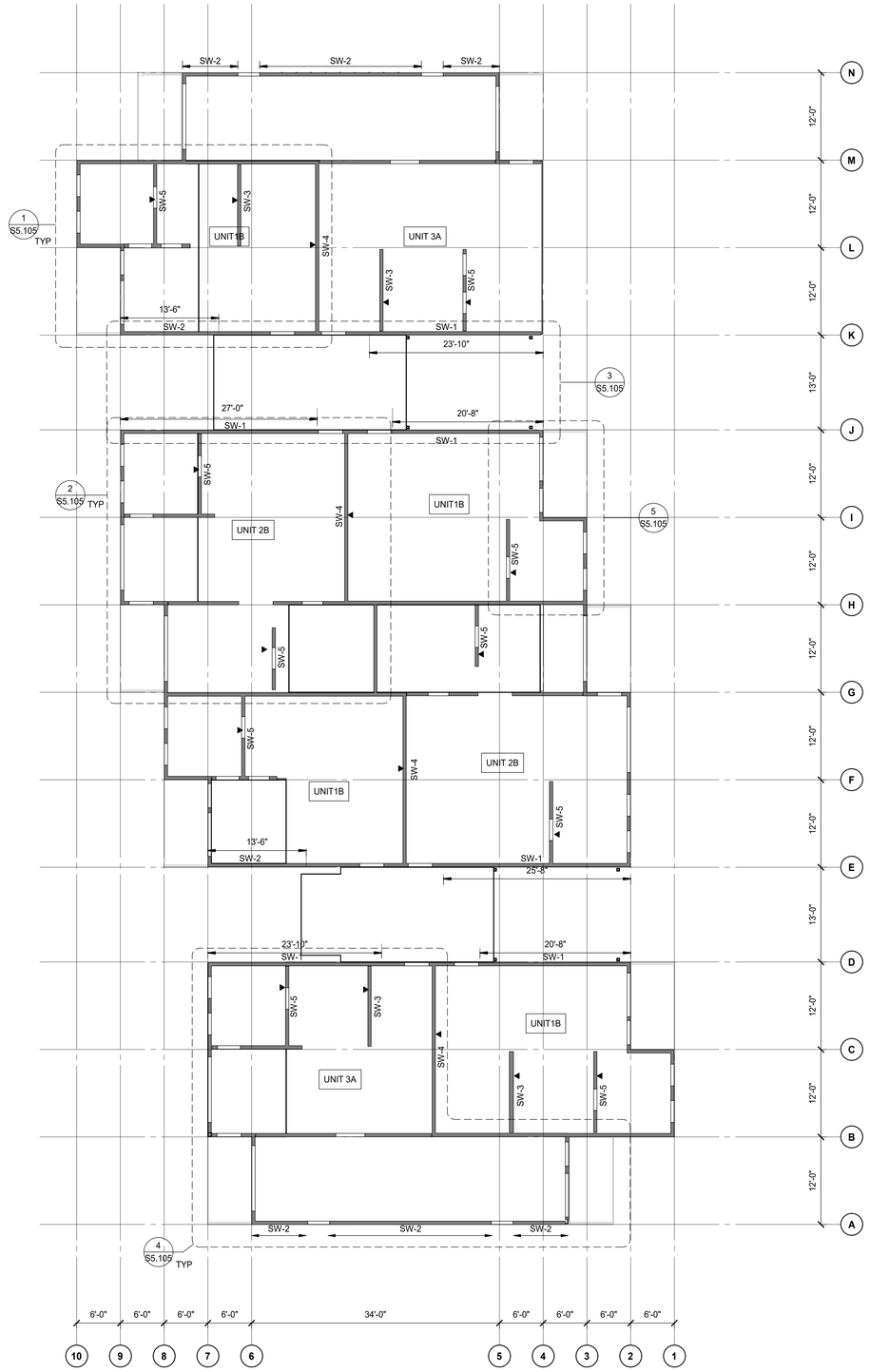
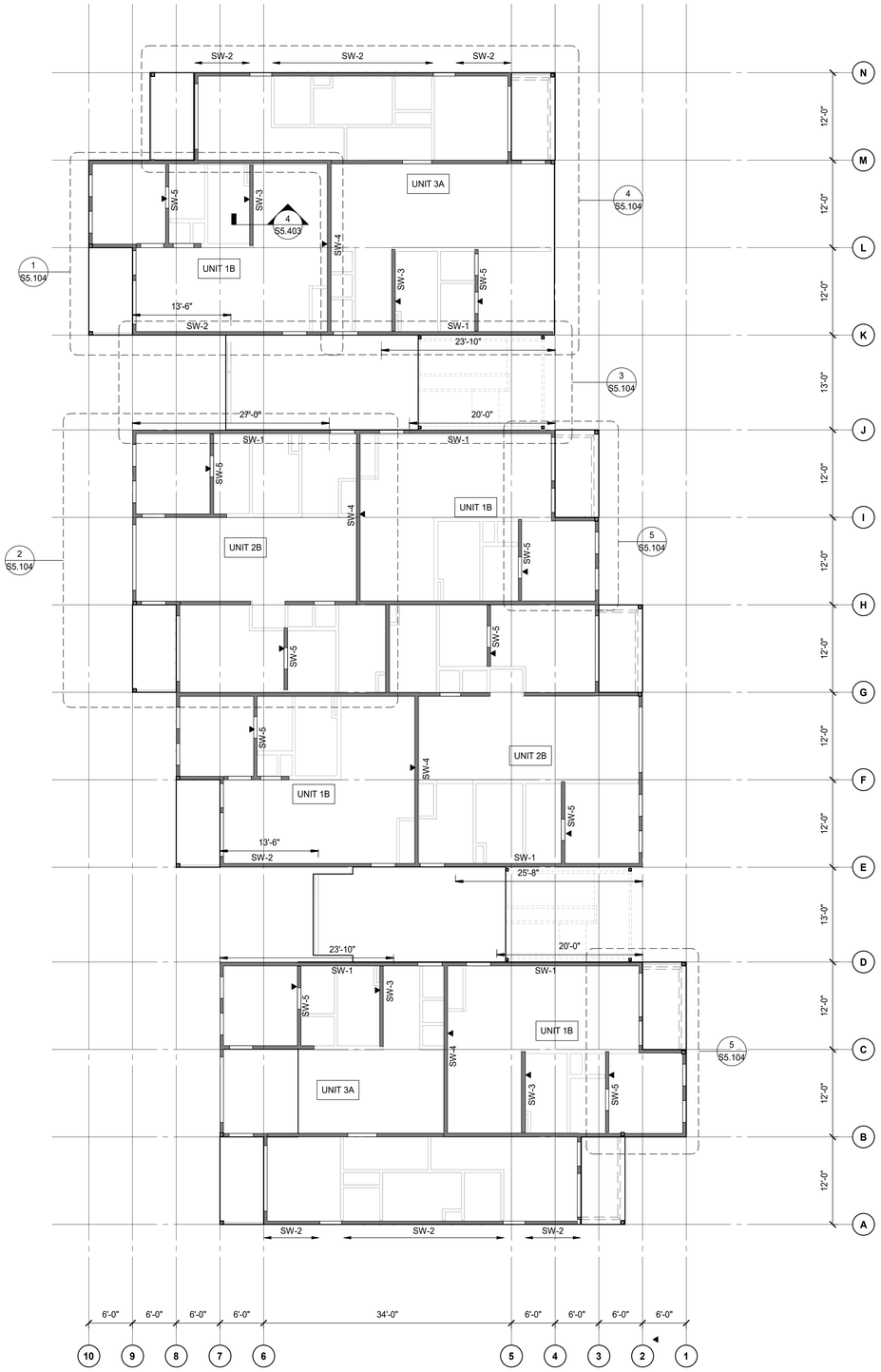


LEVEL 3 FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. SEE UNIT FRAMING PLANS FOR ELEVATED FLOOR CONSTRUCTION NOTES.
2. 3RD FLOOR INTERIOR TOP OF SHEATHING ELEVATION = 32.00' TYP.
3. SW-X DENOTES SHEAR WALL ▶ DENOTES SIDE OF WALL TO BE SHEATHED. SEE S5.403 FOR SHEAR WALL SCHEDULE AND TYPICAL DETAILS. ⟵ XX'XX" ⟶ DENOTES SHEAR WALL EXTENTS.



LEVEL 2 FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. SEE UNIT FRAMING PLANS FOR ELEVATED FLOOR CONSTRUCTION NOTES.
2. 2ND FLOOR TOP OF SHEATHING ELEVATION = 21.33'.
3. SW-X DENOTES SHEAR WALL. ▶ DENOTES SIDE OF WALL TO BE SHEATHED. SEE S5.403 FOR SHEAR WALL SCHEDULE AND TYPICAL DETAILS. ⟵ XX'XX" ⟶ DENOTES SHEAR WALL EXTENTS.

REVISIONS

PERMIT 06/11/2021

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TERRACES AT HIGH MOUNTAIN

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S5.101

LEVEL 2 AND 3 FRAMING PLANS

12/14/20

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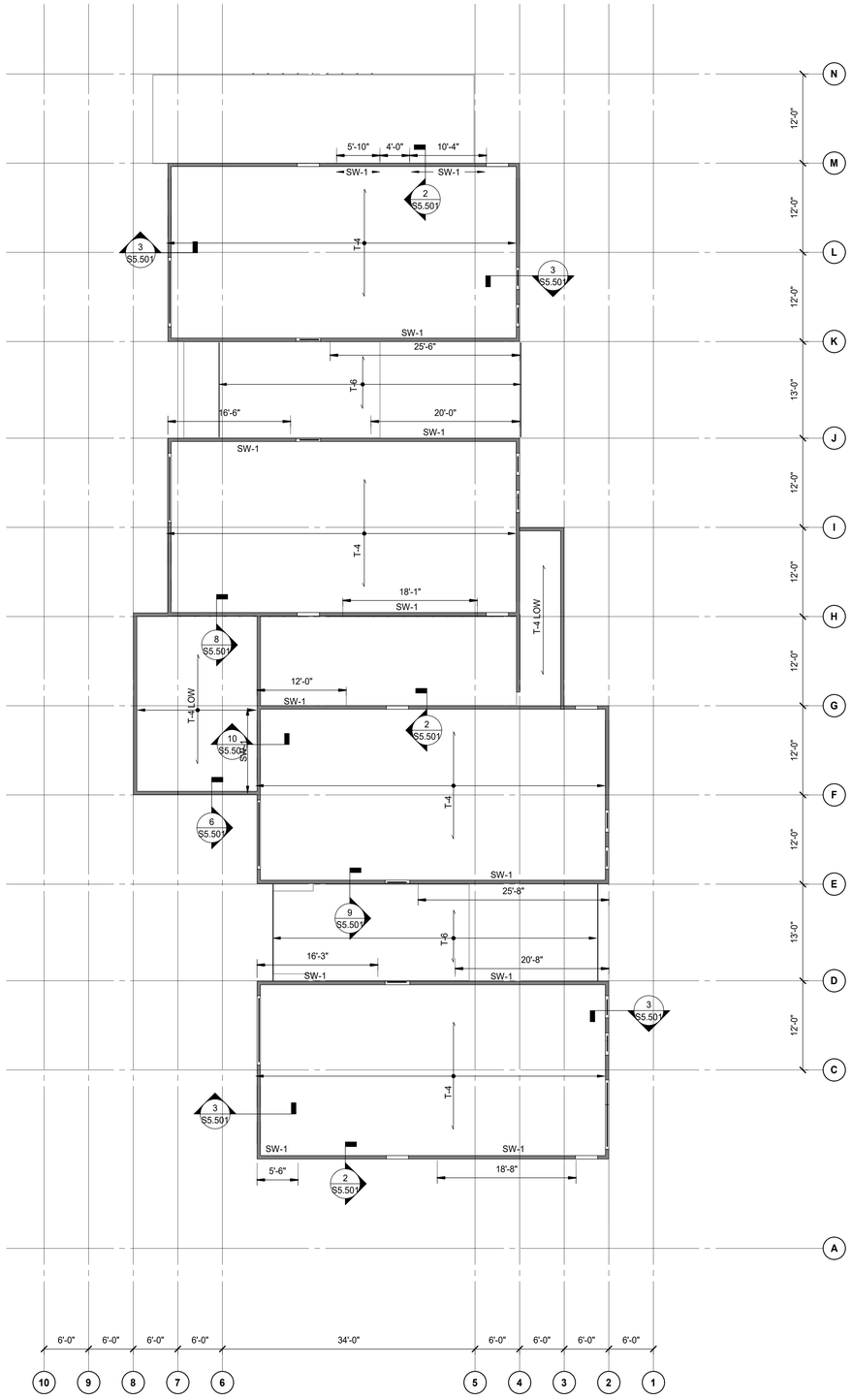

ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. ROOF CONSTRUCTION 3/4" STRUCTURAL SHEATHING ON PRE-ENGINEERED ROOF WOOD TRUSSES @ 24" O.C.
2. ROOF TOP OF SHEATHING ELEVATION VARIES, SEE ARCH.
3. TYPICAL SECTIONS SHOWN ARE APPLICABLE TO SIMILAR CONDITIONS EVEN IF MARKS ARE NOT SHOWN.
4. SEE S5.401 FOR TYPICAL HEADERS AND STUDS AT OPENINGS U.N.O.
5. SEE S5.401 FOR COLUMN BEAM & SCHEDULE
6. SW-X DENOTES SHEAR WALL. ▶ DENOTES SIDE OF WALL TO BE SHEATHED.
   SEE S5.403 FOR SHEAR WALL SCHEDULE AND TYPICAL DETAILS.
   ⟵ XX'XX" ⟶ DENOTES SHEAR WALL EXTENTS.

ALABAMA LICENSED No. 22545 PROFESSIONAL ENGINEER THOMAS J. MOUTTON

REVISIONS
PERMIT 06/11/2021

PERMIT SET

TERRACES AT HIGH MOUNTAIN

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S5.102
ROOF FRAMING PLAN

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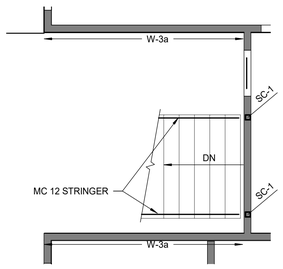
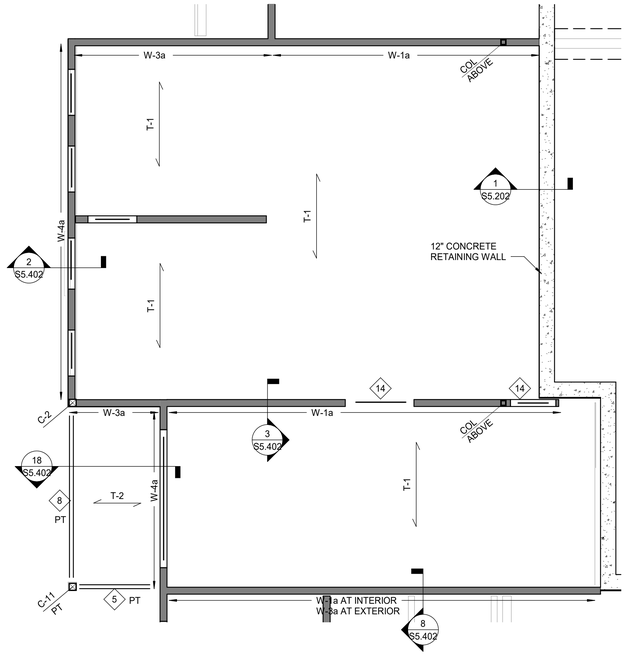
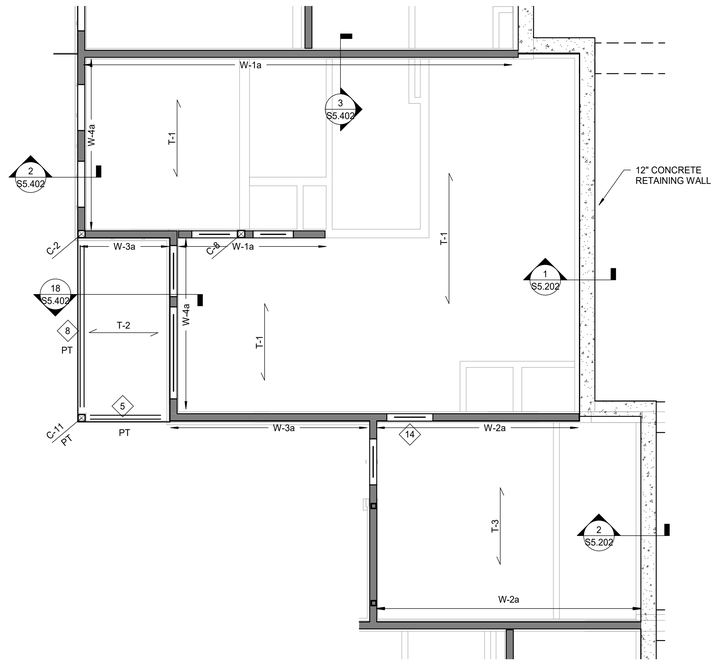


LEVEL 1 BREEZEWAY
SCALE: 1/4" = 1'-0"
3 S5.103



LEVEL 1 UNIT 2A
SCALE: 1/4" = 1'-0"
2 S5.103



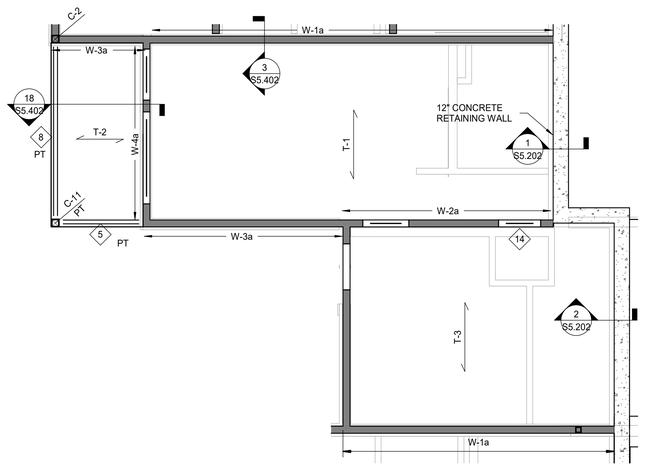
LEVEL 1 UNIT 2C
SCALE: 1/4" = 1'-0"
1 S5.103

LEVEL 1 UNIT FRAMING GENERAL NOTES:

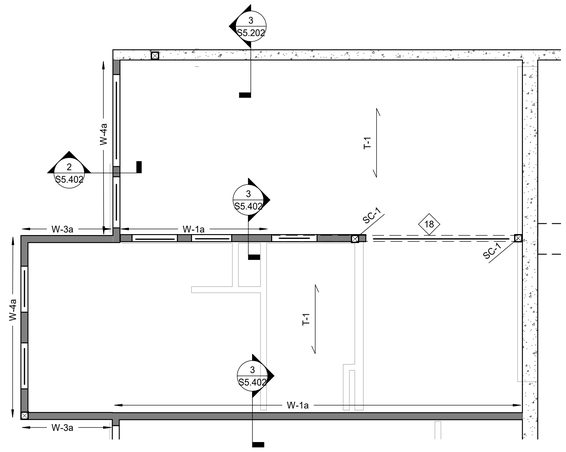
NOTES:

1. TYPICAL FLOOR CONSTRUCTION SHALL BE AS FOLLOWS: UP TO 1" THICK GYPCRETE TOPPING OVER 23/32" (3/4") APA RATED STURDI-FLOOR TONGUE AND GROOVE PANELS GLUED AND SCREWED TO 18" TRUSSES OVER 2x WOOD BEARING WALLS. SEE S5.401 FOR SCHEDULE AND TRUSS LOADING INFORMATION.
2. TOP OF SHEATHING = 10.60' REFERENCE @ LEVEL 1.
3. SEE S5.401 FOR TYPICAL HEADERS, STUDS AT OPENINGS, BEAM AND COLUMN SCHEDULES.
4. DENOTES BEARING WALL.
5. SEE LEVEL 1 FRAMING PLAN ON S5.100 FOR SHEAR WALL LOCATIONS AND NOTES.
6. TYPICAL CONDITIONS ARE APPLICABLE EVEN IFSECTIONS ARE NOT SHOWN.

TRUSS/JOIST SHOP DRAWING SUBMITTAL SHALL BE COORDINATED WITH AND SHALL SHOW ALL BATHTUB, SHOWER AND TOILET DRAINS AND ALL MECHANICAL SHAFTS. ADJUST JOIST SPACING AND/OR ADD JOISTS AND HEADERS TO CLEAR PLUMBING & MECHANICAL.



LEVEL 1 UNIT 1A
SCALE: 1/4" = 1'-0"
5 S5.103



LEVEL 1 UNIT 1C
SCALE: 1/4" = 1'-0"
4 S5.103

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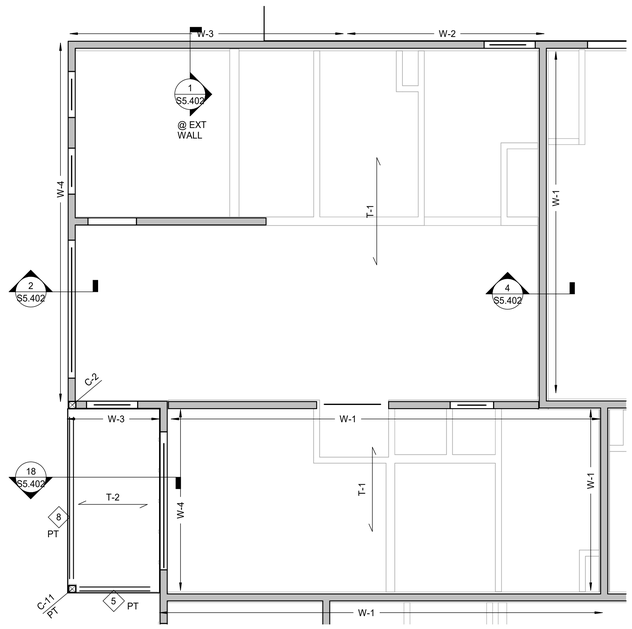
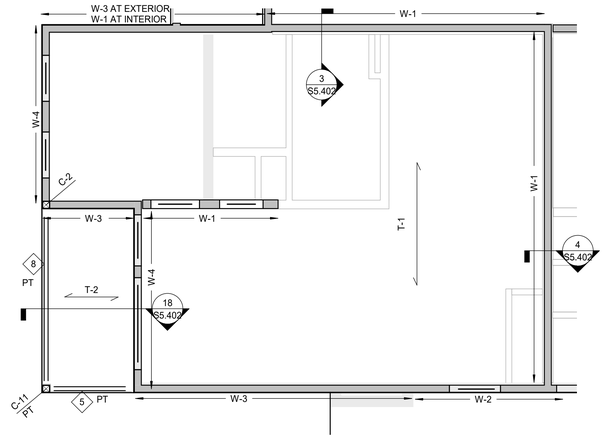
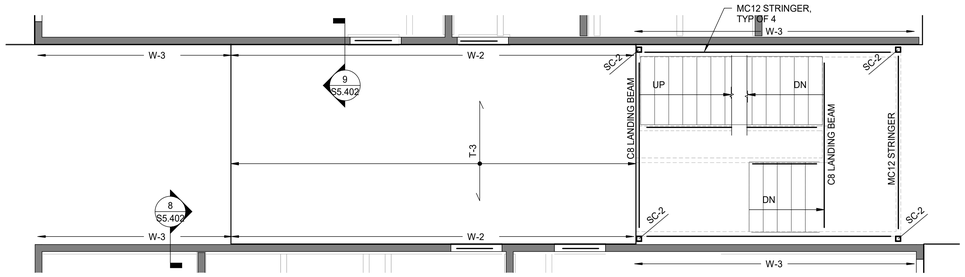


LEVEL 2 UNIT 2B
SCALE: 1/4" = 1'-0"
2 S5.104



LEVEL 2 UNIT 1B
SCALE: 1/4" = 1'-0"
1 S5.104



NOTE: PROVIDE STAIR SHOP DRAWINGS DESIGNED BY SPECIALTY ENGINEER. SIZES ARE SHOWN FOR INTENT ONLY.

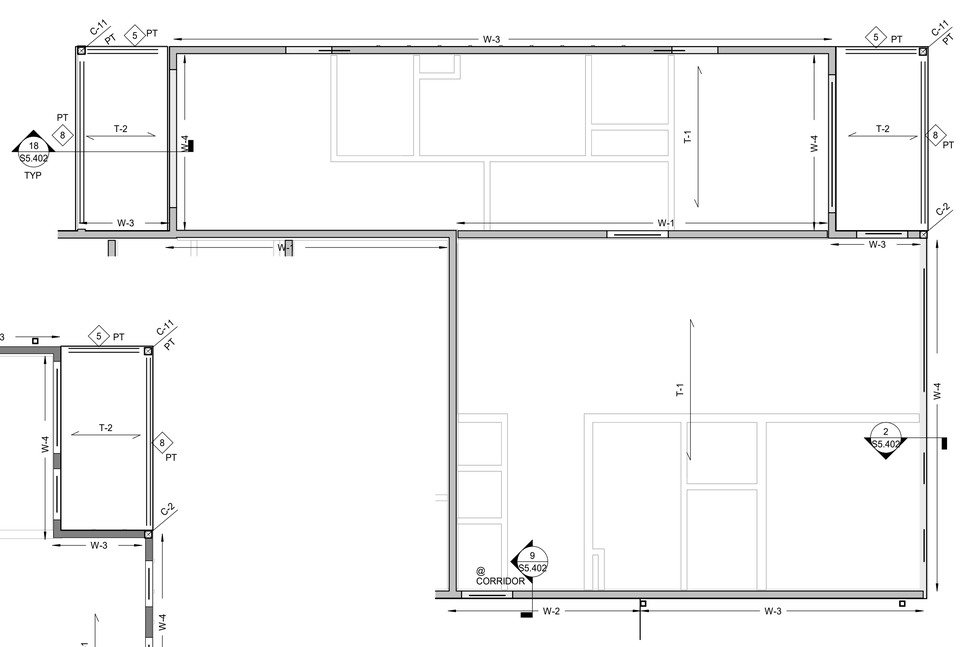
LEVEL 2 BREEZEWAY
SCALE: 1/4" = 1'-0"
3 S5.104

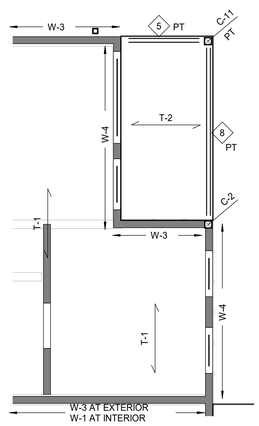
LEVEL 2 UNIT FRAMING GENERAL NOTES:

NOTES:

1. TYPICAL FLOOR CONSTRUCTION SHALL BE AS FOLLOWS: UP TO 1" THICK GYPCRETE TOPPING OVER 23/32" (3/4") APA RATED STURDI-FLOOR TONGUE AND GROOVE PANELS GLUED AND SCREWED TO 18" TRUSSES OVER 2x WOOD BEARING WALLS. SEE S5.401 FOR SCHEDULE AND TRUSS LOADING INFORMATION.
2. TOP OF SHEATHING = 21.33' REFERENCE @ LEVEL 2.
3. SEE S5.401 FOR TYPICAL HEADERS, STUDS AT OPENINGS, BEAM AND COLUMN SCHEDULES.
4. ▭ DENOTES BEARING WALL.
5. SEE LEVEL 2 FRAMING PLAN ON S5.02 FOR SHEAR WALL LOCATIONS AND NOTES.
6. TYPICAL CONDITIONS ARE APPLICABLE EVEN IFSECTIONS ARE NOT SHOWN.

TRUSS/JOIST SHOP DRAWING SUBMITTAL SHALL BE COORDINATED WITH AND SHALL SHOW ALL BATHTUB, SHOWER AND TOILET DRAINS AND ALL MECHANICAL SHAFTS. ADJUST JOIST SPACING AND/OR ADD JOISTS AND HEADERS TO CLEAR PLUMBING & MECHANICAL.





LEVEL 2 PARTIAL BALCONY
SCALE: 1/4" = 1'-0"
5 S5.104

LEVEL 2 UNIT 3A
SCALE: 1/4" = 1'-0"
4 S5.104

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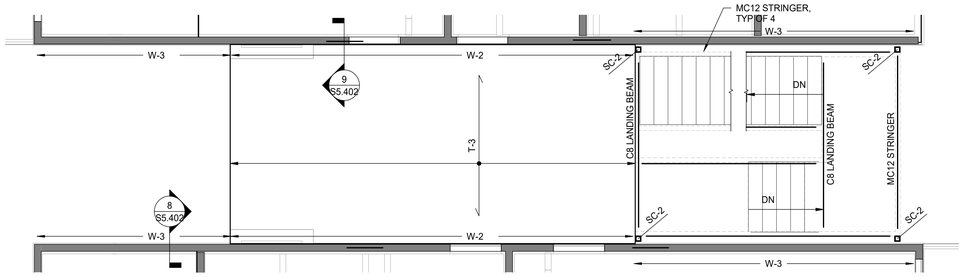
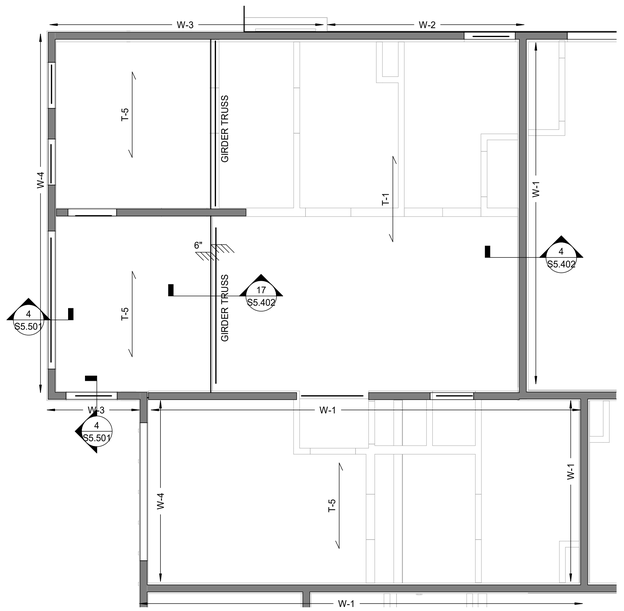
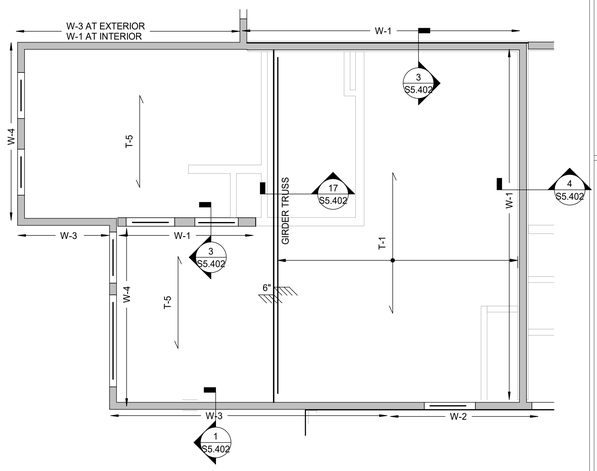


NOTE: PROVIDE STAIR SHOP DRAWINGS DESIGNED BY SPECIALTY ENGINEER. SIZES ARE SHOWN FOR INTENT ONLY.

LEVEL 3 BREEZEWAY
SCALE: 1/4" = 1'-0"
3 S5.105



LEVEL 3UNIT 2B
SCALE: 1/4" = 1'-0"
2 S5.105



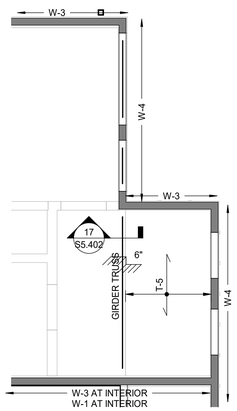
LEVEL 3 UNIT 1B
SCALE: 1/4" = 1'-0"
1 S5.105

LEVEL 3 UNIT FRAMING GENERAL NOTES:

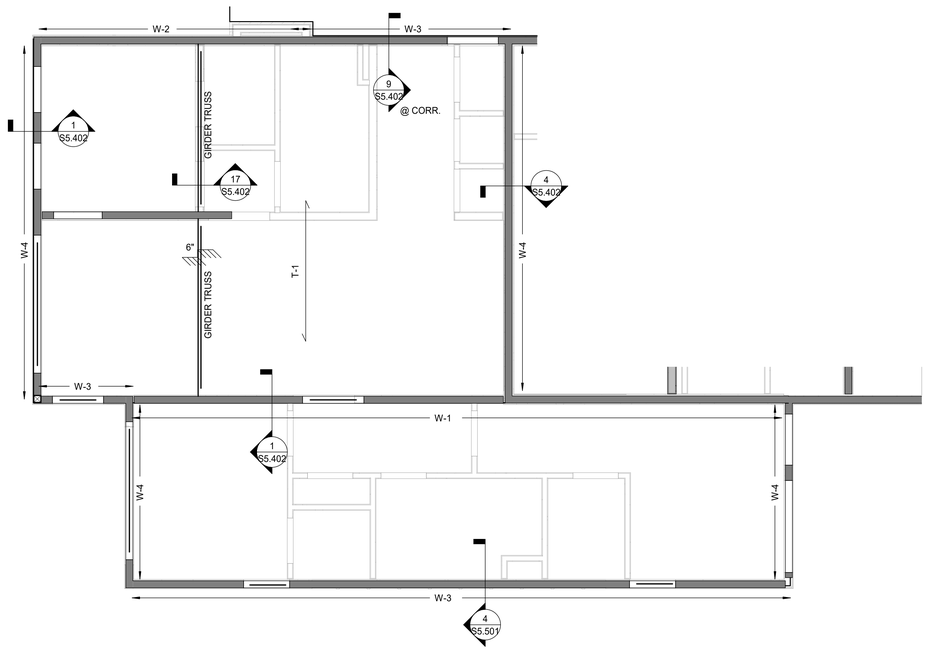
NOTES:

1. TYPICAL FLOOR CONSTRUCTION SHALL BE AS FOLLOWS: UP TO 1" THICK GYPCRETE TOPPING OVER 23/32" (3/4") APA RATED STURDI-FLOOR TONGUE AND GROOVE PANELS GLUED AND SCREWED TO 18" TRUSSES OVER 2x WOOD BEARING WALLS. SEE S5.401 FOR SCHEDULE AND TRUSS LOADING INFORMATION.
2. INTERIOR TOP OF SHEATHING = 32.00' @ LEVEL 3 TYP.
3. SEE S5.401 FOR TYPICAL HEADERS, STUDS AT OPENINGS, BEAM AND COLUMN SCHEDULES.
4. ▭ DENOTES BEARING WALL.
5. SEE LEVEL 3 FRAMING PLAN ON S5.03 FOR SHEAR WALL LOCATIONS AND NOTES.
6. TYPICAL CONDITIONS ARE APPLICABLE EVEN IFSECTIONS ARE NOT SHOWN.

TRUSS/JOIST SHOP DRAWING SUBMITTAL SHALL BE COORDINATED WITH AND SHALL SHOW ALL BATHTUB, SHOWER AND TOILET DRAINS AND ALL MECHANICAL SHAFTS. ADJUST JOIST SPACING AND/OR ADD JOISTS AND HEADERS TO CLEAR PLUMBING & MECHANICAL.



LEVEL 3 PLAN PARTIAL PLAN
SCALE: 1/4" = 1'-0"
5 S5.105



LEVEL 3 UNIT 3A
SCALE: 1/4" = 1'-0"
4 S5.105

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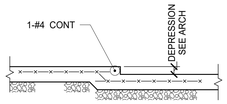
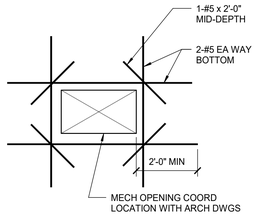


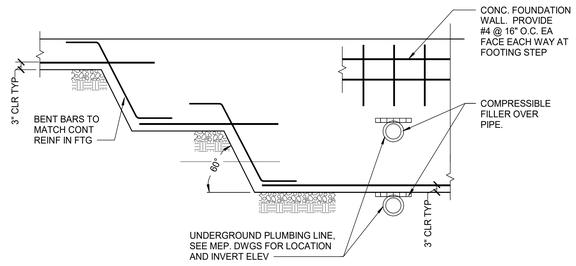
TYPICAL FLOOR DEPRESSION

SECTION 4 S5.201
SCALE: 1/2" = 1'-0"



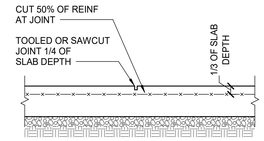
TYPICAL OPENING REINFORCEMENT @ SLAB ON GRADE BLOCK-OUTS

SECTION 3 S5.201
SCALE: 1/2" = 1'-0"

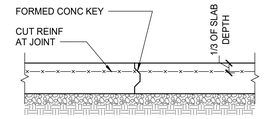


TYPICAL FOOTING STEP (F.S.)

SECTION 2 S5.201
SCALE: 1/2" = 1'-0"



TYPICAL SLAB CONTROL JOINT

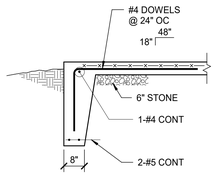


TYPICAL SLAB CONSTRUCTION JOINT

SECTION 1 S5.201
SCALE: 1/2" = 1'-0"

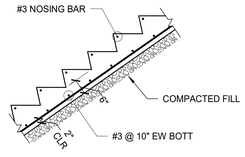
FOOTING SCHEDULE			
MARK	SIZE	BOTTOM REINFORCING	REMARKS
F1	3'-0"x3'-0"x12"	3-#4 BOTT E.W.	
F2	4'-0"x4'-0"x12"	4-#4 BOTT E.W.	
F3	3'-0"x3'-0"x24"	3-#4 BOTT E.W.	

PIER SCHEDULE				
		REINFORCING		
MARK	SIZE	VERTICAL	TIES	REMARKS
P1	12" x 12"	4-#5 VERT	#3 @ 6" OC	



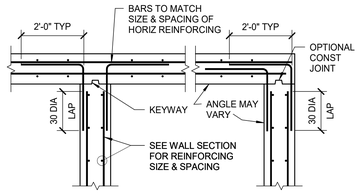
TURN DOWN SLAB EDGE

SECTION 9 S5.201
SCALE: 1/2" = 1'-0"



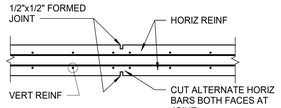
TYPICAL CONCRETE STAIRS

SECTION 8 S5.201
SCALE: 1/2" = 1'-0"

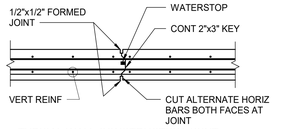


TYPICAL REINFORCING AT WALL INTERSECTIONS

SECTION 7 S5.201
SCALE: 1/2" = 1'-0"



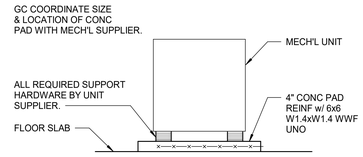
TYPICAL WALL CONTROL JOINT



TYPICAL WALL CONSTRUCTION JOINT

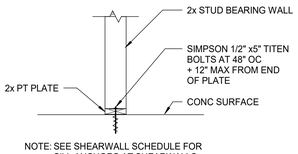
NOTES:
1. PROVIDE CONTROL OR CONSTRUCTION JOINT AT 30'-0" MAX SPACING.
2. LOCATE FIRST JOINT NO FURTHER THAN 15'-0" FROM CORNER.

SECTION 6 S5.201
SCALE: 1/2" = 1'-0"



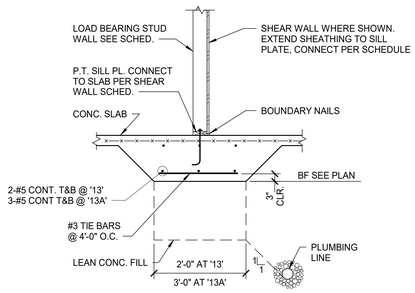
TYPICAL HOUSEKEEPING PAD

SECTION 5 S5.201
SCALE: 1/2" = 1'-0"

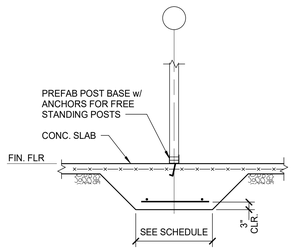


TYPICAL WOOD PLATE TO CONC ANCHORAGE
(EXCEPT SHEAR WALLS)

SECTION 14 S5.201
SCALE: 3/4" = 1'-0"

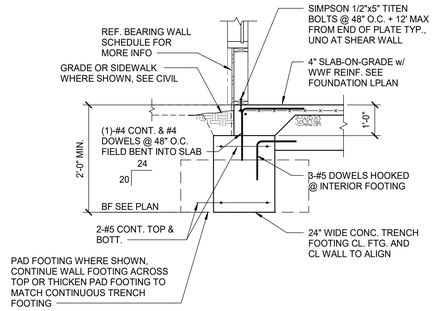


SECTION 13 13A S5.201 S5.201
SCALE: 1/2" = 1'-0"

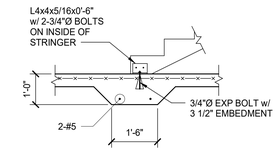


INTERIOR WOOD COLUMN FOOTING

SECTION 12 S5.201
SCALE: 1/2" = 1'-0"

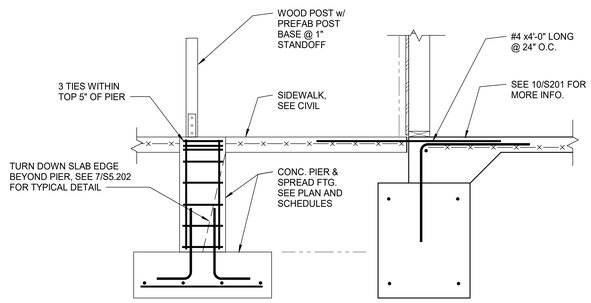


SECTION 11 S5.201
SCALE: 1/2" = 1'-0"

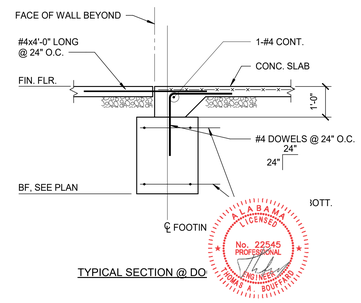


THICKENED SLAB @ STRINGER

SECTION 10 S5.201
SCALE: 1/2" = 1'-0"



SECTION 16 S5.201
SCALE: 3/4" = 1'-0"



TYPICAL SECTION @ DO

SECTION 15 S5.201
SCALE: 1/2" = 1'-0"

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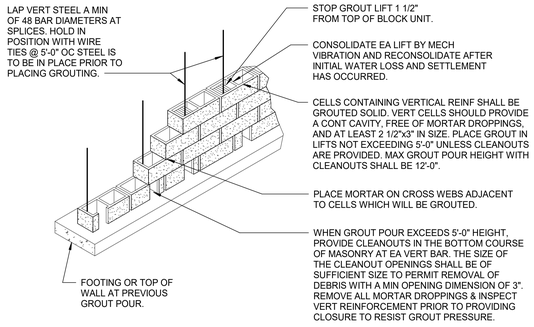
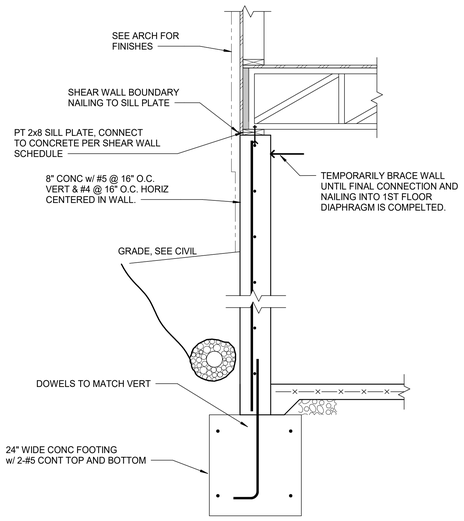
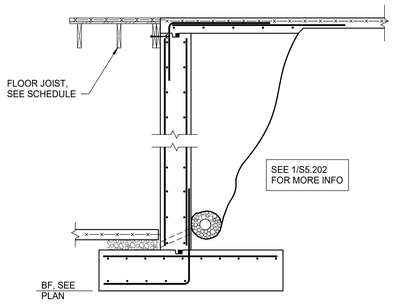


TYPICAL REINFORCED MASONRY CONSTRUCTION

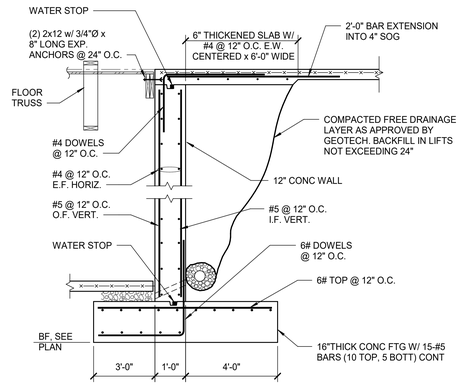
SECTION 4 S5.202
SCALE: 3/4" = 1'-0"



DETAIL 3 S5.202
SCALE: 3/4" = 1'-0"



SECTION 2 S5.202
SCALE: 1/2" = 1'-0"



SECTION 1 S5.202
SCALE: 1/2" = 1'-0"

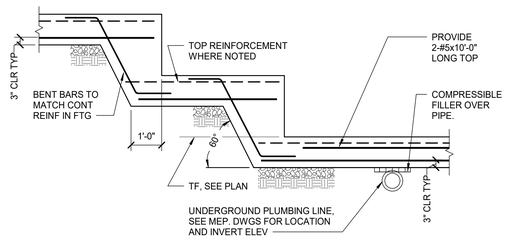
SECTION 8 S5.202
SCALE: 1/2" = 1'-0"

SECTION 7 S5.202
SCALE: 3/4" = 1'-0"

SECTION 6 S5.202
SCALE: 3/4" = 1'-0"

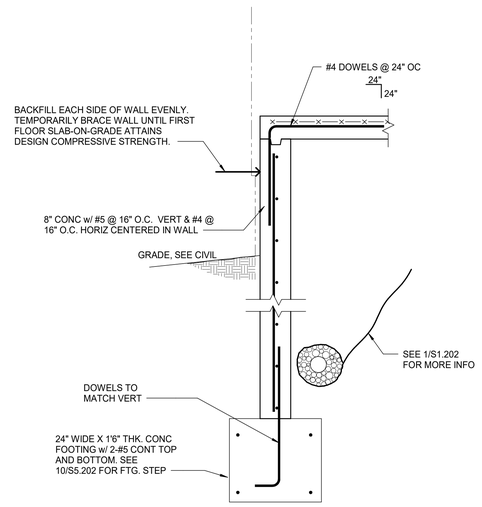
MASONRY CORNER OR INTERSECTION DETAIL

SECTION 5 S5.202
SCALE: 3/4" = 1'-0"



TYPICAL FOOTING STEP (F.S.)

SECTION 10 S5.202
SCALE: 1/2" = 1'-0"



SECTION 9 S5.202
SCALE: 3/4" = 1'-0"

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FLOOR JOIST/TRUSS SCHEDULE

MARK	SIZE	UNIFORM LOADS			DEFLECTION LIMITS			REMARKS
		DL PSF (TOP CHORD)	LL PSF (TOP CHORD)	DL PSF (BOT. CHORD)	LL	TL	△MAX. (INCHES)	
T-1	18" DEEP WOOD TRUSS @ 24" O.C. MAX.	15	40	7	L/480	L/240	1"	FLOOR/UNIT
T-2	PT 2x10 @ 16" OC	-	-	-	-	-	-	EXTERIOR PRIVATE BALCONY
T-3	PT 2x12 @ 12" OC	-	-	-	-	-	-	BREEZEWAY
T-4	METAL PLATED OPEN WEB WOOD TRUSSES @ 24" OC	13	20 MIN.	7	L/360	L/240	1"	PITCHED ROOF AREAS
T-5	12" DEEP WOOD TRUSS @ 24" O.C. MAX.	13	60	7	L/360	L/240	1"	LOW ROOF BALCONY/MECH AREAS
T-6	12" DEEP WOOD TRUSS @ 24" O.C. MAX.	13	20 MIN.	7	L/360	L/240	1"	FLAT ROOF

1. TRUSS SPACING SHALL BE DETERMINED BY TRUSS MANUF, 24" OC MECH TO MEET THE DESIGNATED LOADING AND DEFLECTION CRITERIA.
2. REFER TO FRAMING PLANS AND SECTIONS FOR CONCENTRATED OR OTHER ADDITIONAL LOADS TO TRUSSES.

HEADER SCHEDULE

1	2-2x6
2	3-2x6
3	2-2x8
4	3-2x8
5	2-2x10
6	3-2x10
7	2-2x12
8	3-2x12
9	2-1 3/4"x9 1/2" LVL
10	2-1 3/4"x11 7/8" LVL
11	2-1 3/4"x14" LVL
12	2-1 3/4"x16" LVL
13	2-1 3/4"x18" LVL
14	3-1 3/4"x9 1/2" LVL
15	3-1 3/4"x11 7/8" LVL
16	3-1 3/4"x14" LVL
17	3-1 3/4"x16" LVL
18	2-1 3/4"x11 7/8" LVL w/ (2)-3/8" THK. STL. PL.

NOTE:
PROVIDE DOUBLE STUD @ END OF EACH HEADER UNO

COLUMN SCHEDULE

C-1	2-2x4
C-2	2-2x6
C-3	3-2x4
C-4	3-2x6
C-5	4-2x4
C-6	4-2x6
C-7	5-2x4
C-8	5-2x6
C-9	4x4 POST
C-10	4x6 POST
C-11	6x6 SP #2 POST

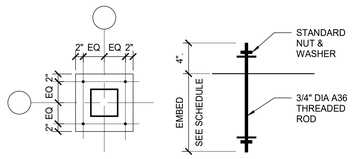
BEARING WALL SCHEDULE

		EXTERIOR NON-LOAD BEARING		EXTERIOR		BREEZEWAY		UNIT SEPARATION/ UNIT INTERIOR		MARK / FLOOR
		W-4a	W-4	W-3a	W-3	W-2a	W-2	W-1a	W-1	
										ROOF
		(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 24" OC	(1)-2x6 @ 24" OC	LEVEL 3
		(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 16" OC	(1)-2x6 @ 24" OC	(1)-2x6 @ 24" OC	(1)-2x6 @ 24" OC	(1)-2x6 @ 24" OC	LEVEL 2
						(2)-2x6 @ 24" OC	(2)-2x6 @ 24" OC	(2)-2x6 @ 24" OC	(2)-2x6 @ 24" OC	LEVEL 1
						(2)-2x6 @ 24" OC		(3)-2x6 @ 24" OC		LOWER LEVEL

BEARING WALL SCHEDULE NOTES:
1. PROVIDE CRIPPLE BLOCKING AS PER TYPICAL DETAILS.
2. SEE TYPICAL SHEARWALL DETAILS FOR ADD'L REQUIREMENTS.
3. REFER TO ARCHITECTURAL DRAWINGS FOR WALL SECTIONS AND LOCATION OF DRAFT STOP.
4. ALL BEARING WALLS SHALL HAVE MID-HEIGHT BLOCKING.

STEEL COLUMN SCHEDULE

			SC-2	SC-1	MARK / FLOOR
					ROOF
			HSS4X4X1/4"		LEVEL 3
					LEVEL 2
					LEVEL 1
				HSS 3 1/2X 3 1/2X5/16"	FOUNDATION
			12X12X3/4"	12X12X3/4"	BASE PLATE
			4-3/4" DIA. X 8" EMBED	4-3/4" DIA. X 8" EMBED	ANCHOR BOLTS
					REMARKS

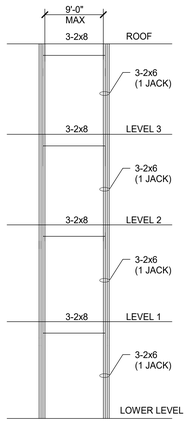


TYP BASE PLATE TYPICAL ANCHOR BOLT

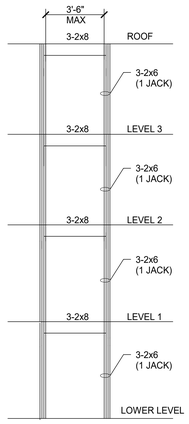
ANCHOR BOLT ARRANGEMENT

WOOD POST SCHEDULE

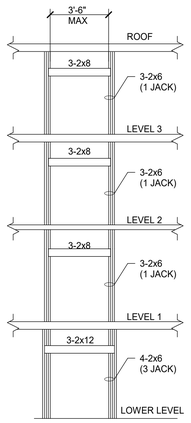
	P2	P1	MARK / FLOOR
			ROOF
			LEVEL 3
			LEVEL 2
			LEVEL 1
			LOWER LEVEL



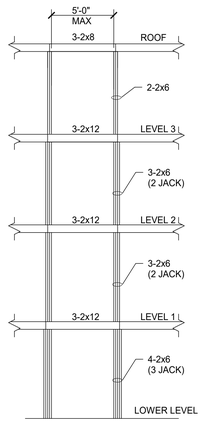
TYPICAL HEADERS/COLUMNS W-4/ W-4a



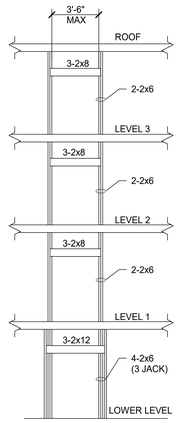
TYPICAL HEADERS/COLUMNS W-3/ W-3a



TYPICAL HEADERS/COLUMNS W-2/ W-2a



TYPICAL FLUSH HEADERS W-1/W-1a



TYPICAL DROPPED HEADERS/COLUMNS W-1/ W-1a

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S5.401
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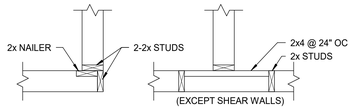
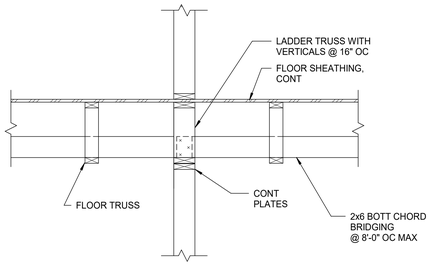


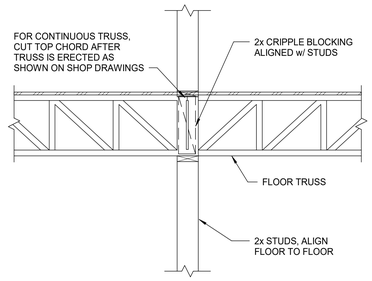
TYPICAL WALL CORNER AND INTERSECTION
(INTERIOR AND EXTERIOR - TYPE V)

SECTION 5 S5.402
SCALE: 3/4" = 1'-0"



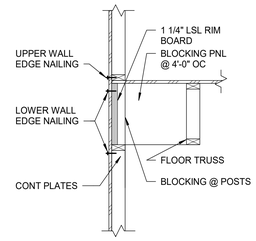
TYPICAL UNIT SEPARATION WALL
PARALLEL TRUSS - SINGLE WALL

SECTION 4 S5.402
SCALE: 3/4" = 1'-0"



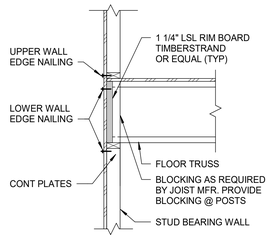
TYPICAL INTERIOR BEARING WALL

SECTION 3 S5.402
SCALE: 3/4" = 1'-0"



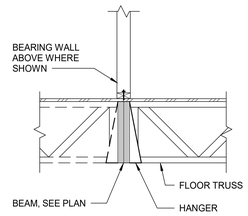
WALL PARALLEL TO JOIST

SECTION 2 S5.402
SCALE: 3/4" = 1'-0"



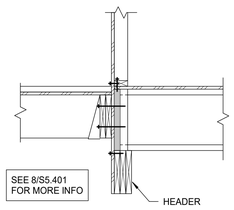
TYPICAL EXTERIOR BRG. WALL

SECTION 1 S5.402
SCALE: 3/4" = 1'-0"

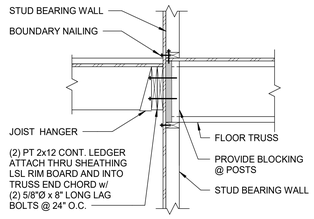


TYPICAL JOIST BRG. ON BEAM

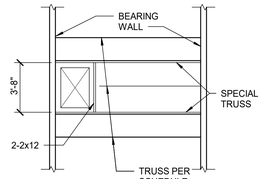
SECTION 10 S5.402
SCALE: 3/4" = 1'-0"



SECTION 9 S5.402
SCALE: 3/4" = 1'-0"



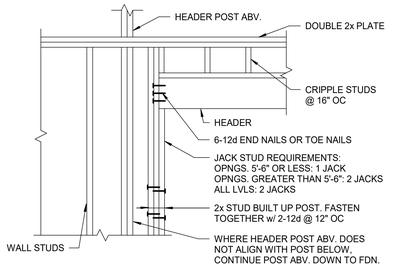
SECTION 8 S5.402
SCALE: 3/4" = 1'-0"



NOTE: DETAIL APPLIES FOR SMALL OPENINGS UP TO 3'-8" WIDE

TYPICAL SHAFT FRAMING AT TRUSS FLOOR

SECTION 7 S5.402
SCALE N.T.S.



TYPICAL DROPPED HEADER AT OPENING

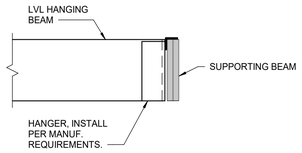
SECTION 6 S5.402
SCALE: 3/4" = 1'-0"

HANGING BEAM SIZE	MAX. LOAD (LB)	REQUIRED * HANGER
2-12"	7,900	SIMPSON HGLT 3.511
2-14"	9,310	SIMPSON HGLT 3.514
2-16"	10,600	SIMPSON HGLT 3.516
2-18"	10,950	SIMPSON HGLT 3.518

* OR EQUAL

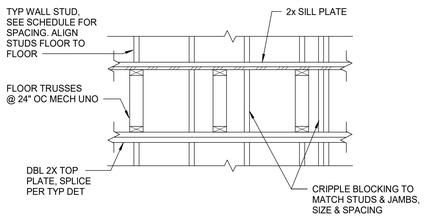
HANGING BEAM SIZE	MAX. LOAD (LB)	REQUIRED * HANGER
3-12"	11,850	SIMPSON HGLT 5.511
3-14"	12,750	SIMPSON HGLT 5.514
3-16"	12,750	SIMPSON HGLT 5.516
3-18"	12,750	SIMPSON HGLT 5.518

* OR EQUAL



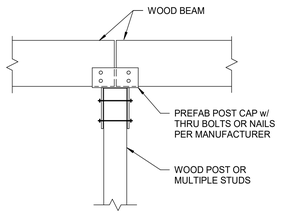
TYPICAL LVL BEAM TO BEAM CONNECTION

SECTION 15 S5.402
SCALE: 3/4" = 1'-0"



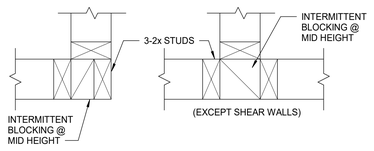
TYPICAL WALL CRIPPLE BLOCKING
AT BEARING WALLS

SECTION 14 S5.402
SCALE: 3/4" = 1'-0"



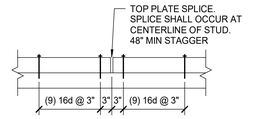
TYPICAL WOOD BEAM TO WOOD POST CONNECTION

SECTION 13 S5.402
SCALE: 3/4" = 1'-0"



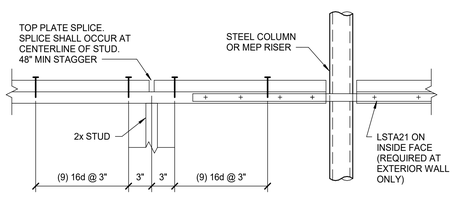
TYP CORNER AND INTERSECTION STUD WALL

SECTION 12 S5.402
SCALE: 3/4" = 1'-0"



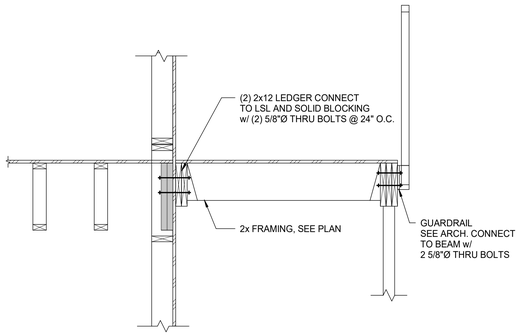
TYPICAL TOP PLATE SPLICE

SECTION 11 S5.402
SCALE: 3/4" = 1'-0"

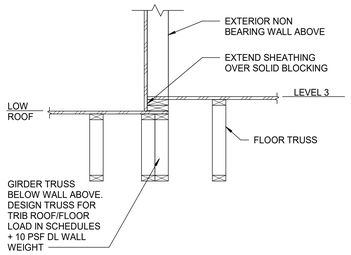


TYPICAL TOP PLATE SPLICE AT ALL WALLS AND
PLATE INTERRUPTION STRAP AT EXTERIOR WALL

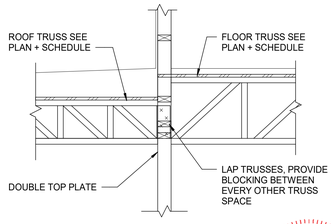
SECTION 19 S5.402
SCALE: 1 1/2" = 1'-0"



SECTION 18 S5.402
SCALE: 3/4" = 1'-0"



SECTION 17 S5.402
SCALE: 3/4" = 1'-0"



SECTION 16 S5.402
SCALE: 3/4" = 1'-0"

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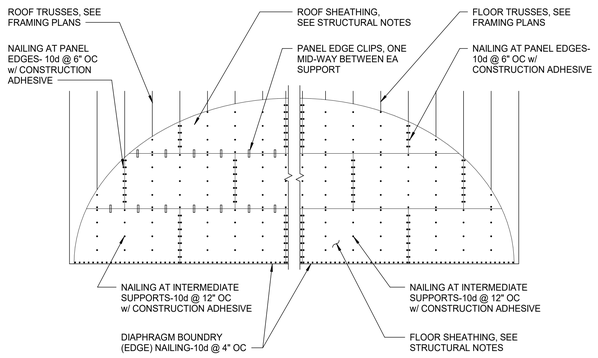


ROOF DIAPHRAGM NAILING PATTERN

FLOOR DIAPHRAGM NAILING PATTERN (WALL NAILING PATTER SIMILIAR - SEE SHEAR WALL SCHEDULE)

SECTION 1 / S5.403

SCALE: 3/4" = 1'-0"

SHEARWALL SCHEDULE

MARK (WALL TYPE)	LEVEL	SHEATHING TYPE	NAIL SIZE / PENETRATION	NAIL SPACING AT EDGES	BLOCKING	MIN END CHORD	SILL ANCHORS	CHORD ANCHORS	REMARKS
SW-1	3RD TO ROOF	7/16" OSB (EXT. FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	2-2X6	(1) ROWS 10d NAILS @ 6" O.C. / (1) ROWS 10d NAILS @ 6" O.C.	HDU2-SDS2.5 w/5/8" ØANCHOR	
SW-1	2ND TO 3RD	7/16" OSB (EXT. FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	2-2X6	(1) ROWS 10d NAILS @ 6" O.C. / (1) ROWS 10d NAILS @ 6" O.C.	NA	
SW-1	1ST TO 2ND	7/16" OSB (EXT. FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	3-2X6	(1) ROWS 10d NAILS @ 4" O.C. / (1) ROWS 10d NAILS @ 4" O.C.	NA	NOTE 1 & NOTE 3
SW-1	LOWER LEVEL	7/16" OSB (EXT. FACE)	8d COMMON / 1 3/8"	4"	BLOCKED	4-2X6	(1) ROWS 1/4" DIA WOOD SCREWS @ 4" O.C. / 1/2" DIA. A.B. @ 24" O.C. THRU PT SILL	NA	

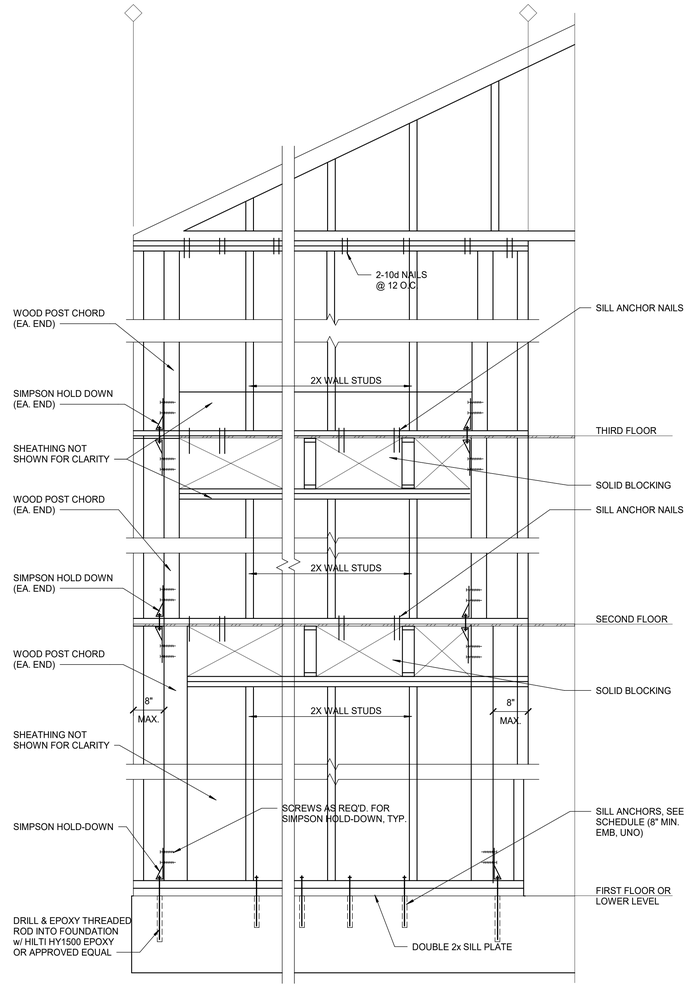
MARK (WALL TYPE)	LEVEL	SHEATHING TYPE	NAIL SIZE / PENETRATION	NAIL SPACING AT EDGES	BLOCKING	END CHORD	SILL ANCHORS	CHORD ANCHORS	REMARKS
SW-2	3RD TO ROOF								
SW-2	2ND TO 3RD	7/16" OSB (EXT. FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	2-2X6	(1) ROWS 10d NAILS @ 6" O.C. / (1) ROWS 10d NAILS @ 6" O.C.	HDU2-SDS2.5 w/5/8" ØANCHOR	
SW-2	1ST TO 2ND	7/16" OSB (EXT. FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	3-2X6	(1) ROWS 10d NAILS @ 4" O.C. / (1) ROWS 10d NAILS @ 4" O.C.	HDU2-SDS2.5 w/5/8" ØANCHOR	NOTE 1 & NOTE 3
SW-2	LOWER LEVEL	7/16" OSB (EXT. FACE)	8d COMMON / 1 3/8"	4"	BLOCKED	4-2X6	(1) ROWS 1/4" DIA WOOD SCREWS @ 4" O.C. / 1/2" DIA. A.B. @ 24" O.C. THRU PT SILL	HDU2-SDS2.5 w/SB5/8x24	

MARK (WALL TYPE)	LEVEL	SHEATHING TYPE	NAIL SIZE / PENETRATION	NAIL SPACING AT EDGES	BLOCKING	END CHORD	SILL ANCHORS	CHORD ANCHORS	REMARKS
SW-3	3RD TO ROOF								
SW-3	2ND TO 3RD	7/16" OSB (1 FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	2-2X6	(1) ROWS 10d NAILS @ 6" O.C. / (1) ROWS 10d NAILS @ 6" O.C.	HDU2-SDS2.5 w/5/8" ØANCHOR	
SW-3	1ST TO 2ND	7/16" OSB (1 FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	2-2X6	(1) ROWS 10d NAILS @ 4" O.C. / (1) ROWS 10d NAILS @ 4" O.C.	HDU5-SDS2.5 w/5/8" ØANCHOR	NOTE 1 & NOTE 3 - SB5/8X24 WHERE TERMINATES @ 1ST
SW-3	LOWER LEVEL	7/16" OSB (1 FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	3-2X6	(1) ROWS 10d NAILS @ 4" O.C. / 1/2" DIA. A.B. @ 32" O.C. THRU PT SILL	HDU8-SDS2.5 w/SB7/8x24	

MARK (WALL TYPE)	LEVEL	SHEATHING TYPE	NAIL SIZE / PENETRATION	NAIL SPACING AT EDGES	BLOCKING	END CHORD	SILL ANCHORS	CHORD ANCHORS	REMARKS
SW-4	3RD TO ROOF								
SW-4	2ND TO 3RD	7/16" OSB (1 FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	2-2X6	(1) ROWS 10d NAILS @ 6" O.C. / (1) ROWS 10d NAILS @ 6" O.C.	HDU2-SDS2.5 w/5/8" ØANCHOR	
SW-4	1ST TO 2ND	7/16" OSB (1 FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	2-2X6	(1) ROWS 10d NAILS @ 4" O.C. / 1/2" DIA. A.B. @ 32" O.C. THRU PT SILL	HDU2-SDS2.5 w/SB5/8x24	
SW-4	LOWER LEVEL								

MARK (WALL TYPE)	LEVEL	SHEATHING TYPE	NAIL SIZE / PENETRATION	NAIL SPACING AT EDGES	BLOCKING	END CHORD	SILL ANCHORS	CHORD ANCHORS	REMARKS
SW-5	3RD TO ROOF								
SW-5	2ND TO 3RD	7/16" OSB (1 FACE)	8d COMMON / 1 3/8"	6"	BLOCKED	2-2X6	(2) ROWS 16d NAILS @ 6" O.C. / (2) ROWS 16d NAILS @ 6" O.C.	HDU4-SDS2.5 w/5/8" ØANCHOR	NOTE 4
SW-5	1ST TO 2ND	7/16" OSB (1 FACE)	8d COMMON / 1 3/8"	4"	BLOCKED	2-2X6	(2) ROWS 16d NAILS @ 6" O.C. / (2) ROWS 16d NAILS @ 6" O.C.	HDU8-SDS2.5 w/7/8" ØANCHOR	NOTE 1, 3 AND 4 - SB7/8X24 WHERE TERMINATES @ 1ST
SW-5	LOWER LEVEL	7/16" OSB (1 FACE)	8d COMMON / 1 3/8"	4"	BLOCKED	6X6 POST	(2) ROWS 16d NAILS @ 4" O.C. / 1/2" DIA. A.B. w/ 4.5" SQ PL WASHERS @ 24" O.C. THRU PT SILL	HDU11-SDS2.5 w/SB1x30	NOTE 4

NOTES:
1. ALL SHEAR WALLS END WHERE SHOWN ON FRAMING PLANS, SEE S1.101-S1.104.
2. WHERE SHEAR WALLS TERMINATE ABOVE FOUNDATION, PROVIDE SILL NAILING PER SHEAR WALL SCHEDULE.
3. WHERE SHEAR WALLS TERMINATE AT FOUNDATIONS, PROVIDE ANCHOR BOLTS THRU PT SILL AS SCHEDULED ON LOWEST LEVEL.
4. SHEAR WALL END CHORDS AND HOLDDOWNS ONLY REQUIRED AT FAR SHEAR WALL ENDS.

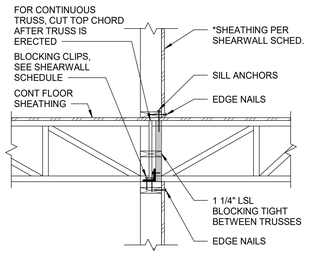


TYPICAL SHEAR WALL DETAIL

TYPICAL SHEARWALL ELEVATION AND SCHEDULE

NOTES:

1. SHEARWALLS ARE INDICATED THUS ON PLAN: "SW-#".
2. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE DIAGONAL BRACING TO ENSURE LATERAL STABILITY OF STUD WALLS, BY USE OF DIAGONAL METAL STRAPS OR OTHER MEANS, PRIOR TO INSTALLATION OF SHEAR WALL SHEATHING.
3. INTERIOR SHEAR WALL SHEATHING SHALL BE INSTALLED AND FASTENED AT SILL PLATE PRIOR TO PLACING OF GYPCRETE.
4. ALL SHEAR WALLS SHALL HAVE A MINIMUM OF TWO STUDS AT EACH END (CHORDS), UNO IN SCHED
5. SCHEDULED SHEARWALL SHEATHING SHALL BE FASTENED DIRECTLY IN CONTACT WITH THE WALL STUDS. IT SHALL NOT BE PERMITTED TO PLACE RESILIENT CHANNELS NOR ANY OTHER MATRIAL IN BETWEEN THE SCHEDULED SHEATHING AND THE STUDS.
6. APPLY PANELS WITH LONG DIMENSION ACROSS STUDS.

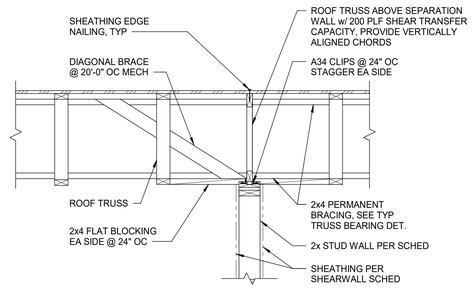


NOTES:
1. '*' -WHERE SHEATHING IS INDICATED ON 1 FACE IN SCHEDULE, PLACE ON SAME FACE AS LSL BLOCKING.

TYPICAL SHEARWALL AT UNIT SEPARATION
PERPENDICULAR TO TRUSS

SECTION 5 / S5.403

SCALE: 3/4" = 1'-0"



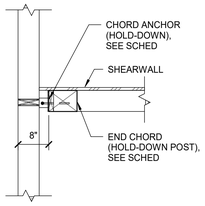
TYPICAL SHEAR WALL AT ROOF
PARALLEL TRUSS

SECTION 4 / S5.403

SCALE: 3/4" = 1'-0"

SECTION A / S5.403

SCALE: 3/4" = 1'-0"

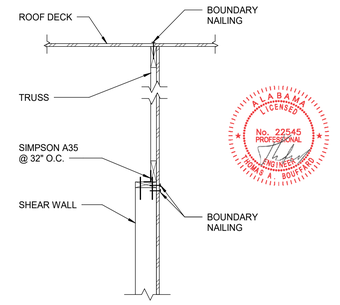


PLAN

TYPICAL DETAIL AT SHEARWALL

SECTION 3 / S5.403

SCALE: 3/4" = 1'-0"



TYPICAL SHEARWALL AT ROOF

SECTION 2 / S5.403

SCALE: 3/4" = 1'-0"

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TERRACES AT HIGH MOUNTAIN
4130 HIGH MOUNTAIN ROAD NE
HUNTSVILLE, AL 35811
06/11/2021

S5.403
WOOD SHEARWALL SCHEDULE & DETAILS
12/15/20

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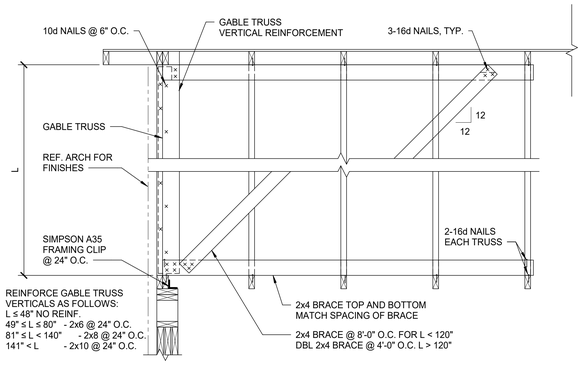
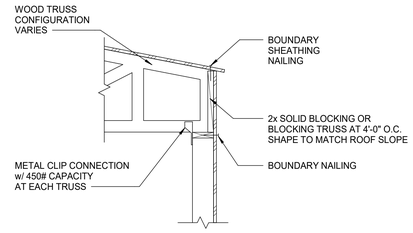
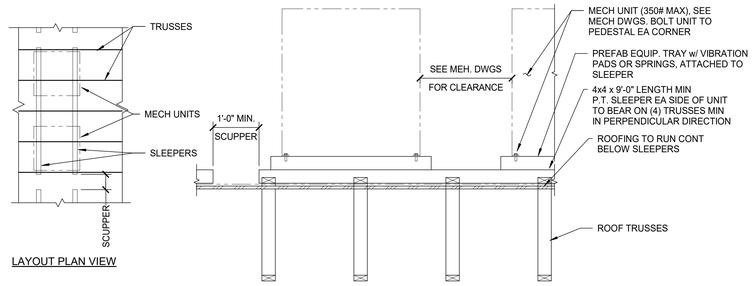


TYPICAL GABLE TRUSS REINFORCEMENT

SECTION 3 S5.501
SCALE: 3/4" = 1'-0"



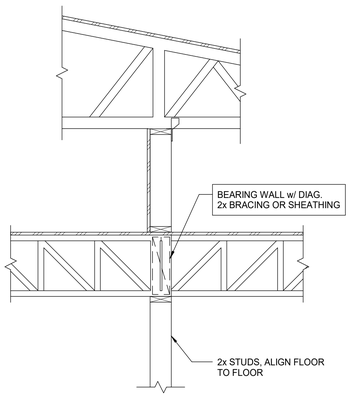
SECTION 2 S5.501
SCALE: 3/4" = 1'-0"



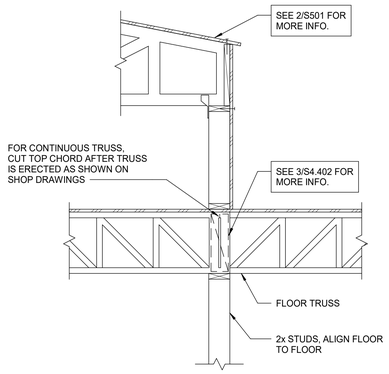
LAYOUT PLAN VIEW

TYPICAL AC UNIT SUPPORT

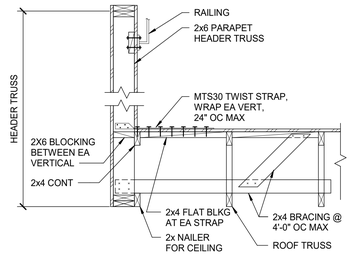
SECTION 1 S5.501
SCALE: 3/4" = 1'-0"



SECTION 7 S5.501
SCALE: 3/4" = 1'-0"

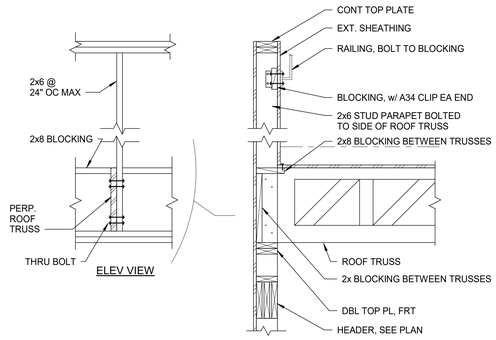


SECTION 6 S5.501
SCALE: 3/4" = 1'-0"



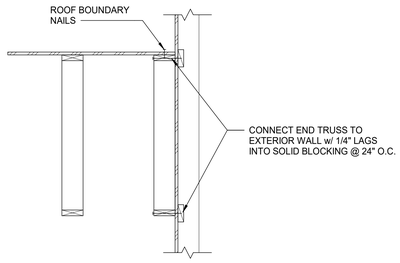
TYPICAL PARAPET DETAIL AT WINDOW HEADER
PARALLEL TRUSS

SECTION 5 S5.501
SCALE: 3/4" = 1'-0"

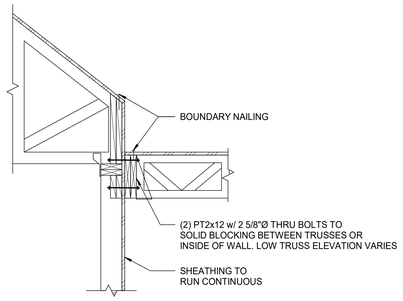


TYPICAL PARAPET DETAIL
PERPENDICULAR TRUSS

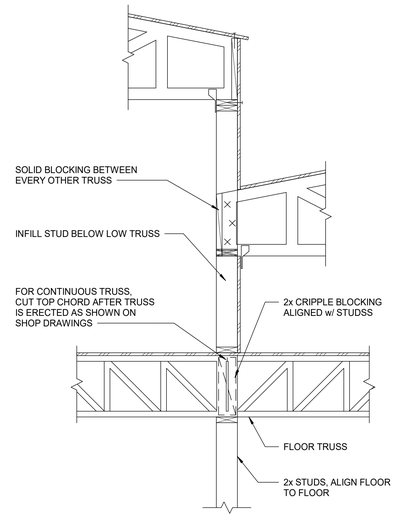
SECTION 4 S5.501
SCALE: 3/4" = 1'-0"



SECTION 10 S5.501
SCALE: 3/4" = 1'-0"



SECTION 9 S5.501
SCALE: 3/4" = 1'-0"



SECTION 8 S5.501
SCALE: 3/4" = 1'-0"

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TERRACES AT HIGH MOUNTAIN
4130 HIGH MOUNTAIN ROAD NE
HUNTSVILLE, AL 35811
06/11/2021

S5.501
WOOD ROOF FRAMING SECTIONS
12/15/20

ALABAMA LICENSED No. 22545 PROFESSIONAL ENGINEER THOMAS A. BOUTWELL

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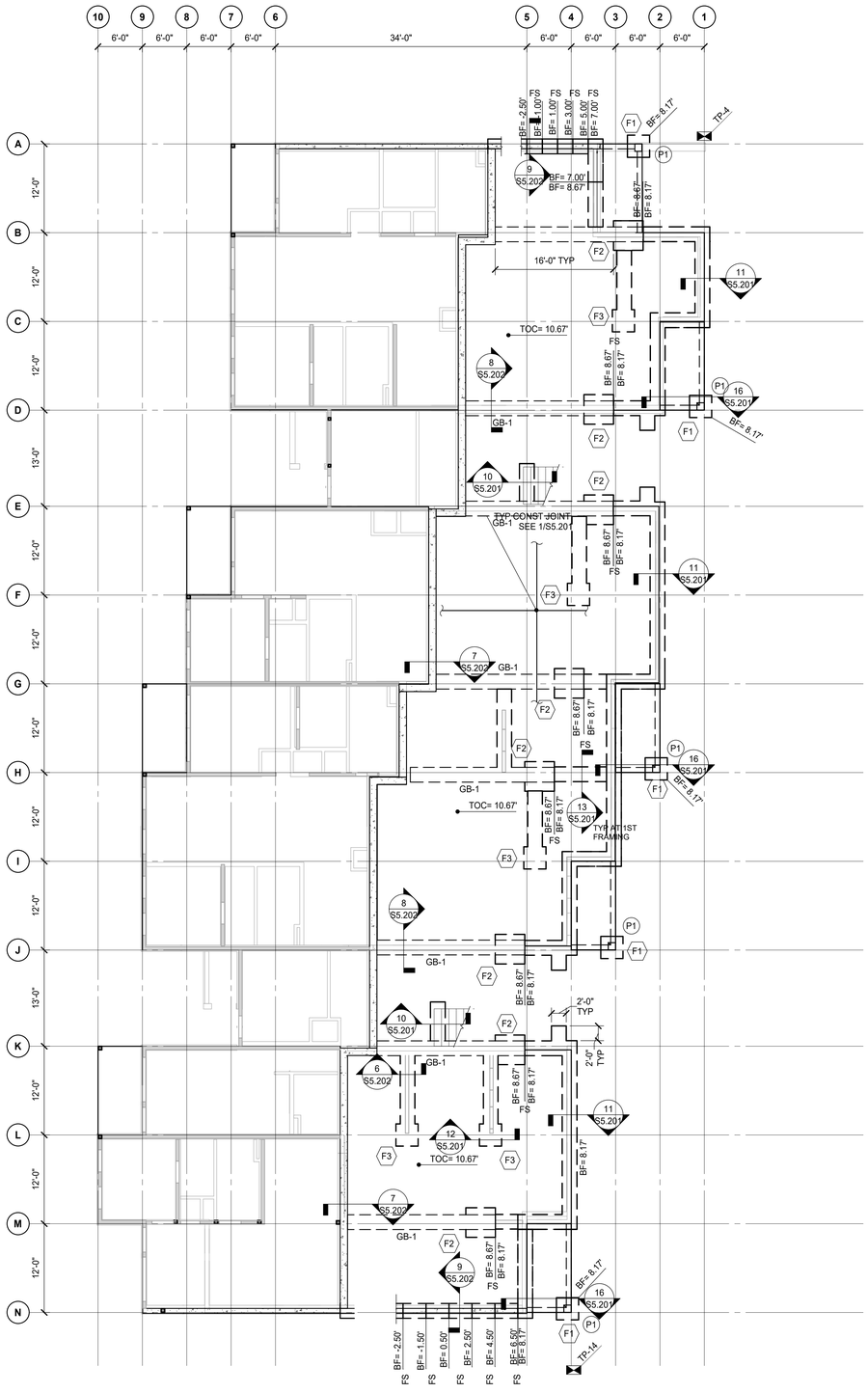
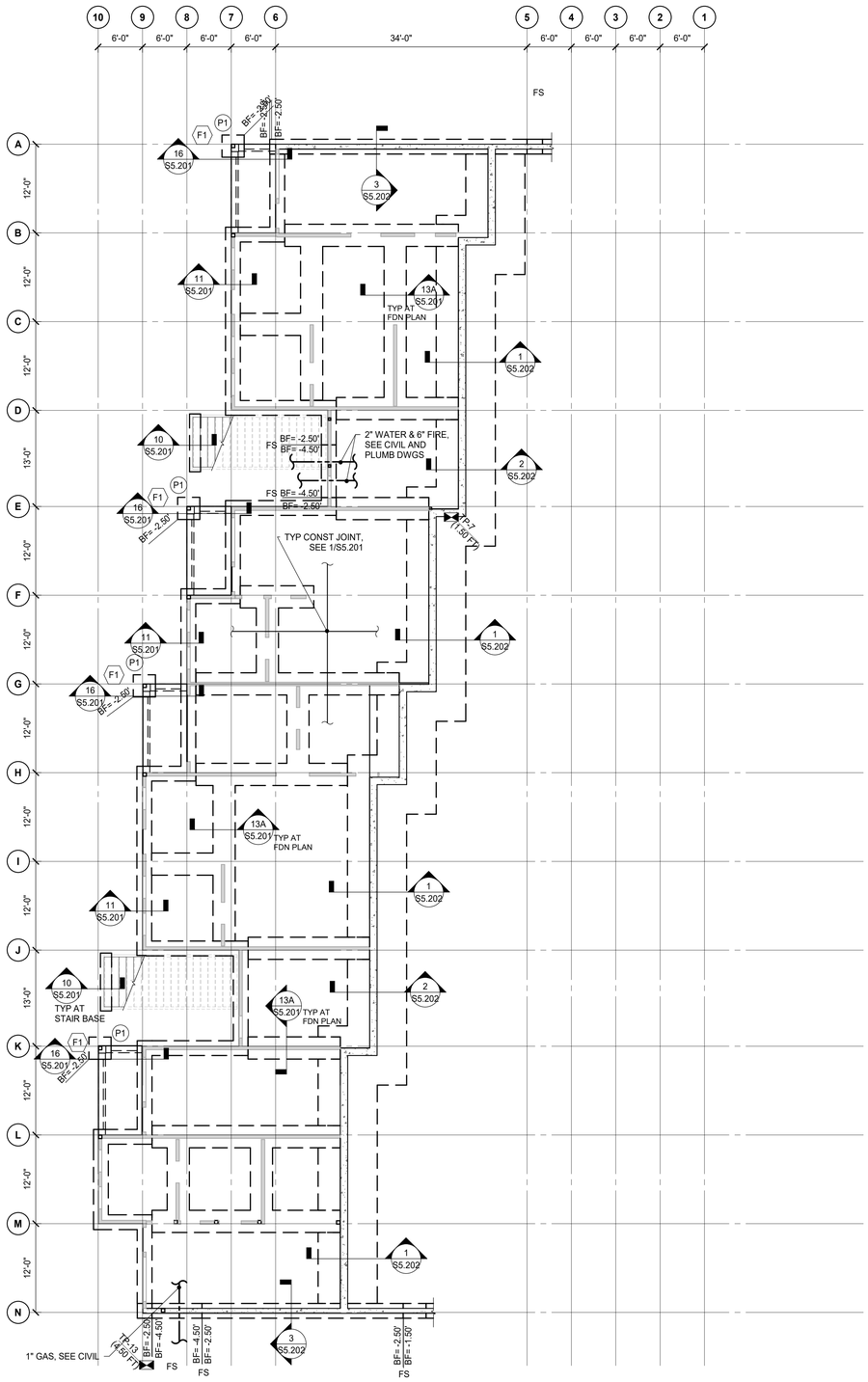


LEVEL 1 FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES:

1. SEE S5.100 FOR TYPICAL FOUNDATION NOTES.
2. TOP OF CONCRETE ELEVATION = 10.67' (ACTUAL EL = 1067.52')



FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. REFERENCE BUILDING A5 DRAWINGS FOR BUILDING STRUCTURAL NOTES, SPECIAL INSPECTIONS, TYPICAL CONSTRUCTION, SECTIONS AND SCHEDULES.
2. TOP OF CONCRETE ELEVATION = 0.00' REFERENCE (ACTUAL ELEVATION= 1056.85')
3. REFER TO S5.100 ON BUILDING A5 FOR BALANCE OF PLAN NOTES NOT SHOWN.

ALABAMA LICENSED No. 22545 PROFESSIONAL ENGINEER THOMAS A. BOUTANG

REVISIONS
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TERRACES AT HIGH MOUNTAIN

4130 HIGH MOUNTAIN ROAD NE
HUNTSVILLE, AL 35811

06/11/2021

S6.100

FOUNDATION PLAN AND LEVEL 1 FRAMING PLANS

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STRUCTURAL NOTES

1. GENERAL

A. THE BUILDING IS DESIGNED UNDER THE PROVISIONS OF THE 2015 INTERNATIONAL BUILDING CODE AND ASCE 7-10.

B. THE FOLLOWING LOADS WERE USED IN THE DESIGN:

BUILDING RISK CATEGORY	II
FLOOR LIVE LOAD	
PUBLIC AREAS	100 PSF
STAIRS/EXITS	100 PSF / 300 LB PT LOAD
ROOF LIVE LOAD	
MINIMUM UNIFORM ROOF LIVE LOAD	20 PSF / 300 PT LOAD
ROOF SNOW LOAD	
GROUND SNOW LOAD (Pg)	10 PSF
FLAT-ROOF SNOW LOAD (Pf)	7 PSF
RAIN-ON-SNOW SURCHARGE	5 PSF
SNOW EXPOSURE FACTOR (Ce)	1.0
SNOW LOAD IMPORTANCE FACTOR (Is)	1.0
THERMAL FACTOR (Ct)	1.0
WIND LOAD	
WIND SPEED (3-SECOND GUST)	
Vult	115 MPH
Vasd; (0.77*VULT)	90 MPH
WIND EXPOSURE	B
INTERNAL PRESSURE COEFFICIENT	+0.18, -0.18
COMPONENTS AND CLADDING WIND LOAD	
EFFECTIVE WIND AREA ZONE PRESSURE	
10 FT.^2 4	+21.8 PSF
10 FT.^2 4	-23.6 PSF
10 FT.^2 5	+21.8 PSF
10 FT.^2 5	-29.0 PSF
EARTHQUAKE DESIGN	
SEISMIC IMPORTANCE FACTOR (Ie)	1.00
SEISMIC DESIGN CATEGORY	C
SITE CLASSIFICATION	D
SEISMIC RESPONSE COEFFICIENTS	
Ss	0.258
S1	0.120
Sds	0.274
Sd1	0.186
DESIGN BASE SHEAR	18 KIPS
SEISMIC-FORCE RESISTING SYSTEM PER ASCE 7-10 TABLE 12.2-1	TYPE A8
R	3.5
Cd	2.25
Cs	0.078
TL	12
ANALYSIS METHOD	ELFP
HANDRAIL AND GUARD LOADS	
HANDRAIL AND GUARD	50 PLF / 200 LB PT LOAD
INTERMEDIATE RAIL	50 LB PT LOAD

C. SEE ARCHITECTURAL DRAWINGS FOR ANGLES, CLIPS, PLATES, ETC., AND OTHER MISCELLANEOUS ITEMS. VERIFY AND COORDINATE ALL FRAMES, OPENINGS, ETC. WITH THE MECHANICAL AND ELECTRICAL CONTRACTORS.

D. SUBMIT SHOP DRAWINGS FOR THE FOLLOWING ITEMS. SUBMITTALS INCLUDE BUT MAY NOT BE LIMITED TO:

--CONCRETE MIX DESIGN
--REINFORCING STEEL
--STRUCTURAL STEEL
--PRE-ENGINEERED BUILDING COMPONENTS
--WOOD TRUSSES

DO NOT USE CONTRACT DRAWINGS AS A BASE FOR SHOPS. REVIEW IS LIMITED TO DESIGN CONFORMANCE. CONTRACTOR IS RESPONSIBLE FOR DIMENSIONS.

E. CONTRACTOR SHALL COORDINATE WITH THE QUALIFIED AGENCY RETAINED BY THE OWNER TO PERFORM INSPECTION AND TESTING. INSPECTIONS REQUIRED INCLUDE, BUT MAY NOT BE LIMITED TO:

--SOILS AND FOUNDATIONS
--CONCRETE
--STRUCTURAL STEEL
--MASONRY

2. EARTHWORK

A. FOUNDATIONS ARE DESIGNED TO BEAR ON ENGINEERED FILL OR NATURAL SOIL WITH A CAPACITY OF 2,000 PSF BASED ON RECOMMENDATIONS IN THE GEOTECHNICAL REPORT PREPARED BY OMI, INC. DATED 12/31/2020. THIS VALUE IS TO BE VERIFIED IN THE FIELD BY THE BUILDING INSPECTOR OR A QUALIFIED TESTING AGENCY.

B. BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 2 FOOT-0 INCH BELOW FINISHED EXTERIOR GRADE. WHERE REQUIRED, STEP FOOTINGS IN RATIO OF 2 HORIZONTAL TO 1 VERTICAL.

C. COMPACTED BACKFILL BELOW BUILDING SLABS AND FOOTINGS: ALL SOIL FILL MATERIAL MUST BE APPROVED BY SOILS ENGINEER PRIOR TO PLACEMENT. PROOFROLL SUBGRADE REMOVING ANY SOFT OR COMPRESSIVE MATERIALS. FILL MATERIAL SHALL BE PLACED IN LAYERS NOT TO EXCEED 8 INCHES AND COMPACTED TO MIN. 95 PERCENT OF THE DRY MAXIMUM DENSITY AS DETERMINED BY ASTM D698.

D. AT ROCKY AREAS AROUND AND BELOW EL 1055 FT MSL: ROCK LEDGES, PINNACLES OR BOULDERS, IF ENCOUNTERED AND CONFLICTING WITH THE PROPOSED FOUNDATION SYSTEM, SHALL BE REMOVED BY BLASTING, RIPPING OR HOE RAMMING PER THE GEOTECHNICAL REPORT. ONCE THE AREA HAS BEENEXCAVATED, A MINIMUM 1 FOOT LAYER OF #2 STONE SHALL BE PLACED ACROSS THE AREA. THE #2 STONE SHALL BE CAPPED WITH A 6" LAYER OF "CRUSHER RUN" (1.5" TO DUST, STONE). SUBSEQUENT LAYERS OF ENGINEERED FILL SHALL THEN BE PLACED TO BUILDING PAD ELEVATIONS, BELOW THE 4" #57 UNDERSLAB BASE.

E. AT SOIL AREAS AROUND AND ABOVE EL 1062 FT MSL: AREAS APPROXIMATLEY AT BUILDING PAD ELAND AREAS THAT WILL RECEIVE ENGINEERED FILL SHALL BE OBSERVED BY THE GER PRIOR TO PLACING THE 1 FOOT LAYER OF #2 STONE AND 6" CAP OF CRUSHER RUN. SUBSEQUENT LAYERS OF ENGINEERED FILL SHALL THEN BE PLACED TO BUILDING PAD ELEVATIONS, BELOW THE 4" #57 UNDERSLAB BASE.

CONCRETE

F. CONCRETE CONSTRUCTION SHALL BE PER THE APPLICABLE BUILDING CODE, ACI 318 AND ACI 301, LATEST EDITIONS.

G. CONCRETE SHALL ATTAIN THE FOLLOWING 28 DAY COMPRESSIVE STRENGTHS PER ASTM A39.

--FOOTINGS, PIERS, WALLS	3,000 PSI
--SLAB-ON-GRADE	3,500 PSI

H. VERIFY CONCRETE STRENGTHS WITH A MINIMUM OF ONE SET OF NINE 4X8-INCH COMPRESSION CYLINDERS, (3 @ 7 DAYS, 3 @ 28, 3 SPARE).

I. EXTERIOR CONCRETE SHALL BE AIR-ENTRAINED TO PROVIDE AN AIR CONTENT OF 6+/-1.5 PERCENT BY VOLUME.

J. PROVIDE CLEAR DISTANCE TO OUTERMOST REINFORCING AS FOLLOWS:

CONCRETE CAST AGAINST EARTH 3 INCHES

CONCRETE EXPOSED TO EARTH OR WEATHER:
#5 OR SMALLER 1-1/2 INCHES
#6 OR LARGER 2 INCHES

K. NON-SHRINK GROUT FOR COLUMNS BASE PLATES SHALL ATTAIN A 28 DAY COMPRESSIVE STRENGTH: F'c = 5,000 PSI.

L. REINFORCING STEEL SHALL CONFORM TO A615-GR60. MESH SHALL CONFORM TO ASTM A185 WITH MINIMUM LAPS OF 8 INCHES. PLACING PLANS AND SHOP FABRICATION DETAILS SHALL BE IN ACCORDANCE WITH "THE MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES". FURNISH SUPPORT BARS AND ACCESSORIES IN ACCORDANCE WITH C.R.S.I. STANDARDS.

M. PROVIDE CORNER BARS TO MATCH HORIZONTAL REINFORCING IN WALLS AND FOOTINGS. SPLICE LAPS SHALL BE A MINIMUM OF 36 BAR DIAMETERS, UNLESS NOTED OTHERWISE. PROVIDE DOWELS BETWEEN FOOTINGS AND WALLS OR PIERS TO MATCH SIZE AND SPACING OF VERTICAL REINFORCING.

N. WALLS WITH LATERAL EARTH PRESSURES SHALL BE ADEQUATELY SHORED OR FLOOR/ROOF CONSTRUCTION SHALL BE IN PLACE AND SECURED PRIOR TO BACKFILLING.

O. INSTALLATION OF ELECTRICAL CONDUIT WITHIN THE CONCRETE SLAB-ON-GRADE IS PROHIBITED.

3. MASONRY

A. MASONRY CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE APPLICABLE BUILDING CODE AND THE "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" TMS 402/ACI-530/ASCE 5 AND THE "SPECIFICATIONS FOR MASONRY STRUCTURES" TMS 602/ACI-530.1/ASCE 6, LATEST EDITIONS.

B. MASONRY TO CONFORM TO THE FOLLOWING SPECIFICATIONS:

HOLLOW LOAD-BEARING C.M.U	ASTM C90
CONCRETE BUILDING BRICK	ASTM C55, GRADE A
MORTAR	ASTM C270, TYPE M OR S
GROUT	ASTM C476

C. MASONRY ASSEMBLIES SHALL HAVE COMPRESSIVE STRENGTH (F'M) GREATER THAN OR EQUAL TO 2000 PSI.

D. WALLS SHALL BE CONSTRUCTED USING A FULL BED OF MORTAR. VERTICAL REINFORCING SHALL BE GROUTED IN PLACE WITH 2500 PSI GROUT (GROUT SLUMP SHALL FALL BETWEEN 8 AND 11 INCHES) POUR HEIGHT AND LIFT HEIGHT SHALL NOT EXCEED 5 FEET - 0 INCHES.

E. PROVIDE CONTINUOUS HORIZONTAL JOINT REINFORCING IN MASONRY WALLS AT 16 INCHES O.C. PROVIDE AT 8 INCHES O.C. AT PARAPETS.

F. CAVITY WALLS OF BRICK AND BLOCK SHALL BE CONSTRUCTED WITH JOINT REINFORCING IN MASONRY AND ADJUSTABLE METAL ANCHORS TO BRICK.

G. UNLESS NOTED OTHERWISE, PROVIDE 16 INCH LONG BY 24 INCHES HIGH SOLID OR GROUTED BLOCK UNDER BEARING ENDS OF BEAMS.

H. PROVIDE 48 INCH REINFORCEMENT LAP AT CONTINUOUS BOND BEAM STEPS.

I. COMPOSITE WALLS SHALL HAVE THE COLLAR JOINT BETWEEN BRICK AND BLOCK GROUTED SOLID AND THE WALLS SHALL BE BUILT WITH BOTH WYTHES SIMULTANEOUSLY.

J. MASONRY WALLS SHALL HAVE CONTROL JOINTS AT 30 FEET ON CENTER UNLESS NOTED OTHERWISE.

K. REINFORCING STEEL SHALL CONFORM TO ASTM A615-GR60. LAP BARS A MINIMUM OF 48 BAR DIAMETERS. GROUT ALL REINFORCED CORES SOLID.

L. UNLESS SHOWN ON PLAN, LINTELS FOR MASONRY WALLS SHALL BE AS FOLLOWS:

OPENINGS TO 3 FT, 0 IN 3-1/2 X 3-1/2 X 1/4
3 FT, 1 IN TO 5 FT, 0 IN 4 X 3-1/2 X 5/16 - 3-1/2 HORIZONTAL
5 FT, 1 IN TO 6 FT, 6 IN 5 X 3-1/2 X 5/16 - 3-1/2 HORIZONTAL
OVER 6 FT, 6 IN CONSULT ARCHITECT/ENGINEER

PROVIDE 1 ANGLE FOR EACH 4 INCHES OF WALL THICKNESS. LINTELS SHALL BEAR 6 INCHES MINIMUM EACH END U.N.O.

M. PROVIDE TWO-PIECE ADJUSTABLE ANCHORS TO MASONRY AT A MAXIMUM SPACING OF 24 INCHES O.C. AT ALL VERTICAL AND HORIZONTAL STRUCTURAL STEEL MEMBERS.

N. CAVITY WALLS OF BRICK WITH STUD BACKUP SHALL BE CONSTRUCTED WITH TWO-PIECE ADJUSTABLE METAL ANCHORS AT A MAXIMUM SPACING OF 16 INCHES O.C. HORIZONTAL (INTO STUDS) AND 24 INCHES O.C. VERICAL. AT BRICK WALLS OVER 3G FEET HIGH, PROVIDE ANCHORS AT 16 INCHES O.C. HORIZONTAL AND VERTICAL.

4. STEEL

A. STEEL CONSTRUCTION SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE APPLICABLE BUILDING CODE AND SHALL CONFORM TO AISC 360. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING ASTM STANDARDS:

WIDE FLANGE SHAPES	A992 – GR50
STEEL PLATES, CHANNELS AND ANGLES	A36
STRUCTURAL PIPES	A53 – GR B
STRUCTURAL RECT/ROUND (HSS)	A500 - GR B
ANCHOR RODS (3/4" DIAM. OR LESS)	F1554 – 36 KSI
ANCHOR RODS (7/8" DIAM. OR GREATER)	F1554 – 55 KSI
HIGH-STRENGTH BOLTS	A325
HEADED STUDS	A108

B. BOLTED CONNECTIONS TO USE A325-TYPE N, HIGH STRENGTH BOLTS IN BEARING TYPE CONNECTIONS TIGHTENED TO A SNUG TIGHT CONDITION IN ACCORDANCE WITH RCSC SPECIFICATIONS.

C. SHOP CONNECTIONS TO BE WELDED OR BOLTED. FIELD CONNECTIONS TO BE BOLTED UNLESS OTHERWISE SHOWN. BOLT HOLES TO BE STANDARD ROUND HOLES (d+1/16 INCHES) UNLESS OTHERWISE NOTED. SHORT SLOTS SHALL BE PERMITTED NORMAL TO THE LOAD DIRECTION IN SLIP CRITICAL AND BEARING TYPE CONNECTIONS AS PER AISC REQUIREMENTS.

D. IF BEAM REACTIONS ARE DENOTED ON THE DRAWINGS, BEAM-WEB CONNECTIONS FOR SHEAR AT EACH END SHALL BE DETAILED TO SUPPORT THE LOADS SHOWN OR PROVIDE THE FOLLOWING MINIMUM NUMBER OF BOLTS, WHICHEVER IS GREATER. STIFFENED SEATS SHALL BE DETAILED TO SUPPORT THE LOADS SHOWN ON THE DRAWINGS OR THE MINIMUM FACTORED LOADS INDICATED BELOW, WHICHEVER IS GREATER.

	BEAM/WEB	STIFF. SEAT
W8 OR W10	2 BOLTS	20K
W12 OR W14	3 BOLTS	30K
W16 OR W18	4 BOLTS	40K
W21 OR W24	5 BOLTS	60K

E. STRUCTURAL STEEL SHALL BE GIVEN ONE SHOP COAT OF APPROVED SHOP PRIMER APPLIED TO CLEAN AND DRY SURFACES. DO NOT PAINT STEEL THAT WILL BE FIREPROOFED OR EMBEDDED IN CONCRETE.

F. STEEL BEAMS SHALL BE WELDED TO STEEL BEARING PLATES WITH 3 INCH LONG BY 1/4-INCH FILLET WELD EACH SIDE OF FLANGE (MINIMUM).

G. WELDING OF STRUCTURAL STEEL SHALL BE WITH E70XX ELECTRODES.

5. WOOD FRAMING (CONVENTIONAL/TYPE V)

A. FRAMING LUMBER FOR STUDS, HEADERS AND JOISTS SHALL BE HEM FIR #2, SPRUCE-PINE-FIR (SPF) #2, OR BETTER, WITH A MAXIMUM MOISTURE CONTENT OF 19-PERCENT, HAVING THE FOLLOWING MINIMUM PROPERTIES (BASED ON 2X12 MEMBERS):

BENDING STRESS "Fb"	= 850 PSI FOR SINGLE MEMBER USE
HORIZONTAL SHEAR "Fv"	= 135 PSI
COMPRESSION PERPENDICULAR TO GRAIN "Fc"	= 405 PSI
COMPRESSION PARALLEL TO GRAIN "Fc11"	= 1150 PSI
MODULUS OF ELASTICITY "E"	= 1,300,000 PSI

NOTE: SPF (SOUTH) IS NOT ACCEPTABLE.

B. ALL EXPOSED EXTERIOR FRAMING AND FRAMING IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE-TREATED (PT). FRAMING SHALL BE PRESSURE-TREATED WITH ALAKALINE COPPER QUAT (ACQ) OR COPPER AZOLE (CBA-A AND CA-B), NOT SODIUM BORATE (SBX). PT LUMBER SHALL NOT BE INCISED.

C. STRUCTURAL POSTS AND TREATED LUMBER (PT) SHALL BE SOUTHERN PINE (SP) #2 OR BETTER, HAVING THE FOLLOWING MINIMUM PROPERTIES (BASED ON 2X12 MEMBERS):

STRENGTH	
BENDING STRESS "Fb"	= 750 PSI SINGLE MEMBER USE
HORIZONTAL SHEAR "Fv"	= 175 PSI
COMPRESSION PERPENDICULAR TO GRAIN "Fc"	= 565 PSI
COMPRESSION PARALLEL TO GRAIN "Fc11"	= 1,250 PSI
MODULUS OF ELASTICITY "E"	= 1,400,000 PSI

D. LAMINATED VENEER LUMBER (LVL OR MICROLAM) BEAMS SHALL CONFORM TO ASTM D 5456 AND SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES (BASED ON 1-3/4 X 11-7/8 MEMBERS):

BENDING STRESS "Fb"	= 2600 PSI
HORIZONTAL SHEAR "Fv"	= 285 PSI
MODULUS OF ELASTICITY "E"	= 2,000,000 PSI
BEARING STRESS "FPERP"	= 750 PSI
EQUIV SPECIFIC GRAVITY FOR CONNECTION DESIGN	= 0.50

E. PARALLEL STRAND LUMBER (PSL) COLUMNS SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES:

BENDING STRESS "Fb"	= 2,400 PSI
HORIZONTAL SHEAR "Fv"	= 190 PSI
COMPRESSION, PARALLEL TO GRAIN "Fc11"	= 2,500 PSI
MODULUS OF ELASTICITY "E"	= 1,800,000 PSI

F. AT EXTERIOR WALLS, PROVIDE SOLID BLOCKING AT 4 FEET ON CENTER BETWEEN BAND JOIST AND FIRST INTERIOR PARALLEL JOIST.

G. PREFABRICATED JOIST HANGERS, BEAM HANGERS, POST CAPS, AND POST BASES SHALL BE SIZED AND ATTACHED PER MANUFACTURER'S RECOMMENDATIONS, TO ACHIEVE AT LEAST THE MINIMUM MANUFACTURER LISTED CAPACITIES, UNO ON THE DRAWINGS. FASTENERS AND CONNECTORS UTILIZED WITH TREATED LUMBER (PT OR FRT) SHALL MEET G185 HOT-DIPPED GALVANIZING.

H. ANCHOR BOLTS CONNECTING PRESSURE-TREATED WOOD PLATES TO FOUNDATIONS, MASONRY WALLS, OR CONCRETE SLABS SHALL BE HOT-DIPPED GALVANIZED.

I. BUILT-UP STUD COLUMNS SHALL HAVE ONE JACK STUD AND THE REMAINING STUDS SHALL BE KING STUDS. MULTIPLE STUDS SHALL BE NAILED WITH 10D NAILS AT 8 INCHES O.C. PROVIDE SOLID BLOCKING OR CRIPPLE STUDS IN FLOOR SYSTEM AT ALL POINT LOADS ABOVE.

J. FREESTANDING POSTS SHALL HAVE PREFAB POSTCAP AND BASE. POSTS WITHIN WALL NEED ONLY HAVE PREFAB CAP ATTACHED TO BEAM, UNO. POSTS WITHIN WALL BEARING ON MASONRY OR CONCRETE SHALL HAVE PREFAB BASE, UNO.

K. STANDARD MEMBER CONNECTIONS SHALL BE PER FASTENING SCHEDULE IN SECTION 23 OF THE INTERNATIONAL BUILDING CODE (IBC), UNO.

L. STUD BEARING WALLS TO BE PROVIDED WITH 2 CONTINUOUS TOP PLATES AND 1 CONTINUOUS BOTTOM PLATE WITH A MINIMUM OF ONE ROW OF HORIZONTAL BRIDGING AT MID-HEIGHT OF WALL UNLESS NOTED OTHERWISE. SPLICES OF TOP PLATES SHALL OCCUR OVER STUD AND SHALL BE STAGGERED A MINIMUM OF FOUR FEET.

M. NAILS FOR FRAMING AND SHEATHING CONNECTIONS SPECIFIED IN THE DRAWINGS AND ASSOCIATED NOTES SHALL CONFORM TO ASTM F1667 AND SHALL MEET THE FOLLOWING MINIMUM SIZE REQUIREMENTS:

TYPE	DIAMETER x LENGTH
8d	0.131" x 2-1/2"
10d	0.148" x 3"
12d	0.148" x 3-1/4"
16d	0.162" x 3-1/2"
20d	0.192" x 4"

SHANK DIAMETER	MINIMUM STRENGTH
0.099" TO 0.142"	100 KSI
0.143" TO 0.177"	90 KSI
0.178" TO 0.254"	80 KSI

NOTE: NAILS USED IN STANDARD CONNECTIONS SHALL BE SIZED PER THE REQUIREMENTS OF THE BUILDING CODE.

N. ROOF MEMBERS SHALL BE CONNECTED AT EACH BEARING POINT WITH ONE PREFABRICATED GALVANIZED METAL ANCHOR. ANCHORS SHALL BE 18 GAGE MINIMUM AND SHALL BE ATTACHED TO HAVE A CAPACITY TO RESIST A 450# UPLIFT LOADING, UNLESS SHOWN OTHERWISE ON DRAWINGS.

O. THE MINIMUM DEPTH AND MAXIMUM SPACING OF WOOD TRUSSES IS SHOWN ON DRAWINGS. THE SUPPLIER SHALL ADJUST SPACING AS REQUIRED TO MEET THE LOADINGS DESIGNATED BELOW.

P. PROVIDE LSL BAND BOARD IN WOOD TRUSS SYSTEMS AT ALL PERIMETER BEARING WALLS. ALTERNATIVELY, PROVIDE 2-3/4 INCH PLYWOOD BANDS GLUED AND SCREWED TOGETHER. PROVIDE SQUASH BLOCKS AND STIFFENERS AS REQUIRED TO DISTRIBUTE LOADINGS AND AS REQUIRED BY MANUFACTURER. PROVIDE SOLID BLOCKING AT INTERIOR TRUSS SUPPORTS WITH BEARING WALLS ABOVE.

Q. DO NOT SPLICE STRUCTURAL MEMBERS BETWEEN SUPPORTS.

R. PREFABRICATED TRUSSES SHALL BE DESIGNED FOR THE LOADS SCHEDULED ON THE DRAWINGS. SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR REVIEW. THE DESIGN OF THE BRACING REQUIRED TO LATERALLY STABILIZE THE TRUSSES AND TRUSS MEMBERS SHALL BE THE RESPONSIBILITY OF THE SPECIALTY TRUSS ENGINEER. AFFIX SEAL OF ENGINEER REGISTERED IN THE STATE OF THE PROPOSED PROJECT. TEMPORARY BRACING DURING ERECTION IS THE RESPONSIBILITY OF THE CONTRACTOR.

6. SHEATHING

A. FLOOR SHEATHING SHALL BE 23/32 (3/4) INCH APA RATED STURD-I (COMBINATION SUBFLOOR-UNDERLAYMENT) WOOD STRUCTURAL PANEL, TONGUE AND GROOVE, WITH SPAN RATING OF 48/24. PANELS SHALL HAVE LONG DIMENSION ORIENTED ACROSS THREE OR MORE JOISTS AND SHALL BE FASTENED WITH CONSTRUCTION ADHESIVE AND NAILS AT PANEL EDGES AND INTERMEDIATE SUPPORTS AS SCHEDULED ON THE DRAWINGS. UNLESS NOTED OTHERWISE, PANEL EDGES NEED NOT BE BLOCKED. INSTALL PER MANUFACTURER'S INSTALLATION INSTRUCTIONS.

B. EXTERIOR SHEATHING SHALL BE 7/16 (1/2) INCH APA RATED WOOD STRUCTURAL PANELS U.N.O. AS SHEAR WALL. FASTEN PANELS TO STUDS WITH 8d NAILS AT 6 INCHES ON CENTER AT INTERMEDIATE SUPPORTS. UNLESS NOTED OTHERWISE, PANEL EDGES NEED NOT BE BLOCKED. IF EXTERIOR WALLS ARE DENOTED AS SHEAR WALLS, THEY SHALL BE SHEATHED, FASTENED AND BLOCKED AS SCHEDULED ON THE DRAWINGS.

C. SHEARWALLS SHALL BE SHEATHED, FASTENED AND BLOCKED AS SCHEDULED ON THE DRAWINGS.

D. ROOF SHEATHING SHALL BE 23/32 (3/4) INCH APA RATED WOOD STRUCTURAL PANEL, TONGUE AND GROOVE, WITH SPAN RATING OF 48/24. PANELS SHALL HAVE LONG DIMENSION ORIENTED ACROSS THREE OR MORE JOISTS AND SHALL BE FASTENED WITH CONSTRUCTION ADHESIVE AND NAILS AT PANEL EDGES AND INTERMEDIATE SUPPORTS AS SCHEDULED ON THE DRAWINGS. UNLESS NOTED OTHERWISE, PANEL EDGES NEED NOT BE BLOCKED.

7. POST-INSTALLED ANCHORS IN CONCRETE AND MASONRY

A. GENERAL

INSTALL ANCHORS IN STRICT CONFORMANCE WITH THE MANUFACTURER'S PUBLISHED INSTALLATION INSTRUCTIONS AND PROCEDURES. ALL POST-INSTALLED ANCHORS IN CONCRETE SHALL HAVE ICC APPROVAL FOR USE IN CRACKED CONCRETE.

SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE SUBMITTED FOR APPROVAL PRIOR TO USE. CONTRACTOR SHALL PROVIDE LOAD CAPACITIES DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT.

PROVIDE STAINLESS STEEL FASTENERS FOR EXTERIOR USE OR WHEN PERMANENTLY EXPOSED TO WEATHER. PROVIDE GALVANIZED CARBON STEEL ANCHORS AT OTHER LOCATIONS, UNLESS OTHERWISE NOTED.

B. PRODUCTS

ANCHORS IN CONCRETE:
--EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT TZ.
--UNDERCUT ANCHORS SHALL BE HILTI HDA.
--SCREW ANCHORS SHALL BE HILTI KWIK HUS.
--ADHESIVE ANCHORS SHALL BE HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HIT-Z ROD OR WITH HILTI HOLLOW DRILL BIT SYSTEM WITH HAS-E THREADED ROD.

ANCHORS IN MASONRY:
--EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT TZ. GROUT MASONRY CELLS SOLID WITH 2000 PSI GROUT AT ANCHOR LOCATIONS.
--SCREW ANCHORS SHALL BE HILTI KWIK HUS. GROUT MASONRY CELLS SOLID WITH 2000 PSI GROUT AT ANCHOR LOCATIONS.
--ADHESIVE ANCHORS IN SOLID MASONRY SHALL BE HILTI HIT-HY 270 ADHESIVE ANCHORING SYSTEM. STEEL ANCHOR ELEMENT SHALL BE HILTI HAS-E CONTINUOUSLY THREADED ROD OR HILTI HIS-N INTERNALLY THREADED INSERT.
--ADHESIVE ANCHORS IN HOLLOW OR MULTI-WYTHE MASONRY SHALL BE HILTI HIT-HY 270 ADHESIVE ANCHORING SYSTEM. STEEL ANCHOR ELEMENT SHALL BE HILTI HAS-E CONTINUOUSLY THREADED ROD OR HILTI HIT-IC INTERNALLY THREADED INSERT. THE APPROPRIATE SIZE SCREEN TUBE SHALL BE USED PER THE ADHESIVE MANUFACTURER'S RECOMMENDATION.

C. INSTALLATION

ALL INSTALLATION PROCEDURES SHALL BE PER MANUFACTURERS RECOMMENDATIONS. COORDINATE AND/OR PROVIDE FOR THIRD PARTY INSPECTION AS REQUIRED BY BUILDING CODE OR LOCAL JURISDICTION.

ANCHOR CAPACITY IS DEPENDENT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHOR TO EDGE OF CONCRETE OR MASONRY. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE DISTANCE INDICATED ON THE DRAWINGS. IF NOT SHOWN, COMPLY WITH MINIMUM SPACING AND EDGE DISTANCE FOR FULL ANCHOR CAPACITY, AS SPECIFIED BY MANUFACTURER.

EXISTING REINFORCING BARS IN THE CONCRETE OR MASONRY STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. DO NOT CUT OR DAMAGE REINFORCING BARS UNLESS SPECIFICALLY PERMITTED IN THE DRAWINGS.

PRIOR TO DRILLING, THE CONTRACTOR SHALL LOCATE REINFORCING BAR POSITIONS IN THE IMMEDIATE VICINITY OF PROPOSED POST-INSTALLED ANCHORS USING GPR, X-RAY, OR OTHER NON-DESTRUCTIVE MEANS.

WHEN CONFLICTS BETWEEN PROPOSED ANCHORS AND EXISTING REINFORCING BARS EXIST, SUBMIT RESULTS OF BAR LOCATIONS TO ARCHITECT / ENGINEER FOR REVIEW AND FURTHER DIRECTION.

WARNING: THE STRUCTURAL INTEGRITY OF THE BUILDING SHOWN ON THESE PLANS IS DEPENDENT UPON COMPLETION ACCORDING TO PLANS AND SPECIFICATIONS. STRUCTURAL MEMBERS ARE NOT SELF-BRACING UNTIL PERMANENTLY AFFIXED TO THE STRUCTURE. THE STRUCTURAL ENGINEERS ASSUME NO LIABILITY FOR THE STRUCTURE DURING CONSTRUCTION.

WARNING: THE STRUCTURAL INTEGRITY OF THE BUILDING SHOWN ON THESE PLANS IS DEPENDENT UPON COMPLETION ACCORDING TO PLANS AND SPECIFICATIONS. STRUCTURAL MEMBERS ARE NOT SELF-BRACING UNTIL PERMANENTLY AFFIXED TO THE STRUCTURE. THE STRUCTURAL ENGINEERS ASSUME NO LIABILITY FOR THE STRUCTURE DURING CONSTRUCTION.

ABBREVIATIONS & LEGEND

A		K	
AB	ANCHOR BOLT	K	KIP
ADDL	ADDITIONAL	KO	KNOCK-OUT
ADJ	ADJACENT	KSI	KIPS PER SQ. INCH
AFF	ABOVE FINISH FLOOR		
ALT	ALTERNATE	L	
APPROX	APPROXIMATE(LY)	L	LINTEL MARK
ARCH	ARCHITECT(URAL)	LLH	LONG LEG HORIZONTAL
		LLV	LONG LEG VERTICAL
B		LL	LIVE LOAD
B	BEAM MARK	LP	LOW POINT
BF	BOTTOM OF FOOTING ELEVATION	LVL	LAMINATED VENEER LUMBER
	BRACED FRAME MARK		
BLKG	BLOCKING	M	
BLDG	BUILDING	MANUF	MANUFACTURER(ED)
BM	BEAM	MAS	MASONRY
BOD	BOTTOM OF DECK	MAX	MAXIMUM
BOS	BOTTOM OF STEEL	MIN	MINIMUM
BOTT	BOTTOM	MISC	MISCELLANEOUS
BP	BEARING PLATE MARK	MO	MASONRY OPENING
BRG	BEARING	MATL	MATERIAL
BSMT	BASEMENT	MTL	METAL
BTWN	BETWEEN		
		N	
C		NTS	NOT TO SCALE
C	COLUMN MARK	NS	NEAR SIDE
CIP	CAST IN PLACE	NIC	NOT IN CONTRACT
CJ	CONTROL/CONSTRUCTION JOINT		
CLR	CLEAR(ANCE)	O	
CMU	CONCRETE MASONRY UNIT	OC	ON CENTER(S)
COL	COLUMN	OPNG	OPENING
COM	CENTER OF MASONRY WALL	OPP	OPPOSITE
COMP	COMPOSITE	OF	OUTSIDE FACE
CONC	CONCRETE		
CONN	CONNECTION	P	
CONST	CONSTRUCTION	P	PIER MARK
CONT	CONTINUOUS	PC	PRECAST CONCRETE
COORD	COORDINATE(TION)	PDF	POWER DRIVEN FASTENER
COS	CENTER OF STUD	PEB	PRE-ENGINEERED BUILDING
		PERIM	PERIMETER
D		PL	PLATE
DBA	DEFORMED BAR ANCHORS	PLF	POUNDS PER LINEAR FOOT
DTL	DETAIL	PP	PRECAST PLANK MARK
DIAM	DIAMETER	PROJ	PROJECTION
DIAG	DIAGONAL	PSF	POUNDS PER SQ. FOOT
DN	DOWN	PSI	POUNDS PER SQ. INCH
DWG	DRAWING	PSL	PARALLEL STRAND LUMBER COLUMN
DBL	DOUBLE	PT	POST TENSION(ED)/PRESSURE TREATED
DL	DEAD LOAD		
		Q	
E		QTY	QUANTITY
EA	EACH		
EE	EACH END	R	
EF	EACH FACE	RAD	RADIUS
EL	ELEVATION	RD	ROOF DRAIN
ELEV	ELEVATOR	REV	REVISION, REVISE(D)
EOD	EDGE OF DECK	REINF	REINFORCE(D), (ING)
EOJ	EDGE OF JOIST	REM	REMAINDER
EOS	EDGE OF SLAB	REQ'D	REQUIRED
EQ	EQUAL	RTU	ROOF TOP UNIT
EQUIP	EQUIPMENT		
ES	EACH SIDE	S	
EW	EACH WAY	SB	SOIL BORING
EXIST, EX	EXISTING	SC	SLIP CRITICAL
EXP	EXPANSION	SE	SPECIALTY DESIGN ENGINEER
EXT	EXTERIOR	SIM	SIMILAR
		SJI	STEEL JOIST INSTITUTE
F		SOG	SLAB ON GRADE
F	FOOTING MARK	SQ	SQUARE
FD	FLOOR DRAIN	STD	STANDARD
FDN	FOUNDATION	STL	STEEL
FOB	FACE OF BUILDING	STRUCT	STRUCTURAL
FOM	FACE OF MASONRY WALL	SPA	SPACES
FOS	FACE OF STUD	SL	SNOW LOAD
FS	FOOTING STEP	SS	STAINLESS STEEL
FTG	FOOTING		
FUT	FUTURE	T	
		TEMP	TEMPORARY
G		TF	TOP OF FOOTING ELEVATION
GA	GAGE, GAUGE	THK	THICK(NESS), (ENED)
GALV	GALVANIZED	TJI	WOOD I JOIST
GC	GENERAL CONTRACT(OR)	TO	THROUGH OUT
GT	GIRDER TRUSS	TOC	TOP OF CONCRETE
		TOP	TOP OF PIER ELEVATION
H		TOS	TOP OF STEEL ELEV

30x42 SCHEDULE OF SPECIAL INSPECTIONS

		VERIFICATION AND INSPECTION	Y/N	FREQUENCY CONTINUOUS	FREQUENCY PERIODIC	REFERENCED STANDARD	IBC REFERENCE	SCOPE OF SERVICE	RESPONSIBLE PARTY
		STRUCTURAL STEEL					1705.2	-	
1.		HIGH-STRENGTH BOLTING: INSPECTION TASKS PRIOR TO BOLTING							
	A.	MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS.	Y	X	-	-	1705.2	-	SIER
	B.	FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	Y	-	X	AISC 360 & applicable ASTM material standards	1705.2	-	SIER
	C.	PROPER FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE).	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS ONLY	SIER
	D.	PROPER BOLTING PROCEDURE SELECTED FOR JOINT DETAIL.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS ONLY	SIER
	E.	CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION & HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS ONLY	SIER
	F.	PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED & DOCUMENTED FOR FASTENER ASSEMBLIES & METHODS USED.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS ONLY	SIER
	G.	PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS, & OTHER FASTENER COMPONENTS.	Y	-	X	AISC 360	1705.2	-	SIER
2.		HIGH-STRENGTH BOLTING: INSPECTION TASKS DURING BOLTING							
	A.	FASTENER ASSEMBLIES, OF SUITABLE CONDITION, PLACED IN ALL HOLES & WASHERS (IF REQUIRED) ARE POSITIONED AS REQUIRED.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS USING CALIBRATED WRENCH METHOD OR TURN-OF-NUT WITHOUT MATCHMARKING ONLY	SIER
	B.	JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO PRETENSIONING OPERATION.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS USING CALIBRATED WRENCH METHOD OR TURN-OF-NUT WITHOUT MATCHMARKING ONLY	SIER
	C.	FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS USING CALIBRATED WRENCH METHOD OR TURN-OF-NUT WITHOUT MATCHMARKING ONLY	SIER
	D.	FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES.	Y	-	X	AISC 360	1705.2	FOR PRETENSIONED & SLIP CRITICAL JOINTS USING CALIBRATED WRENCH METHOD OR TURN-OF-NUT WITHOUT MATCHMARKING ONLY	SIER
3.		HIGH-STRENGTH BOLTING: INSPECTION TASKS AFTER BOLTING							
	A.	DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS.	Y	X	-	AISC 360	1705.2	-	SIER
4.		WELDING: INSPECTION TASKS PRIOR TO WELDING							
	A.	WELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE.	Y	X	X	AISC 360 & applicable AWS Documents	1705.2	ESTABLISH THE JOINT WELDING PROCEDURES ARE PREQUALIFIED OR TEST IN ACCORDANCE WITH AWS D1.1 QUALIFICATION PROCEDURES	SIER
	B.	MANUFACTURER'S CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE.	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	C.	MATERIAL IDENTIFICATION (TYPE/GRADE).	Y	-	X	AISC 360	1705.2	-	SIER
	D.	FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY):							
		1) JOINT PREPARATION	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		2) DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		3) CLEANLINESS (CONDITION OF STEEL SURFACES)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		4) TACKING (TACK WELD QUALITY & LOCATION)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		5) BACKING TYPE & FIT (IF APPLICABLE)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	E.	CONFIGURATION & FINISH OF ACCESS HOLES	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	F.	FIT-UP OF FILLET WELDS:							
		1) DIMENSIONS (ALIGNMENT, GAPS AT ROOT)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		2) CLEANLINESS (CONDITION OF STEEL SURFACES)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
		3) TACKING (TACK WELD QUALITY & LOCATION)	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
5.		WELDING: INSPECTION TASKS DURING WELDING							
	A.	USE OF QUALIFIED WELDERS	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	WELDERS MUST BE CURRENTLY CERTIFIED UNDER AMERICAN WELDING SOCIETY QUALIFICATION PROCEDURES.	SIER
	B.	CONTROL & HANDLING OF WELDING CONSUMABLES INCLUDING PACKAGING & EXPOSURE CONTROL	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	C.	ENVIRONMENTAL CONDITIONS: WIND SPEED WITHIN LIMITS, PRECIPITATION, & TEMPERATURE.	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	D.	WPS FOLLOWED: SETTINGS ON WELDING EQUIPMENT, TRAVEL SPEED, SELECTED WELDING MATERIALS, SHIELDING GAS TYPE/FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED (MIN./MAX.), PROPER POSITION (F,V,H,OH).	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	E.	WELDING TECHNIQUES: INTERPASS & FINAL CLEANING, EACH PASS WITHIN PROFILE LIMITATIONS, EACH PASS MEETS QUALITY REQUIREMENTS.	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
6.		WELDING: INSPECTION TASKS AFTER WELDING							
	A.	WELDS CLEANED	Y	-	X	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	B.	SIZE, LENGTH, & LOCATION OF WELDS	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	C.	WELDS MEET VISUAL ACCEPTANCE CRITERIA: CRACK PROHIBITION, WELD/BASE-METAL FUSION, CRATER CROSS SECTION, WELD PROFILES, WELD SIZE, UNDERCUT, POROSITY	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	PERFORM ULTRASONIC TESTING OF ALL FULL PENETRATION FIELD & SHOP WELDS TO COMPLY WITH ASTM E 164 PER PROJECT SPECIFICATIONS.	SIER
	D.	ARC STRIKES	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	E.	K-AREA	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	WHEN WELDING DOUBLER PLATES, CONTINUITY PLATES, OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 IN. OF THE WELD.	SIER
	F.	BACKING REMOVED & WELD TABS REMOVED (IF REQUIRED)	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	G.	REPAIR ACTIVITIES	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
	H.	DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	Y	X	-	AISC 360 & applicable AWS Documents	1705.2	-	SIER
7.		STEEL ELEMENT OF COMPOSITE CONSTRUCTION PRIOR TO CONCRETE PLACEMENT							
	A.	PLACEMENT & INSTALLATION OF STEEL DECK	N	-	-	AWS D1.3 AISC 360	1705.2	-	-
	B.	PLACEMENT & INSTALLATION OF STEEL HEADED STUD ANCHORS	N	-	-	AWS D1.1 AISC 360	1705.2	-	-
	C.	DOCUMENT ACCEPTANCE OR REJECTION OF STEEL ELEMENTS	N	-	-	AISC 360	1705.2	-	-
8.		INSPECTION OF FABRICATORS & FABRICATION PROCEDURES	N	-	-	AISC 360	1704.2.5	-	-
		COLD-FORMED METAL DECK							
1.		VERIFY COMPLIANCE OF MATERIALS (ALL DECK AND ACCESSORIES) WITH CONSTRUCTION DOCUMENTS, INCLUDING PROFILES, MATERIAL PROPERTIES, AND BASE METAL THICKNESS AND DOCUMENT ACCEPTANCE OR REJECTION	N	-	-	SDI QA/QC	1705.2.2	-	-
2.		VERIFY COMPLIANCE OF INSTALLATION OF ALL DECK AND ACCESSORIES WITH CONSTRUCTION DOCUMENTS AND VERIFY ACCEPTANCE OR REJECTION	N	-	-	SDI QA/QC	1705.2.2	-	-
3.		VERIFY DECK MATERIALS ARE REPRESENTED BY THE MILL CERTIFICATIONS THAT COMPLY WITH THE CONSTRUCTION DOCUMENTS	N	-	-	SDI QA/QC	1705.2.2	-	-
4.		DECK WELDING							
	A.	WELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE.	N	-	-	SDI QA/QC & applicable AWS Documents		-	-
	B.	MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	C.	MATERIAL IDENTIFICATION (TYPE/GRADE).	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	D.	CHECK WELDING EQUIPMENT	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	E.	USE OF QUALIFIED WELDERS	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	F.	CONTROL & HANDLING OF WELDING CONSUMABLES	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	G.	ENVIRONMENTAL CONDITIONS: WIND SPEED WITHIN LIMITS, PRECIPITATION, & TEMPERATURE.	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	H.	WPS FOLLOWED: SETTINGS ON WELDING EQUIPMENT, TRAVEL SPEED, SELECTED WELDING MATERIALS, SHIELDING GAS TYPE/FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED (MIN./MAX.), PROPER POSITION (F,V,H,OH).	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	I.	VERIFY SIZE AND LOCATION OF WELDS, INCLUDING SUPPORT, SIDELAP, AND PERIMETER WELDS	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	J.	WELDS MEET VISUAL ACCEPTANCE CRITERIA: CRACK PROHIBITION, WELD/BASE-METAL FUSION, CRATER CROSS SECTION, WELD PROFILES, WELD SIZE, UNDERCUT, POROSITY	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	K.	VERIFY REPAIR ACTIVITIES	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
	L.	DOCUMENT ACCEPTANCE OR REJECTION OF WELDS	N	-	-	SDI QA/QC & applicable AWS Documents	1705.2.2	-	-
5.		DECK MECHANICAL FASTENING							
	A.	MANUFACTURER INSTALLATION INSTRUCTIONS AVAILABLE FOR MECHANICAL FASTENERS	N	-	-	SDI QA/QC	1705.2.2	-	-
	B.	PROPER TOOLS AVAILABLE FOR FASTENER INSTALLATION	N	-	-	SDI QA/QC	1705.2.2	-	-
	C.	PROPER STORAGE FOR MECHANICAL FASTENERS	N	-	-	SDI QA/QC	1705.2.2	-	-
	D.	FASTENERS ARE POSITIONED AS REQUIRED	N	-	-	SDI QA/QC	1705.2.2	-	-
	E.	FASTENERS ARE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS	N	-	-	SDI QA/QC	1705.2.2	-	-
	F.	CHECK SPACING, TYPE, AND INSTALLATION OF SUPPORT FASTENERS.	N	-	-	SDI QA/QC	1705.2.2	-	-
	G.	CHECK SPACING, TYPE, AND INSTALLATION OF SIDELAP FASTENERS.	N	-	-	SDI QA/QC	1705.2.2	-	-
	H.	CHECK SPACING, TYPE, AND INSTALLATION OF PERIMETER FASTENERS.	N	-	-	SDI QA/QC	1705.2.2	-	-
	I.	VERIFY REPAIR ACTIVITIES	N	-	-	SDI QA/QC	1705.2.2	-	-
	J.	DOCUMENT ACCEPTANCE OR REJECTION OF MECHANICAL FASTENERS	N	-	-	SDI QA/QC	1705.2.2	-	-
		OPEN-WEB STEEL JOISTS AND JOIST GIRDERS							
1.		END CONNECTIONS - WELDING OR BOLTED	N	-	-	SJI Specifications	1705.2.3	-	-
2.		BRIDGING - HORIZONTAL OR DIAGONAL							
	A.	STANDARD BRIDGING	N	-	-	SJI Specifications	1705.2.3	-	-
	B.	BRIDGING THAT DIFFERS FROM THE SJI SPECIFICATIONS	N	-	-	Construction documents & approved shop drawings	1705.2.3	-	-
		CONCRETE							
1.		INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, & PLACEMENT.	Y	-	X	ACI 318	1705.3	VERIFY SIZE, LOCATION, SPACING ORIENTATION, COVER, SPLICING, & CONFORMANCE WITH THE CONTRACT DOCUMENTS, AS SUPPLEMENTED WITH APPROVED SHOP DRAWINGS OR OTHER SUBMITTALS. CONFIRM THAT THE SURFACE OF THE REINFORCING STEEL IS FREE OF FORM RELEASE OIL OR OTHE	SIER
2.		INSPECTION OF REINFORCING STEEL WELDING.							
	A.	VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706.	N	-	-	AWS D1.4 ACI 318	1705.3	-	-
	B.	INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"	N	-	-	AWS D1.4 ACI 318	1705.3	-	-
	C.	INSPECT ALL OTHER WELDS	N	-	-	AWS D1.4 ACI 318	1705.3	-	-

30x42 SCHEDULE OF SPECIAL INSPECTIONS

		VERIFICATION AND INSPECTION	Y/N	FREQUENCY CONTINUOUS	FREQUENCY PERIODIC	REFERENCED STANDARD	IBC REFERENCE	SCOPE OF SERVICE	RESPONSIBLE PARTY
3.		INSPECT ANCHORS CAST-IN CONCRETE.	Y	-	X	ACI 318	1705.3	PRECISE LOCATION OF ANCHOR RODS IS NOT EXPECTED BUT VERIFY THE CONTRACTOR HAS TAKEN APPROPRIATE STEPS TO CORRECTLY POSITION THEM.SUCH AS ENGAGING A SURVEYOR OR SETTING UP A SYSTEM OF STRING LINES & BATTER BOARDS & THAT CORRECT GRADE & SIZE OF ANCHORS IS	SIER
4.		INSPECTION OF ANCHORS POST-INSTALLED IN HARDENED CONCRETE.							
	A.	ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS	Y	X	-	ACI 318		0	
	B.	MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED ABOVE	Y	-	X	ACI 318		0	
5.		VERIFYING USE OF REQUIRED DESIGN MIX.	Y	-	X	ACI 318	1705.3	VERIFY APPROVED MIX DESIGN	SIER
6.		AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM, SLUMP & AIR CONTENT TESTS, & DETERMINE THE TEMPERATURE OF THE CONCRETE.	Y	X	-	ASTM C 172/ASTM C 31/ACI 318	1705.3	TEST IN ACCORDANCE WITH PROJECT SPECIFICATIONS, BUT NOT LESS THAN ONCE PER DAY PER CLASS OF CONCRETE OR ONCE PER 150 CUBIC YARDS PER DAY OR ONCE PER 5,000 SQUARE FEET OF SLAB OR WALL PER DAY. CYLINDERS MUST BE PROPERLY HANDLED & STORED ON SITE UNTIL TRANS	SIER
7.		INSPECTION OF CONCRETE & SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	Y	X	-	ACI 318	1705.3	VERIFY CONFORMANCE WITH PROJECT SPECIFICATIONS. INSPECTOR SHALL BE WHERE THE CONCRETE IS BEING PLACED RATHER THAN WHERE CONCRETE TRUCKS ARE DISCHARGING THEIR LOADS. INSPECTOR NEEDS TO BE PRESENT WHILE SLAB IS BEING FLOATED & POWER TROWELED.	SIER
8.		INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE & TECHNIQUES.	Y	-	X	ACI 318	1705.3	VERIFY CONFORMANCE WITH PROJECT SPECIFICATIONS & ACI	SIER
9.		INSPECTION OF PRESTRESSED CONCRETE:							
	A.	APPLICATION OF PRESTRESSING FORCES.	N	-	-	ACI 318	1705.3	-	-
	B.	GROUTING OF BONDED PRESTRESSING TENDONS.	N	-	-	ACI 318	1705.3	-	-
10.		ERECTION OF PRECAST CONCRETE MEMBERS.	N	-	-	ACI 318	1705.3	-	-
11.		VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POSTTENSIONED CONCRETE & PRIOR TO REMOVAL OF SHORES & FORMS FROM BEAMS & STRUCTURAL SLABS.	N	-	-	ACI 318	1705.3	-	-
12.		INSPECT FORMWORK FOR SHAPE, LOCATION & DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	N	-	-	ACI 318	1705.3	-	-
		MASONRY							
		LEVEL A QUALITY ASSURANCE: RISK CATEGORY I, II, OR III STRUCTURES DESIGNED IN ACCORDANCE WITH PART 4 OR APPENDIX A.	Y				1705.4		
		LEVEL B QUALITY ASSURANCE: RISK CATEGORY IV STRUCTURES DESIGNED IN ACCORDANCE WITH CHAPTERS 12 OR 13 & RISK CATEGORY I, II, OR III STRUCTURES DESIGNED IN ACCORDANCE WITH CHAPTERS OTHER THAN THOSE IN PART 4 OR APPENDIX A.	N				1705.4		
		LEVEL C QUALITY ASSURANCE: RISK CATEGORY IV STRUCTURES DESIGNED IN ACCORDANCE WITH CHAPTERS OTHER THAN PART 4 OR APPENDIX A.	N				1705.4		
1.		PRIOR TO CONSTRUCTION VERIFY CERTIFICATES OF COMPLIANCE USED IN MASONRY CONSTRUCTION AND DURING CONSTRUCTION COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS & THE APPROVED SUBMITTALS SHALL BE VERIFIED.	Y	-	X	ACI530.1	1705.4	VERIFY COMPLIANCE WITH APPROVED SHOP DRAWINGS.	SIER
2.		VERIFICATION OF F'M & F'AAC PRIOR TO CONSTRUCTION (AND FOR EVERY 5,000 SQUARE FEET DURING CONSTRUCTION FOR LEVEL C)	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
3.		VERIFICATION OF PROPORTIONS OF MATERIALS IN PREMIXED OR PREBLENDED MORTAR, PRESTRESSING GROUT, & GROUT OTHER THAN SELF-CONSOLIDATING GROUT AS DELIVERED TO THE SITE OR PROPORTIONS OF SITE PREPARED MORTAR.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
4.		VERIFICATION OF SLUMP FLOW & VSI AS DELIVERED TO THE SITE FOR SELF-CONSOLIDATING GROUT.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
5.		THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:							
	A.	PROPORTIONS OF SITE-PREPARED MORTAR, GROUT, & PRESTRESSING GROUT FOR BONDED TENDONS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	B.	PLACEMENT OF MASONRY UNITS & CONSTRUCTION OF MORTAR JOINTS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	C.	GRADE, TYPE, & SIZE OF REINFORCEMENT, ANCHOR BOLTS, PRESTRESSING TENDONS, & ANCHORAGES	N	-	-	TMS 402/ACI 530/ASCE 5/TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	D.	PLACEMENT OF REINFORCEMENT, CONNECTORS & PRESTRESSING TENDONS & ANCHORAGES.	N	-	-	TMS 402/ACI 530/ASCE 5/TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	E.	GROUT SPACE PRIOR TO GROUTING.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	F.	PLACEMENT OF GROUT & PRESTRESSING GROUT FOR BONDED TENDONS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	G.	SIZE & LOCATION OF STRUCTURAL ELEMENTS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	H.	TYPE, SIZE & LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION.	N	-	-	TMS 402/ACI 530/ASCE 5	1705.4	-	-
	I.	WELDING OF REINFORCEMENT.	N	-	-	TMS 402/ACI 530/ASCE 5	1705.4	-	-
	J.	PREPARATION, CONSTRUCTION & PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F).	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	K.	APPLICATION & MEASUREMENT OF PRESTRESSING FORCE.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	L.	PLACEMENT OF AAC MASONRY UNITS & CONSTRUCTION OF THIN-BED MORTAR JOINTS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	M.	PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
	N.	VERIFY PRE-STRESSING TECHNIQUE IS IN COMPLIANCE AS CONSTRUCTION BEGINS	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	
6.		OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS.	N	-	-	TMS 602/ACI 530.1/ASCE 6	1705.4	-	-
		WOOD CONSTRUCTION							
1.		INSPECTION OF FABRICATORS & FABRICATION PROCEDURES FOR PREFABRICATED WOOD STRUCTURAL ELEMENTS.	Y	-	X		1705.5	-	SIER
2.		HIGH-LOAD DIAPHRAGMS DESIGNED IN ACCORDANCE WITH SECTION 2306.2	N	-	X		1705.5	-	SIER
3.		METAL PLATE CONNECTED WOOD TRUSSES SPANNING 60 FEET OR MORE	N	-	-		1705.5	-	-
		SOILS						-	
1.		VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	Y	-	X	Geotechnical Report	1705.6	-	SIER
2.		VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH & HAVE REACHED PROPER MATERIAL.	Y	-	X	Geotechnical Report	1705.6	-	SIER
3.		PERFORM CLASSIFICATION & TESTING OF COMPACTED FILL MATERIALS.	Y	-	X	Geotechnical Report	1705.6	-	SIER
4.		VERIFY USE OF PROPER MATERIALS, DENSITIES, & LIFT THICKNESSES DURING PLACEMENT & COMPACTION OF COMPACTED FILL.	Y	X	-	Geotechnical Report	1705.6	TEST FREQUENCY PER SPECIFICATIONS, BUT NOT LESS THAN ONE TEST EVERY 2,000 SQUARE FEET FOR EACH LAYER OF FILL OR PROOF-ROLLING	SIER
5.		PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE & VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	Y	-	X	Geotechnical Report	1705.6	-	SIER
		DRIVEN DEEP FOUNDATIONS							
1.		VERIFY ELEMENT MATERIALS, SIZES, & LENGTHS COMPLY WITH THE REQUIREMENTS.	N	-	-		1705.7	-	-
2.		DETERMINE CAPACITIES OF TEST ELEMENTS & CONDUCT ADDITIONAL LOAD TESTS, AS REQUIRED.	N	-	-		1705.7	-	-
3.		OBSERVE DRIVING OPERATIONS & MAINTAIN COMPLETE & ACCURATE RECORDS FOR EACH ELEMENT.	N	-	-		1705.7	-	-
4.		VERIFY PLACEMENT LOCATIONS & PLUMBNESS, CONFIRM TYPE & SIZE OF HAMMER, RECORD NUMBER OF BLOWS PER FOOT OF PENETRATION, DETERMINE REQUIRED PENETRATIONS TO ACHIEVE DESIGN CAPACITY, RECORD TIP & BUTT ELEVATIONS, & DOCUMENT ANY DAMAGE TO FOUNDATION ELEMENT.	N	-	-		1705.7	-	-
5.		PERFORM ADDITIONAL INSPECTIONS FOR STEEL ELEMENTS PER STEEL INSPECTION REQUIREMENTS.	N	-	-		1705.7	-	-
6.		PERFORM ADDITIONAL INSPECTIONS FOR CONCRETE & CONCRETE-FILLED ELEMENTS PER CONCRETE INSPECTION REQUIREMENTS.	N	-	-		1705.7	-	-
7.		PERFORM ADDITIONAL INSPECTIONS FOR SPECIALTY ELEMENTS AS DETERMINED BY THE REGISTERED DESIGN PROFESSIONAL IN CHARGE.	N	-	-		1705.7	-	-
		CAST-IN-PLACE DEEP FOUNDATIONS							
1.		OBSERVE DRILLING OPERATIONS & MAINTAIN COMPLETE & ACCURATE RECORDS FOR EACH ELEMENT.	N	-	-		1705.8	-	-
2.		VERIFY PLACEMENT LOCATIONS & PLUMBNESS, CONFIRM ELEMENT DIAMETERS, BELL DIAMETERS (IF APPLICABLE), LENGTHS, EMBEDMENT INTO BEDROCK (IF APPLICABLE), & ADEQUATE END-BEARING STRATA CAPACITY. RECORD CONCRETE OR GROUT VOLUMES.	N	-	-		1705.8	-	-
3.		PERFORM ADDITIONAL INSPECTIONS FOR CONCRETE ELEMENTS PER CONCRETE INSPECTION REQUIREMENTS.	N	-	-		1705.8	-	-
		HELICAL PILE FOUNDATIONS							
1.		RECORD INSTALLATION EQUIPMENT USED, PILE DIMENSIONS, TIP ELEVATIONS, FINAL DEPTH, & FINAL INSTALLATION TORQUE.	N	-	-	Geotechnical Report & Approved Shop Drawings	1705.9	-	-
		SPRAYED FIRE-RESISTANT MATERIALS						-	
1.		CONDITIONS OF SUBSTRATES	N	-	-	Approved fire-resistance design	1705.14	-	-
2.		THICKNESS OF APPLICATION	N	-	-	Approved fire-resistance design	1705.14	-	-
3.		DENSITY IN POUNDS PER CUBIC FOOT	N	-	-	Approved fire-resistance design	1705.14	-	-
4.		BOND STRENGTH ADHESION /COHESION	N	-	-	Approved fire-resistance design	1705.14	-	-
5.		CONDITION OF FINISHED APPLICATION	N	-	-	Approved fire-resistance design	1705.14	-	-
6.		FIRE-RESISTANT PENETRATIONS & JOINTS	N	-	-	Approved fire-resistance design	1705.17	-	-
		LIGHT GAGE METAL FRAMING	N	-	-	Approved shop drawings	-	-	-
		MASTIC & INTUMESCENT FIRE-RESISTANT COATINGS	N	-	-	AWCI-12B & approved fire-resistance design	1705.15	-	-
		EXTERIOR INSULATION & FINISH SYSTEMS (EIFS)	N	-	-	-	1705.16	-	-

*	INSPECTION AGENTS	NAME	COMPANY	ADDRESS
	1. SPECIAL INSPECTION ENGINEER OF RECORD (SIER)	1.	1.	1.
	2. INSPECTION & TESTING AGENCY (ITA)	2.	2.	2.
	3. GEOTECHNICAL ENGINEER OF RECORD (GER)	3.	3.	3.
	4. SPECIALTY ENGINEER (SE)	4.	4.	4.
	5. STRUCTURAL ENGINEER OF RECORD (SER)	5. THOMAS A. BOUFFARD, PE	5. EHLERT BRYAN	5. 8609 WESTWOOD CENTER DRIVE, SUITE 800, TYSONS, VA 22182

ALABAMA LICENSED No. 22545 PROFESSIONAL ENGINEER THOMAS A. BOUFFARD

REVISIONS
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PERMIT SET

TERRACES AT HIGH MOUNTAIN
4130 HIGH MOUNTAIN ROAD NE
HUNTSVILLE, AL 35811
06/11/2021

S7.002
SPECIAL INSPECTIONS
01/19/21

3 NORTH

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KEY PLAN
SCALE: 1" = 50'-0"

ALABAMA LICENSED No. 22545 PROFESSIONAL ARCHITECT THOMAS A. BOUFFARD

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TERRACES AT HIGH MOUNTAIN
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S7.003
KEY PLAN
03/03/21

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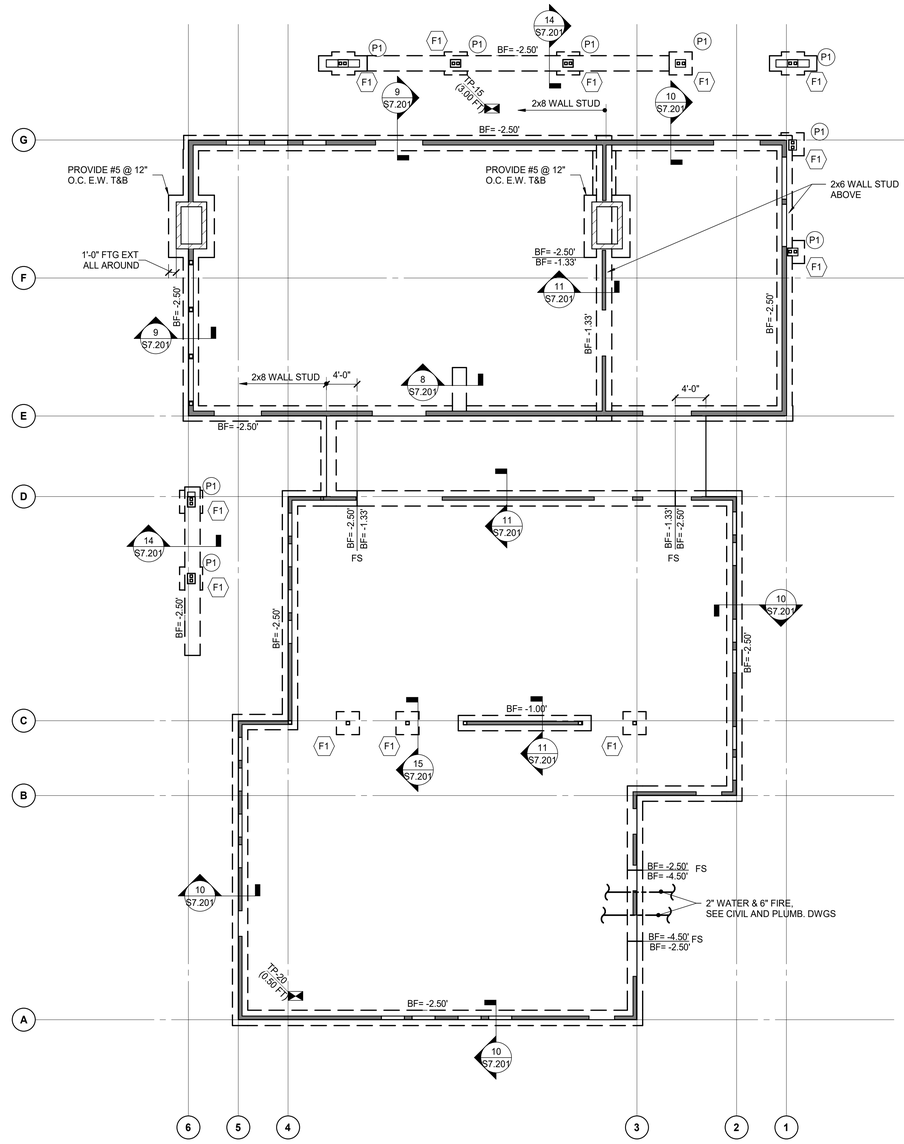

FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. FLOOR CONSTRUCTION: 4'' CONCRETE SLAB-ON-GRADE REINFORCED WITH ONE LAYER OF 6X6-W1.4 X W1.4 WWF IN THE TOP 1/3 OF SLAB PLACED OVER 10 MIL VAPOR RETARDER ON 4'' LAYER OF COMPACTED #57 STONE.
2. TOP OF CONCRETE ELEVATION = 0.0' REFERENCE (ACTUAL ELEVATION= 1012.40')
3. FS DENOTES FOOTING STEP, SEE 2/S7.201
4. TP-X DENOTES APPROXIMATE TEST PIT LOCATION. (XX') DENOTES ESTIMATED UNDERCUTS TO SUITABLE SOILS.

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06/11/2021

S7.100
FOUNDATION PLAN

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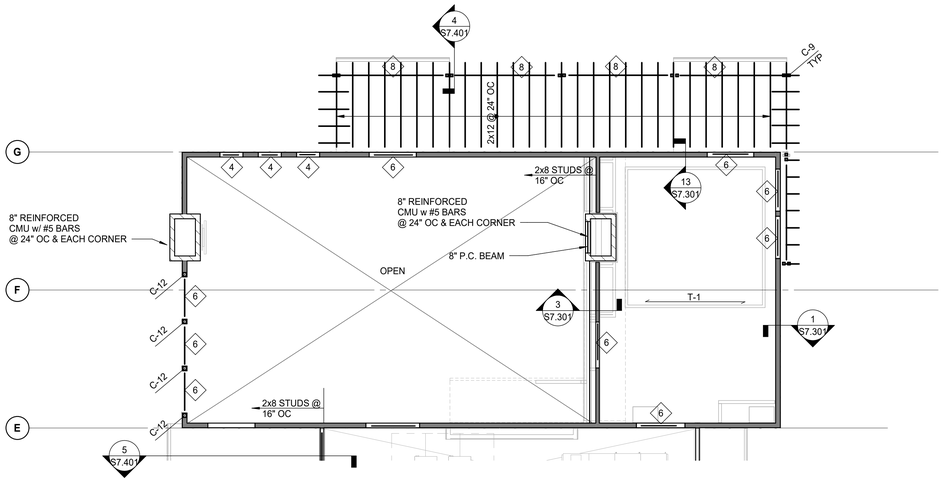


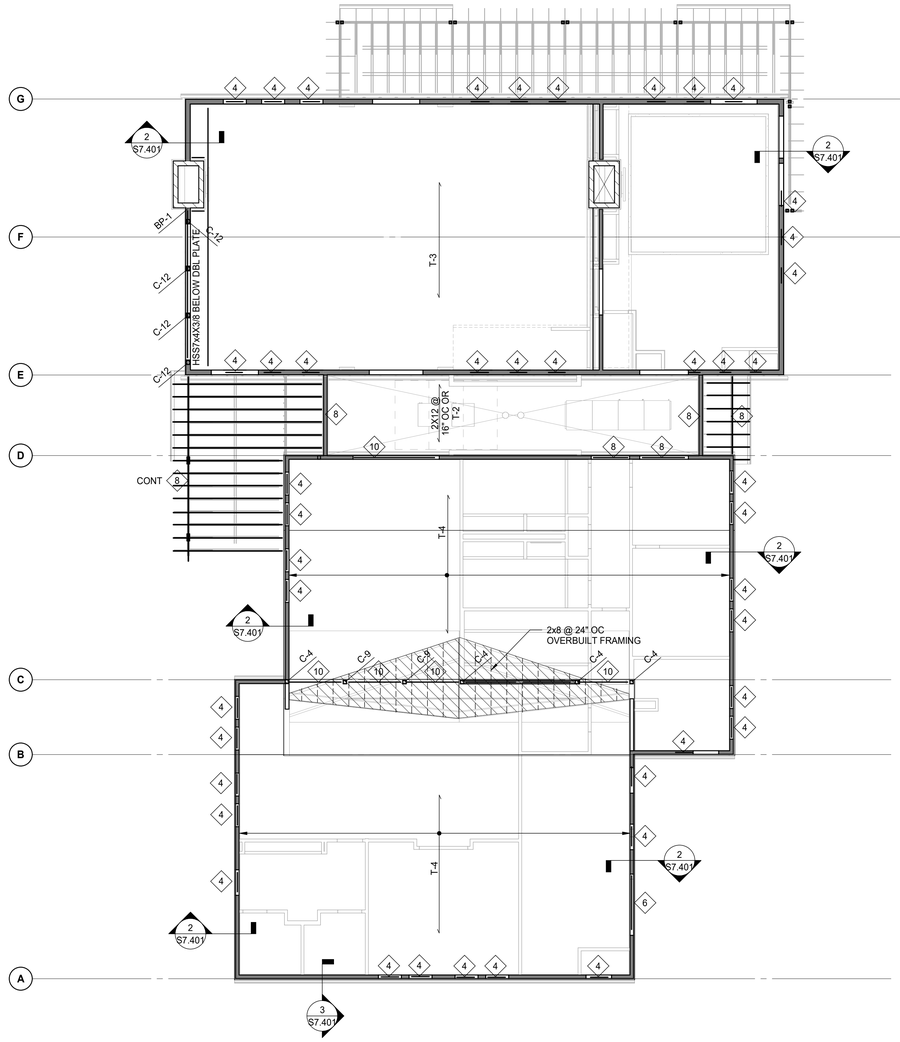
LEVEL 2 FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES:

1. TYPICAL FLOOR CONSTRUCTION SHALL BE AS FOLLOWS: UP TO 1" THICK GYPCRETE TOPPING OVER 23/32" (3/4") APA RATED STURDI-FLOOR TONGUE AND GROOVE PANELS GLUED AND SCREWED TO 18" TRUSSES OVER 2x WOOD BEARING WALLS. SEE THIS SHEET FOR SCHEDULE AND TRUSS LOADING INFORMATION.
2. LOW ROOF CONSTRUCTION SHALL BE AS FOLLOWS: 3/4" STRUCTURAL SHEATHING ON PRE-ENGINEERED WOOD ROOF TRUSSES @ 24" OC.
3. INTERIOR TOP OF SHEATHING = 11'-8".
3. LOW ROOF SHEATHING VARIES, SEE ARCH.
5. SEE S7.301 FOR TYPICAL HEADERS, STUDS AT OPENINGS, BEAM AND COLUMN SCHEDULES.
6. ▬ DENOTES BEARING WALL. WALL STUDS TO BE 2x6 @ 16" OC, UNO ON PLAN.

TRUSS/JOIST SHOP DRAWING SUBMITTAL SHALL BE COORDINATED WITH AND SHALL SHOW ALL BATHTUB, SHOWER AND TOILET DRAINS AND ALL MECHANICAL SHAFTS. ADJUST JOIST SPACING AND/OR ADD JOISTS AND HEADERS TO CLEAR PLUMBING & MECHANICAL.



ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES

1. ROOF CONSTRUCTION 3/4'' STRUCTURAL SHEATHING ON PRE-ENGINEERED ROOF WOOD TRUSSES @ 24'' O.C.
2. ROOF TOP OF SHEATHING VARIES, SEE ARCH.
3. TYPICAL SECTIONS SHOWN ARE APPLICABLE TO SIMILAR CONDITIONS EVEN IF MARKS ARE NOT SHOWN.
4. SEE S7.301 FOR TYPICAL HEADERS AND STUDS AT OPENINGS, BEAM AND COLUMN SCHEDULES.
5. ▬ DENOTES BEARING WALL. WALL STUDS TO BE 2x6 @ 16" OC, UNO ON PLAN.
6. BP-1 DENOTES 15x7x1/2 STEEL PLATE @/ (2)-1/2"Ø x 6" LONG HEADED STUDS INTO SOLID GROUTED CMU.

ALABAMA LICENSED No. 22545 PROFESSIONAL ENGINEER THOMAS A. BOUFFARD

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HUNTSVILLE, AL 35811

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S7.101

LEVEL 2 & ROOF FRAMING PLANS

12/11/20

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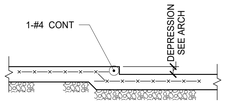
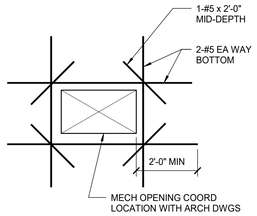


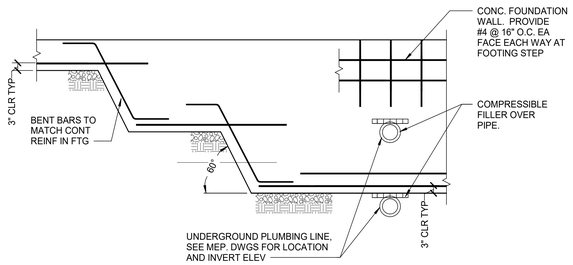
TYPICAL FLOOR DEPRESSION

SECTION 4 S7.201
SCALE: 1/2" = 1'-0"



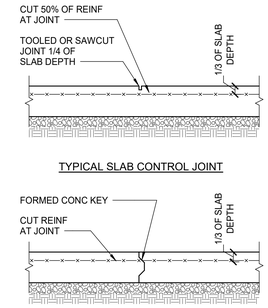
TYPICAL OPENING REINFORCEMENT @ SLAB ON GRADE BLOCK-OUTS

SECTION 3 S7.201
SCALE: 1/2" = 1'-0"



TYPICAL FOOTING STEP (F.S.)

SECTION 2 S7.201
SCALE: 1/2" = 1'-0"



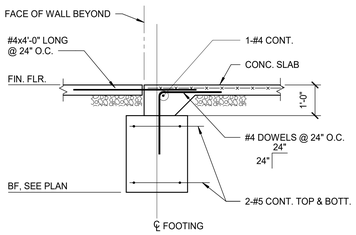
TYPICAL SLAB CONTROL JOINT

TYPICAL SLAB CONSTRUCTION JOINT

SECTION 1 S7.201
SCALE: 1/2" = 1'-0"

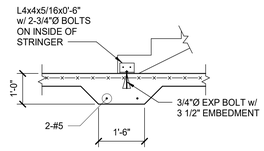
FOOTING SCHEDULE			
MARK	SIZE	BOTTOM REINFORCING	REMARKS
F1	3'-0"x3'-0"x12"	3-#4 BOTT. E.W.	

PIER SCHEDULE				
		REINFORCING		
MARK	SIZE	VERTICAL	TIES	REMARKS
P1	8x16/12x16	2-#4 VERT		CMU GROUT SOLID



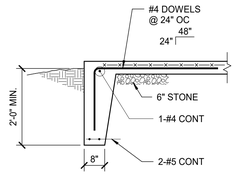
TYPICAL SECTION @ DOOR ENTRY

SECTION 9 S7.201
SCALE: 1/2" = 1'-0"



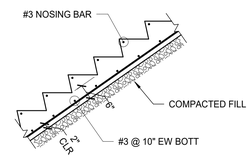
THICKENED SLAB @ STRINGER

SECTION 8 S7.201
SCALE: 1/2" = 1'-0"



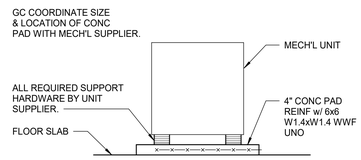
TURN DOWN SLAB EDGE

SECTION 7 S7.201
SCALE: 1/2" = 1'-0"



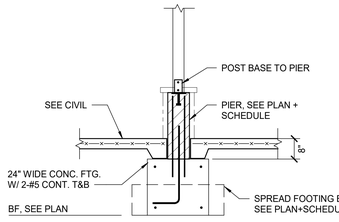
TYPICAL CONCRETE STAIRS

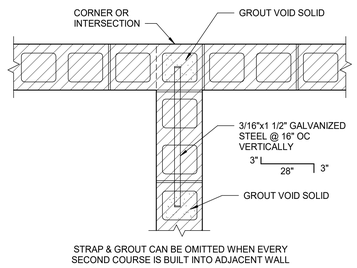
SECTION 6 S7.201
SCALE: 1/2" = 1'-0"



TYPICAL HOUSEKEEPING PAD

SECTION 5 S7.201
SCALE: 1/2" = 1'-0"



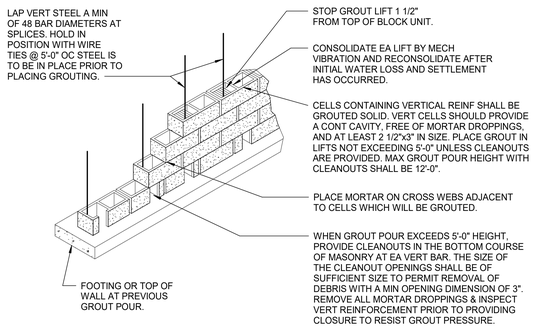


STRAP & GROUT CAN BE OMITTED WHEN EVERY SECOND COURSE IS BUILT INTO ADJACENT WALL

MASONRY CORNER OR INTERSECTION DETAIL

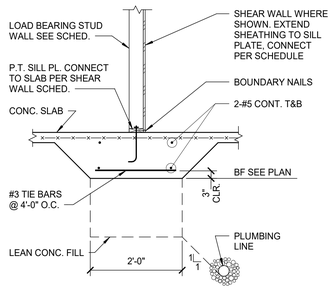
SECTION 14 S7.201
SCALE: 1/2" = 1'-0"

SECTION 13 S7.201
SCALE: 3/4" = 1'-0"

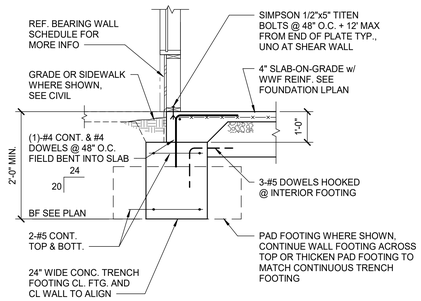


TYPICAL REINFORCED MASONRY CONSTRUCTION

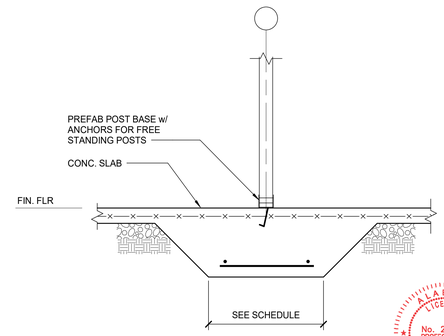
SECTION 12 S7.201
SCALE: 3/4" = 1'-0"



SECTION 11 S7.201
SCALE: 1/2" = 1'-0"



SECTION 10 S7.201
SCALE: 1/2" = 1'-0"



INTERIOR WOOD COLUMN FOOTING

SECTION 15 S7.201
SCALE: 3/4" = 1'-0"

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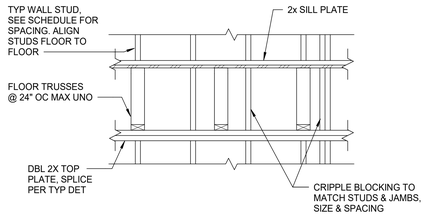
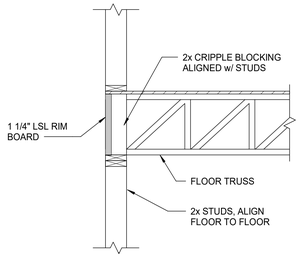


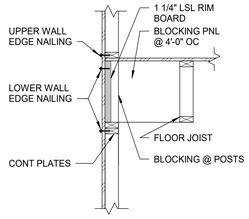
TYPICAL WALL CRIPPLE BLOCKING AT BEARING WALLS

SECTION 4 S7.301
SCALE: 3/4" = 1'-0"



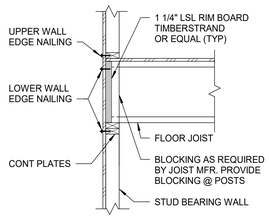
TYPICAL INTERIOR BEARING WALL

SECTION 3 S7.301
SCALE: 3/4" = 1'-0"



WALL PARALLEL TO JOIST

SECTION 2 S7.301
SCALE: 3/4" = 1'-0"



TYPICAL EXTERIOR BRG. WALL

SECTION 1 S7.301
SCALE: 3/4" = 1'-0"

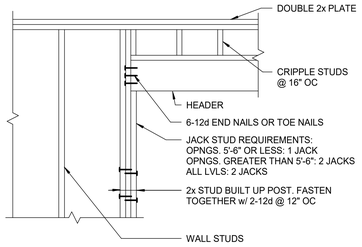
HEADER SCHEDULE

1	2-2x6
2	3-2x6
3	2-2x8
4	3-2x8
5	2-2x10
6	3-2x10
7	2-2x12
8	3-2x12
9	2-1 3/4"x9 1/2" LVL
10	2-1 3/4"x11 7/8" LVL
11	2-1 3/4"x14" LVL
12	2-1 3/4"x16" LVL
13	2-1 3/4"x18" LVL
14	3-1 3/4"x9 1/2" LVL
15	3-1 3/4"x11 7/8" LVL
16	3-1 3/4"x14" LVL
17	3-1 3/4"x16" LVL

COLUMN SCHEDULE

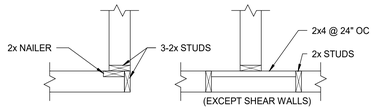
C-1	2-2x4
C-2	2-2x6
C-3	3-2x4
C-4	3-2x6
C-5	4-2x4
C-6	4-2x6
C-7	5-2x4
C-8	5-2x6
C-9	4x4 POST
C-10	4x6 POST
C-11	5 1/4x7 1.8E PSL POST
C-12	7 1/4x7 1/4 1.8E PSL POST

NOTE:
PROVIDE DOUBLE STUD @ END OF EACH HEADER UNLESS NOTED OTHERWISE



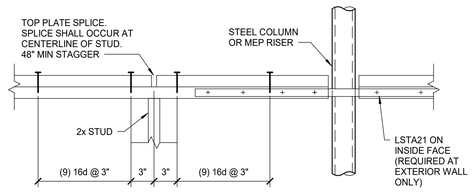
TYPICAL DROPPED HEADER AT OPENING

SECTION 7 S7.301
SCALE: 3/4" = 1'-0"



TYPICAL WALL CORNER AND INTERSECTION
(INTERIOR AND EXTERIOR - TYPE V)

SECTION 6 S7.301
SCALE: 3/4" = 1'-0"



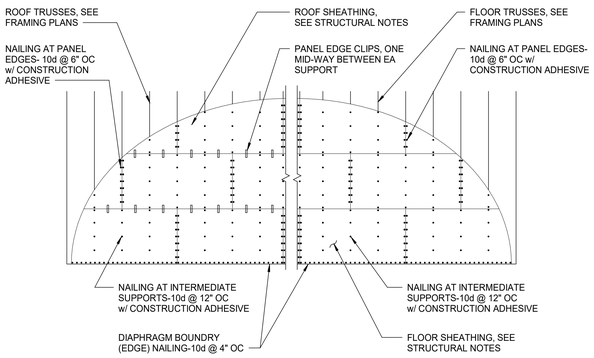
TYPICAL TOP PLATE SPLICE AT ALL WALLS AND PLATE INTERRUPTION STRAP AT EXTERIOR WALL

SECTION 5 S7.301
SCALE: 1 1/2" = 1'-0"

FLOOR JOIST/TRUSS SCHEDULE

MARK	SIZE	UNIFORM LOADS			DEFLECTION LIMITS			REMARKS
		DL PSF (TOP CHORD)	LL PSF (TOP CHORD)	DL PSF (BOT. CHORD)	LL	TL	△ MAX (INCHES)	
T-1	18" DEEP WOOD TRUSS @ 24" OC MAX	15	100	7	L/480	L/240	1"	PUBLIC AREA
T-2	12" DEEP WOOD TRUSS @ 24" OC MAX	18	60 PSF	7	L360	L/240	1"	FLAT ROOF MECH AREAS
T-3	METAL PLATED OPEN-WEB WOOD SCISSOR TRUSSES @ 24" OC	20	20 MIN.	10	L360	L/240	1"	
T-4	METAL PLATED OPEN-WEB WOOD TRUSSES @ 24" OC	20	20 MIN.	10	L360	L/240	1"	PITCHED ROOF AREAS

1. TRUSS SPACING SHALL BE DETERMINED BY TRUSS MANUF. 24" OC MAX TO MEET THE DESIGNATED LOADING AND DEFLECTION CRITERIA.
2. REFER TO FRAMING PLANS AND SECTIONS FOR CONCENTRATED OR OTHER ADDITIONAL LOADS TO TRUSSES.



ROOF DIAPHRAGM NAILING PATTERN

FLOOR DIAPHRAGM NAILING PATTERN (WALL NAILING PATTER SIMILIAR - SEE SHEAR WALL SCHEDULE)

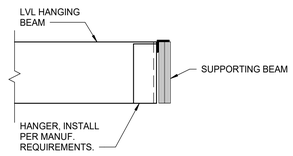
SECTION 12 S7.301
SCALE N.T.S.

HANGING BEAM SIZE	MAX. LOAD (LB)	REQUIRED * HANGER
2-12"	7,900	SIMPSON HGLT 3.511
2-14"	9,310	SIMPSON HGLT 3.514
2-16"	10,600	SIMPSON HGLT 3.516
2-18"	10,950	SIMPSON HGLT 3.518

* OR EQUAL

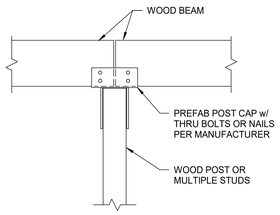
HANGING BEAM SIZE	MAX. LOAD (LB)	REQUIRED * HANGER
3-12"	11,850	SIMPSON HGLT 5.511
3-14"	12,750	SIMPSON HGLT 5.514
3-16"	12,750	SIMPSON HGLT 5.516
3-18"	12,750	SIMPSON HGLT 5.518

* OR EQUAL



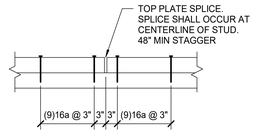
TYPICAL LVL BEAM TO BEAM CONNECTION

SECTION 11 S7.301
SCALE N.T.S.



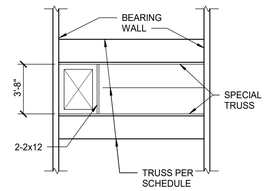
TYPICAL WOOD BEAM TO WOOD POST CONNECTION

SECTION 10 S7.301
SCALE N.T.S.



TYPICAL TOP PLATE SPLICE

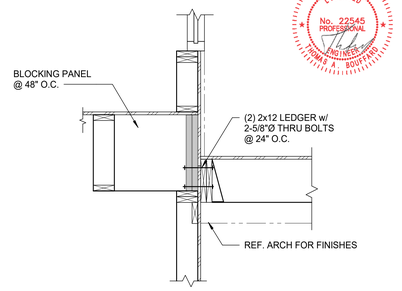
SECTION 9 S7.301
SCALE N.T.S.



NOTE: DETAIL APPLIES FOR SMALL OPENINGS UP TO 3'-8" WIDE

TYPICAL SHAFT FRAMING AT TRUSS FLOOR

SECTION 8 S7.301
SCALE N.T.S.



SECTION 13 S7.301
SCALE N.T.S.

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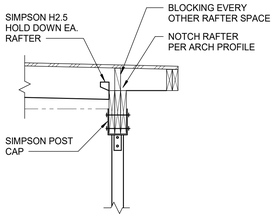
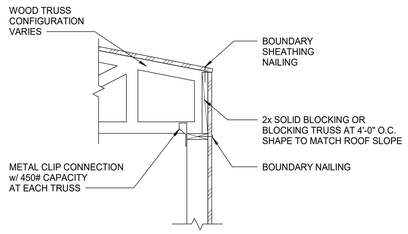
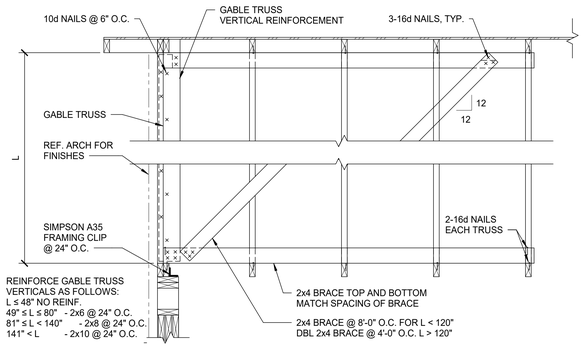


SECTION 4 S7.401
SCALE: 3/4" = 1'-0"

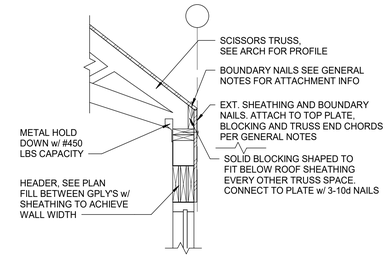


SECTION 3 S7.401
SCALE: 3/4" = 1'-0"

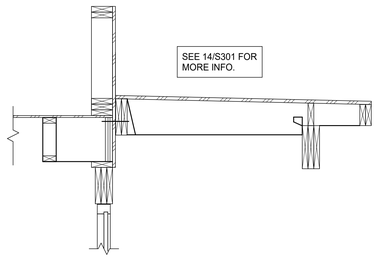


TYPICAL GABLE TRUSS REINFORCEMENT

SECTION 2 S7.401
SCALE: 3/4" = 1'-0"



SECTION 1 S7.401
SCALE: 3/4" = 1'-0"



SECTION 5 S7.401
SCALE: 3/4" = 1'-0"

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