# **Surface Water Management Report**

For

# **Inspiration at South Pointe**

**SEPTEMBER 17, 2021** 

Prepared for:

# ZIMMER DEVELOPMENT COMPANY 111 PRINCESS ST WILMINGTON, NC 28401

Prepared by:



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This item has been electronically signed and sealed by Ricardo Acosta, P.E. on September 17, 2021 using SHA-1 authentication code.

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## **Table of Contents**

	Page No.
Purpose	1
Existing Conditions	1
Proposed Conditions	2
Land Use Breakdown	3
Design Summary	4
Surface Water Management Calculations	5

Appendix - ICPR MODEL



### **PURPOSE**

The purpose of this application is to request an Environmental Resource Permit (ERP) for the Inspiration at South Pointe.

The vertical control datum for this project is North American Vertical Datum of 1988 (NAVD 88). Existing designs and permitting documents for areas surrounding this property referenced the National Geodetic Vertical Datum of 1929 (NGVD 29).

The conversion to NAVD 88 is: NAVD 88 = NGVD 29 - 1.18 ft.

### **EXISTING CONDITIONS**

The project site is in Fort Myers, Lee County, Florida, approximately 0.5 miles east of the intersection of McGregor Blvd and College Pkwy, lying in Section 15, Township 45 South, Range 24 East.

The existing site is an agricultural parcel without permitted stormwater management facilities. The site is generally flat, with ground elevations ranging from 6 feet to 8 feet. Stormwater runoff sheetflows overland to the south, into the College Parkway Right-of-Way roadside ditch formally known as IDD Canal H-8. The roadside swale was previously permitted in ERP Nos. 87 00087-S and 36-02810-S to serve as both a detention area for College Parkway and a conveyance canal for upstream tributary areas, including the subject site. The calculations submitted in support of ERP App. No. 980424-17 demonstrated the downstream conveyance was designed for a peak flowrate of approximately 190 cfs (0.88 cfs/ac) for the 25-year, 3-day storm event, more than sufficient to convey the runoff from the proposed project.

To the north and west of the site lies the College Pointe development, permitted by ERP No. 36 03472-P to have a control elevation of 2.82 feet NAVD 88. The downstream control structure in the roadside swale College Parkway (IDD Canal H-8) has a permitted control elevation of 1.32 feet NAVD 88 (2.5 feet NGVD 29) and an as-built control elevation of 1.18 feet NAVD 88 (2.36 feet NGVD 29). Northeast of the site there is a restaurant with a standalone surface water management system with an apparent permitted control elevation of 4.82 feet NAVD 88 (ERP App. No. X000000464), based on WSWT elev of 6.0 feet, NGVD 29 and bottom of swales at elevation 7.0 feet, NGVD. There was no overflow structure in the permit plans. however, there is an inlet presumably at elevation 8.0 feet, NGVD 29 (6.8 feet NAVD 88).

The site is located within the Whiskey Creek watershed, as identified by the Florida Department of Environmental Protection (FDEP) Water Body Identification Number (WBID) 3240H. This watershed is not included in the comprehensive verified list of impaired water bodies.



The site is in the Whiskey Creek sub-watershed north of College Parkway, which was assigned a recommended unit runoff rate of 108 CSM in the 1991 Lee County Surface Water Management Plan by Johnson Engineering, Inc.

The site is currently in a FEMA Zone X as identified in the FEMA FIRM No. 12071C0417F with effective date of August 28, 2008. The preliminary FEMA FIRM dated June 28th, 2019, proposes to revise the flood zone at the site to Zone AE, with an elevation of 8 feet NAVD 88.

### PROPOSED CONDITION

The proposed project consists of construction of a mixed-use development that will include apartment buildings, an amenity area, outparcels, parking areas, and associated infrastructure. The proposed surface water management system will consist of a combination of wet pond and underground chambers.

Based on the surrounding control elevations, the control elevation for the proposed stormwater management system was set to elevation 4.5 feet NAVD 88.

There will be two outparcels, totaling 2.71 acres, that will not be developed as part of this application. Only minor grading and construction of perimeter berm will occur at present time. New applications will need to be submitted at the time of their future development and will have to address construction of pre-treatment areas, pre-treatment control structures and necessary revisions to the orifices in the outfall structure from the pond.

The ERP 36-02810-S will be modified to accommodate the work within the College Pkwy right-of-way (ROW) consisting of construction of a turn lane. A portion of the existing linear pond within the ROW will be replaced with pipe. Compensatory treatment for the turn lane and the loss of treatment volume within the ROW will be provided in the onsite surface water management system.

The following calculations demonstrate the proposed system provides the required treatment and attenuation. 50% additional treatment is provided.



## Land Use Breakdown

Building
Impervious/Amenity
Pond
Dry Detention
Pervious
Total

	Parcel	College	Total	
Apartments	Road	Outparcels	Pkwy	Total
2.40	0.00	0.00	0.00	2.40
4.33	0.77	0.00	0.21	5.31
0.53	0.00	0.00	0.00	0.53
0.00	0.00	0.00	0.00	0.00
2.55	0.06	2.71	0.16	5.48
9.81	0.83	2.71	0.37	13.72

## **Parcel Areas**

	Routed to	Not Routed	Total
_	Pond	to Pond	Total
Building	2.40	0.00	2.40
Impervious/Amenity	4.71	0.39	5.10
Pond	0.53	0.00	0.53
Dry Detention	0.00	0.00	0.00
Pervious	5.32	0.00	5.32
Total	12.96	0.39	13.35

## **College Parkway ROW Change in Areas**

_	Existing	Proposed	Change
Building	0.00	0.00	0.00
Impervious	0.08	0.21	0.13
Pond	0.08	0.00	-0.08
Dry Detention	0.00	0.00	0.00
Pervious	0.21	0.16	-0.05
Total	0.37	0.37	0.00



•

# Design Summary

Water Quality	
Control Elevation (ft)	4.5
Water Quality Volume required (ac-ft)	2.03
Volume of Underground Storage (ac-ft)	1.14
Water Quality Elevation (ft)	6.6
5-Year, 1-Day Storm	
Rainfall (in)	5.5
Peak Stage (ft)	8.7
Minimum Parking Elev.(ft)	9.0
25-Year, 3-Day Storm	
Rainfall (in)	11.5
Allowable Basin Discharge Rate * (CSM)	108
Allowable Discharge (cfs)	2.19
Design Discharge (cfs)	2.18
Peak Stage (ft)	10.0
Minimum Berm Elevation (ft)	10.3
100-Year, 3-Day Storm (Zero Discharge)	
Rainfall (in)	15.0
FEMA Zone X (ft)	N/A
Peak Stage (ft)	10.7
Minimum Building Finished Floor (ft)	10.8

Elevations Reference NAVD 88



4

<sup>\*</sup> Whiskey Creek watershed

## **Surface Water Management Calculations**

Acreage Breakdown				College Parkway
	Routed	Not Routed	Total for WQ	WQ Compensation
Building	2.40	0.00	2.40	
Impervious/Amenity	4.71	0.39	5.10	0.13
Pond	0.53	0.00	0.53	-0.08
Dry Detention	0.00	0.00	0.00	
Pervious	5.32	0.00	5.32	
Total	12.96	0.39	13.35	

## **Design Parameters:**

Control Elevation (ft, NAVD) =	4.50 ft
Min Road Elevation (ft, NAVD) =	9.00 ft
Min Berm Elevation (ft, NAVD) =	10.30 ft
Min FFE Elevation (ft, NAVD) =	10.80 ft

## **Water Quality Calculations:**

## First One Inch of Runoff from the Basin

Total Site Area x 1 inch = 1.11 ac-ft

## 2.5 Inches times the Percent Impervious

Site Area for Water Quality (SAWQ) Total Site - (Lake + Roof) = 10.4 ac

Imperv Area for Water Quality (IAWQ)
Impervious/Amenity = 5.1 ac

Percent Imperv for Water Quality Perv/Imperv Calculations (%IMP) (IAWQ / SAWQ) x 100% = 49%

2.5 Inches x Percent Imperv
2.5 Inches x %IMP = 1.2 in

Volume Required to be Treated (ac-ft)
(2.5 Inches x %IMP) x (Total Site - Lake) x 1 ft./12 in. = 1.31 ac-ft



## **Required Wet Detention:**

Compensatory Treatment for Offsite Areas:

Increased Impervious = 
$$0.13 \text{ ac}$$

Decreased Pond =  $0.08 \text{ ac}$ 

Total =  $0.21 \text{ ac}$ 

Wet Pond Storage = Treatment requirement 
$$+ 50\% = 2.03 \text{ ac-ft}$$

Note: Whiskey Creek watershed (WBID 3240H) is listed as impaired due to Dissolved Oxygen, and a causative pollutant has not been identified.

This volume will be provided in pond and underground storage. See Water Quality Volume table.

## **Soil Storage:**

Soil storage for the pervious areas has been calculated using the compacted water storage capability estimate by the Soil Conservation Service for the sandy material in South Florida Water Management District.

Average Site Grade =		7.00 ft
Depth to Water Table (Avg site grad - WSWT E	Elv.) =	2.5 ft
Soil Storage in pervious areas =		3.42 in
Curve Number (CN) =	$\frac{1000}{S+10}$ =	75



Time of concentration:

The following time of concentration calculations have been performed using the method outlined in the NRCS Urban Hydrology for Small Watersheds (TR-55).

## Sheet Flow:

Manning's Roughness Coef "n" =	0.41
Flow Length "L" =	100 ft
2 Year 24 Hour Rainfall "P" =	5.0 in
Hydraulic Grade Line Slope "s" =	0.01 ft/ft
Time = $0.007*(n*L)^0.8/[(P)^0.5*s^0.4]*60 =$	23.1 min

## System Bleed-down

The following calculations indicate the discharge below water treatment elevation exceeds 0.5 inches in 24 hours. However, all bleed-down discharge occurs through a minimally-sized bleeder.

0	5	inch	bleed	down	vo	lume

0.5 men bieed down volume		
0.5 in. x (total site for WQ - lake)	0.53 ac-ft	
Treatment Elevation =	6.58 ft	
Total Vol Out at time of Treatment Elevation	10.57 ac-ft	
Time of treatment elevation	120 hr	
Time of treatment elevation + 24 hours	144 hr	
Total Vol Out at treatment elevation + 24 hours	11.16 ac-ft	
Volume discharged in 24 hours	0.59 ac-ft	* 3" bleeder



Stage Storage Table

Above Ground					
	Pond	Dry Det	Pervious	Impervious	
Start Elev>	4.5 ft	7.0 ft	8.7 ft	9.0 ft	
StartArea>	0.53 ac	0.00 ac	0.00 ac	0.00 ac	
End Elev>	10.3 ft	8.5 ft	10.7 ft	10.6 ft	
End Area>	0.58 ac	0.00 ac	5.27 ac	4.71 ac	Total
Stage	Area	Area	Area	Area	Area
(ft, NAVD)	(ac)	(ac)	(ac)	(ac)	(ac)
4.50	0.53	0.00	0.00	0.00	0.53
6.50	0.55	0.00	0.00	0.00	0.55
7.00	0.55	0.00	0.00	0.00	0.55
8.60	0.57	0.00	0.00	0.00	0.57
9.00	0.57	0.00	0.79	0.00	1.36
10.30	0.58	0.00	4.22	3.83	8.62
10.60	0.58	0.00	5.01	4.71	10.30

0.00

0.00

5.27

5.27

Underground Chambers					
Stage	Total Volume				
(ft, NAVD)	(ac-ft)				
4.50	0				
5.00	0.16				
5.25	0.32				
5.50	0.48				
6.00	0.75				
6.58	0.96				
7.17	1.14				
7.18	1.14				

\* Chamber System Layers:

0.58

0.58

10.70

10.80

Layer	Thickness	Bottom Elev	Top Elev.
Gravel Below Chambers:	6 in	4.50 ft	5.00 ft
Chambers:	16 in	5.00 ft	6.33 ft
Gravel Above Chambers:	<u>10 in</u>	6.33 ft	7.17 ft

10.56

10.56

Total: 32 in

## **Water Quality Volume**

4.71

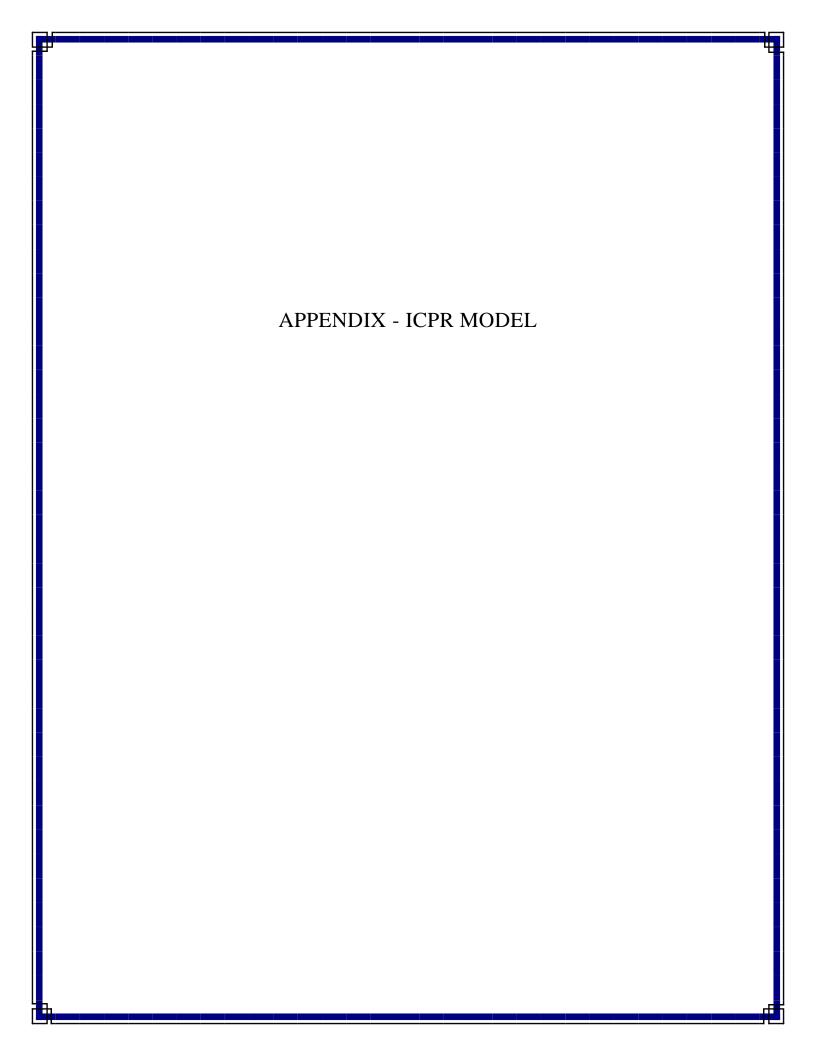
4.71

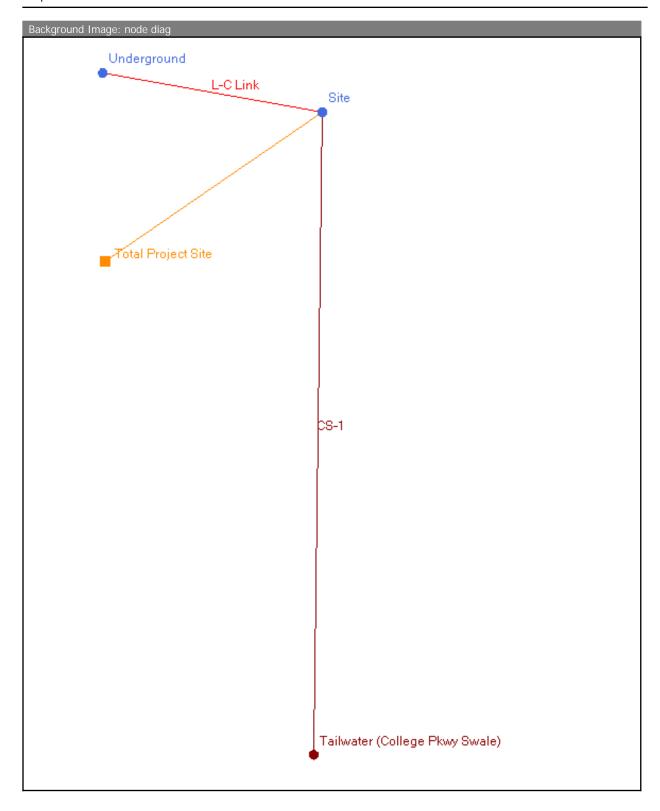
Start Elev>		4.5 ft			
StartArea>		0.53 ac		Chambers	
End Elev>		10.3 ft			
End Area>		0.60 ac			
Stage	Area	Volume	Cum. Volume	Cum. Volume	
(ft, NAVD)	(ac)	(ac-ft)	(ac-ft)	(ac-ft)	
4.50	0.53	0.53 0.00 0.00			
6.58	0.55 1.12 1.12			0.96	
7.17	0.55	0.32	1.44	1.14	
9.00	0.57	1.03	2.47	1.14	

Total Volume
(ac-ft)
0
2.08
2.59
2.47

<-- W.Q. Elev







## Manual Basin: Total Project Site

Scenario: Scenario1

Node: Site

Hydrograph Method: NRCS Unit Hydrograph

Infiltration Method: Curve Number
Time of Concentration: 23.0000 min
Max Allowable Q: 0.00 cfs
Time Shift: 0.0000 hr

Unit Hydrograph: UH256 Peaking Factor: 256.0

Area:	12.9600 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name	Crop Coefficient Zone	Reference ET Station
2.4000	Building	Α			
4.7100	Impervious	Α			
0.5300	Lake	Α			
0.0000	Dry Detention	Α			
5.3200	Pervious	Α			

### Comment:

### Manual Basin Runoff Summary [Scenario1]

Basin	Sim Name	Max Flow	Time to	Total	Total	Area [ac]	Equivalent	% Imperv	% DCIA
Name		[cfs]	Max Flow	Rainfall	Runoff [in]		Curve		
			[hrs]	[in]			Number		
Total	005Y-24H	31.17	12.1667	5.50	4.27	12.9600	89.2	0.00	0.00
Project									
Site									
Total	025Y-72H	45.06	60.1333	11.50	10.03	12.9600	88.2	0.00	0.00
Project									
Site									
Total	100Y-72H	59.75	60.1333	15.00	13.47	12.9600	88.0	0.00	0.00
Project									
Site									

## Curve Number: 1 [Set]

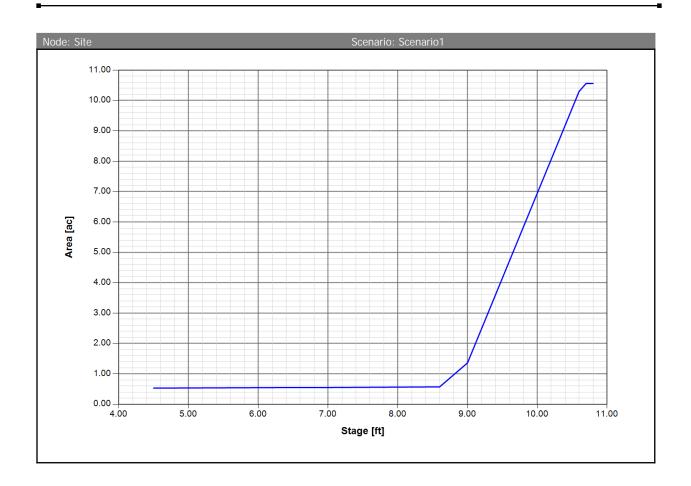
Land Cover Zone	Soil Zone	Curve Number [dec]
Building	А	98.0
Dry Detention	A	98.0
Impervious	A	98.0
Lake	A	98.0
Pervious	А	75.0

## Node: Site

Scenario: Scenario1 Type: Stage/Area Base Flow: 0.00 cfs Initial Stage: 4.50 ft Warning Stage: 10.80 ft

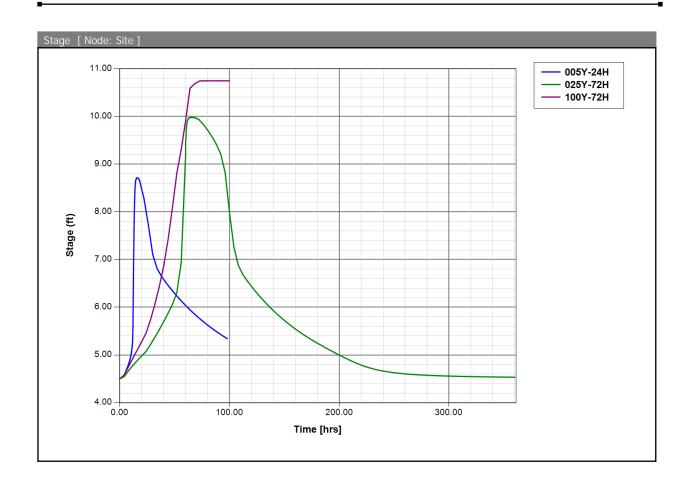
Stage [ft]	Area [ac]	Area [ft2]
4.50	0.5300	23087
6.50	0.5500	23958
7.00	0.5500	23958
8.60	0.5700	24829
9.00	1.3600	59242
10.30	8.6200	375487
10.60	10.3000	448668
10.70	10.5600	459994
10.80	10.5600	459994

Comment:



### Node Max Conditions [Scenario1]

Node Name	Sim Name	Warning Stage [ft]	Max Stage [ft]	Min/Max Delta Stage [ft]	Max Total Inflow [cfs]	Max Total Outflow [cfs]	Max Surface Area [ft2]
Site	005Y-24H	10.80	8.71	0.0010	31.17	14.59	34478
Site	025Y-72H	10.80	9.98	0.0010	45.06	2.16	297606
Site	100Y-72H	10.80	10.74	0.0010	59.75	0.49	459994

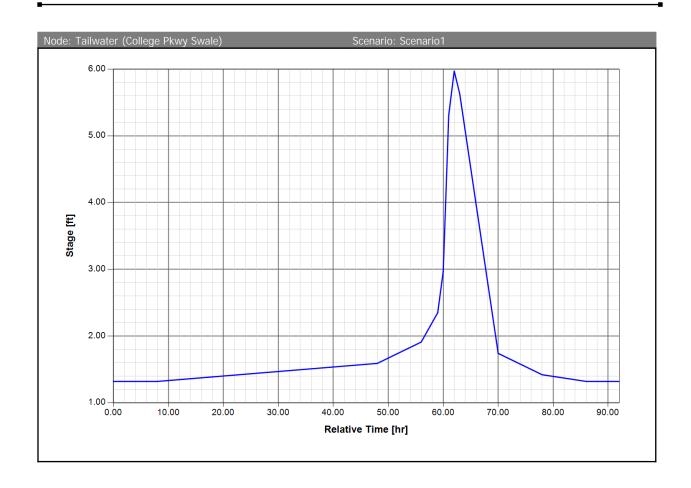


## Node: Tailwater (College Pkwy Swale)

Scenario: Scenario1
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 1.32 ft
Warning Stage: 6.00 ft
Boundary Stage: College Pkwy

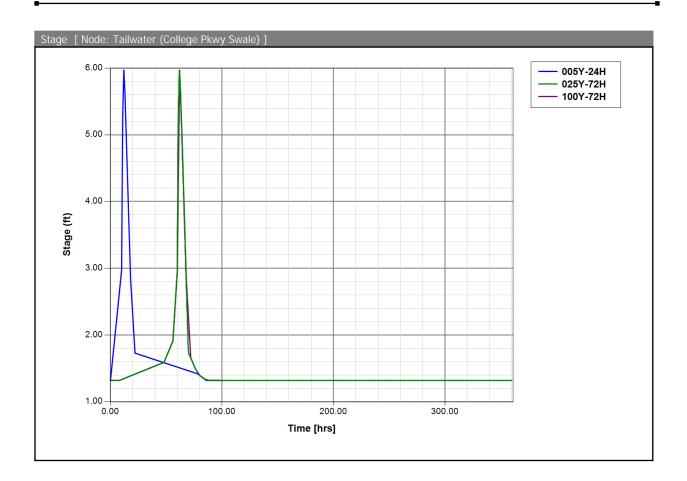
Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	1.32
0	0	0	8.0000	1.32
0	0	0	48.0000	1.59
0	0	0	56.0000	1.91
0	0	0	59.0000	2.35
0	0	0	60.0000	2.97
0	0	0	61.0000	5.31
0	0	0	62.0000	5.97
0	0	0	63.0000	5.63
0	0	0	70.0000	1.74
0	0	0	78.0000	1.42
0	0	0	86.0000	1.32
0	0	0	92.0000	1.32

Comment:



Node Max Conditions [Scenario1]

Node Name	Sim Name	Warning Stage [ft]	Max Stage [ft]	Min/Max Delta Stage [ft]	Max Total Inflow [cfs]	Max Total Outflow [cfs]	Max Surface Area [ft2]
Tailwater (College Pkwy Swale)	005Y-24H	6.00	5.97	0.0195	1.73	0.16	0
Tailwater (College Pkwy Swale)	025Y-72H	6.00	5.97	0.0052	2.16	0.00	0
Tailwater (College Pkwy Swale)	100Y-72H	6.00	5.97	0.0064	0.00	0.00	0



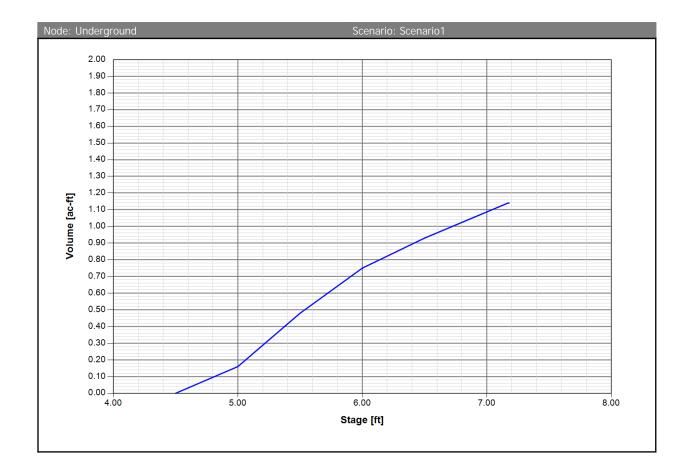
Node: Underground

Scenario: Scenario1 Type: Stage/Volume

Base Flow: 0.00 cfs Initial Stage: 4.50 ft Warning Stage: 10.80 ft

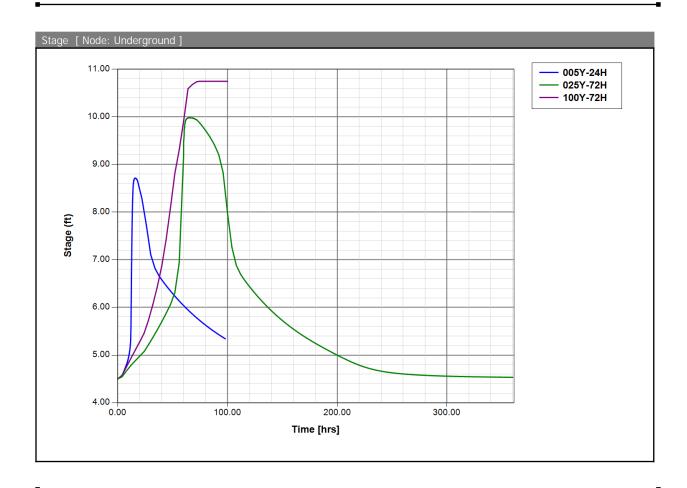
Stage [ft]	Volume [ac-ft]	Volume [ft3]
4.50	0.00	0
5.00	0.16	6970
5.25	0.32	13939
5.50	0.48	20909
6.00	0.75	32670
6.50	0.93	40511
7.17	1.14	49658
7.18	1.14	49658

Comment:



Node Max Conditions [Scenario1]

Node Name	Sim Name	Warning Stage [ft]	Max Stage [ft]	Min/Max Delta Stage [ft]	Max Total Inflow [cfs]	Max Total Outflow [cfs]	Max Surface Area [ft2]
Underground	005Y-24H	10.80	8.71	0.0010	14.59	0.21	27879
Underground	025Y-72H	10.80	9.98	-0.0010	0.67	0.21	27879
Underground	100Y-72H	10.80	10.74	0.0010	0.49	0.04	27878



Drop Structure Link:	CS-1	Upstrea	ım Pipe	Downstr	eam Pipe
Scenario:	Scenario1	Invert:	0.00 ft	Invert:	0.00 ft
From Node:	Site	Manning's N:	0.0130	Manning's N:	0.0130
To Node:	Tailwater (College	Geometry	: Circular	Geometr	y: Circular
	Pkwy Swale)	Max Depth:	1.50 ft	Max Depth:	1.50 ft
Link Count:	1			Bottom Clip	
Flow Direction:	Both	Default:	0.00 ft	Default:	0.00 ft
Solution:	Combine	Op Table:		Op Table:	
Increments:	0	Ref Node:		Ref Node:	
Pipe Count:	1	Manning's N:	0.0000	Manning's N:	0.0000

9

Damping: 0.0000 ft

Length: 640.00 ft

Default: 0.00 ft

Manning's N: 0.0000

Op Table:

Default: 0.00 ft

FHWA Code: 0 Entr Loss Coef: 1.50

Op Table: Ref Node:

Ref Node:

Manning's N: 0.0000

Exit Loss Coef: 3.00 Bend Loss Coef: 0.00

Bend Location: 0.00 dec

Energy Switch: Energy

Pipe Comment:

Weir Component

Weir: Weir Count: 1

Weir Flow Direction: Both

Damping: 0.0000 ft

Weir Type: Sharp Crested Vertical

Geometry Type: Circular Invert: 4.50 ft

Control Elevation: 4.50 ft

Max Depth: 0.25 ft

Bottom Clip

Default: 0.00 ft

Op Table: Ref Node:

Top Clip

Default: 0.00 ft

Op Table: Ref Node:

Discharge Coefficients

Weir Default: 3.200 Weir Table: Orifice Default: 0.600

Orifice Table:

Weir Comment:

Weir: Weir Count: Weir Flow Direction: Both

Damping: 0.0000 ft

Weir Type: Sharp Crested Vertical

Geometry Type: Rectangular

Invert: 6.60 ft Control Elevation: 6.60 ft

> Max Depth: 0.38 ft Max Width: 0.50 ft

> > Fillet: 0.00 ft

Default: 0.00 ft

Op Table: Ref Node:

Top Clip

Default: 0.00 ft

Op Table: Ref Node:

Weir Default: 3.200 Weir Table:

Orifice Default: 0.600

Orifice Table:

Weir Comment:

Drop Structure Comment:

Link Min/Max Conditions [Scenario1]

Link Name Min Flow [cfs] Max Us Max Ds Max Avg Velocity [fps] Velocity [fps] Velocity [fps]

Link Name	Sim Name	Max Flow [cfs]	Min Flow [cfs]	Min/Max Delta Flow [cfs]	Max Us Velocity [fps]	Max Ds Velocity [fps]	Max Avg Velocity [fps]
CS-1 - Pipe	025Y-72H	2.16	0.00	0.00	0.00	0.00	0.00
CS-1 - Weir:	025Y-72H	0.55	0.00	0.00	0.00	0.00	0.00
CS-1 - Weir: 2	025Y-72H	1.61	0.00	0.00	8.60	8.60	8.60

Simulation: 005Y-24H

Min Calculation Time:

Scenario: Scenario1

Run Date/Time: 9/17/2021 3:45:36 PM Program Version: ICPR4 4.07.08

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	100.0000

 Hydrology [sec]
 Surface Hydraulics
 Groundwater [sec]

 [sec]
 60.0000
 0.1000
 900.0000

Max Calculation Time: 30.0000

### **Output Time Increments**

## Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	240.0000

## Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	240.0000
0	0	0	8.0000	15.0000
0	0	0	18.0000	240.0000

### Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

#### Resources & Lookup Tables

Resources

Rainfall Folder: Reference ET Folder: Unit Hydrograph Folder: Lookup Table:

Boundary Stage Set: 5 Extern Hydrograph Set: Curve Number Set: 1

Green-Ampt Set:
Vertical Layers Set:
Impervious Set: 1
Roughness Set:
Crop Coef Set:
Fillable Porosity Set:
Conductivity Set:
Leakage Set:

### Tolerances & Options

Time Marching: SAOR IA Recovery Time: 24.0000 hr
Max Iterations: 6 ET for Manual Basins: False

Over-Relax Weight 0.5 dec

Fact:

dZ Tolerance: 0.0010 ft Smp/Man Basin Rain Global

Opt:

Max dZ: 1.0000 ft OF Region Rain Opt: Global
Link Optimizer Tol: 0.0001 ft Rainfall Name: ~SCSII-24

Rainfall Amount: 5.50 in

(1D):

Edge Length Option: Automatic Storm Duration: 24.0000 hr

Dflt Damping (2D): 0.0050 ft
Min Node Srf Area 100 ft2

Dflt Damping (1D): 0.0050 ft
Min Node Srf Area 100 ft2

(2D):

Energy Switch (2D): Energy Energy Switch (1D): Energy

Comment:

Simulation: 025Y-72F

Scenario: Scenario1

Run Date/Time: 9/17/2021 3:45:44 PM Program Version: ICPR4 4.07.08

Genera

Run Mode: Normal

 Year
 Month
 Day
 Hour [hr]

 Start Time:
 0
 0
 0
 0.0000

End Time: 0 0 0 360.0000

 Hydrology [sec]
 Surface Hydraulics
 Groundwater [sec]

 [sec]
 60.0000
 0.1000
 900.0000

 Min Calculation Time:
 60.0000
 0.1000
 900.000

 Max Calculation Time:
 30.0000

#### Output Time Increments

#### Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	240.0000

## Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	240.0000
0	0	0	60.0000	15.0000
0	0	0	72.0000	240.0000

#### Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	480.0000

### Restart File

Save Restart: False

### Resources & Lookup Table

### Resources

Reference ET Folder: Unit Hydrograph Folder:

Rainfall Folder:

### Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: 1

Green-Ampt Set:
Vertical Layers Set:
Impervious Set:
Roughness Set:
Crop Coef Set:
Fillable Porosity Set:
Conductivity Set:
Leakage Set:

## Tolerances & Options

Time Marching: SAOR IA Recovery Time: 24.0000 hr
Max Iterations: 6 ET for Manual Basins: False

Over-Relax Weight 0.5 dec

Fact:

dZ Tolerance: 0.0010 ft Smp/Man Basin Rain Global

Opt:

Max dZ: 1.0000 ft OF Region Rain Opt: Global

Link Optimizer Tol: 0.0001 ft Rainfall Name: ~SFWMD-72

Rainfall Amount: 11.50 in

Edge Length Option: Automatic Storm Duration: 72.0000 hr

Dflt Damping (2D): 0.0050 ft
Min Node Srf Area 100 ft2

Dflt Damping (1D): 0.0050 ft
Min Node Srf Area 100 ft2

(2D): (1D):

Energy Switch (2D): Energy Energy Switch (1D): Energy

Comment:

Simulation: 100Y-72H

Min Calculation Time:

Scenario: Scenario1

Run Date/Time: 9/17/2021 3:45:12 PM Program Version: ICPR4 4.07.08

Genera

Run Mode: Normal

_	year	Month	Day	Hour [nr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	100.0000

 Hydrology [sec]
 Surface Hydraulics
 Groundwater [sec]

 [sec]
 60.0000
 0.1000
 900.0000

Max Calculation Time: 30.0000

### Output Time Increments

### Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	240.0000

### Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	240.0000

### Groundwater

Year	Month	Dav	Hour [hr]	Time Increment [min]

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	480.0000

#### Restart File

Save Restart: False

### Resources & Lookup Tables

Resources

Reference ET Folder: Unit Hydrograph Folder:

Rainfall Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: 1

Green-Ampt Set:
Vertical Layers Set:
Impervious Set: 1
Roughness Set:
Crop Coef Set:
Fillable Porosity Set:
Conductivity Set:
Leakage Set:

#### **Tolerances & Options**

Time Marching: SAOR IA Recovery Time: 24.0000 hr
Max Iterations: 6 ET for Manual Basins: False

Over-Relax Weight 0.5 dec

Fact:

dZ Tolerance: 0.0010 ft Smp/Man Basin Rain Global

Opt:

Energy Switch (1D): Energy

Max dZ: 1.0000 ft OF Region Rain Opt: Global
Link Optimizer Tol: 0.0001 ft Rainfall Name: ~SFWMD-72
Rainfall Amount: 15.00 in

Edge Length Option: Automatic Storm Duration: 72.0000 hr

Dflt Damping (2D): 0.0050 ft
Min Node Srf Area 100 ft2

Dflt Damping (1D): 0.0050 ft
Min Node Srf Area 100 ft2

(1D):

Energy Switch (2D): Energy

(2D):

Comment:

### Boundary Stage: College Pkwy

Boundary Stage Set: 5-yr

Year	Month	Day	Hour [hr]	Stage [ft]
0	0	0	0.0000	1.32

Year	Month	Day	Hour [	hr]	Stage [ft]
0	0	0		10.0000	2.97
0	0	0		11.0000	5.31
0	0	0		12.0000	5.97
0	0	0		13.0000	5.63
0	0	0		20.0000	1.74
0	0	0		78.0000	1.42
0	0	0		86.0000	1.32
0	0	0		92.0000	1.32

Comment: Same peak stage as 25-yr, 3-day storm but adjusted to peak at time = 12 hr