GENERAL NOTES

1. IT IS THE CONTRACTORS SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF WHATEVER TEMPORARY BRACING, GUYS OR TIE-DOWNS MAY BE NECESSARY

CODES AND SPECIFICATIONS

A GENERAL BUILDING CODE

I. DESIGN WIND LOAD SHALL BE BASED ON THE FLORIDA BUILDING CODE 2017 (6TH EDITION).

20 PSF

B. CONCRETE

1. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318-89) 2. SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301-84).

DESIGN LOADS

ROOF

A. DESIGN LIVE LOADS

	20 01
ELEVATED FLOORS	40 PSF
BALCONY	100 PSF
CORRIDORS/LANDINGS	100 PSF
STAIRS	100 PSF
B. WIND LOADS:	

1) BASIC WIND SPEED = 156 MPH (ULTIMATE)

121 MPH (NOMINAL)

2) RISK CATAGORY II

3) WIND EXPOSURE B.

4) INTERNAL PRESSURE COEFFICIENT: 0,18 FULLY ENCLOSED STRUCTURE 5) COMPONENTS AND CLADDING (ASD): +24.11 PSF AND -30.44 PSF FOR DESIGN WIND PRESSURES.

FOUNDATIONS

- 1. FOUNDATION DESIGN IS BASED ON AN ALLOWABLE SOIL BEARING PRESSURE OF 2500 PSF. WHICH SHALL BE VERIFIED BY A REGISTERED GEOTECHNICAL ENGINEER PRIOR TO THE START OF WORK.
- 2. PLACE FOOTINGS/SLAB ON COMPACTED SOIL. FOLLOW RECOMMENDATIONS OF SOILS REPORT.
- 3. VERIFY BEARING CAPACITY BY TESTING PRIOR TO SLAB PLACEMENT. SUBMIT REPORTS TO ARCHITECT

CAST-IN-PLACE CONCRETE

- 1. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS, HAVE A SLUMP OF 4" PLUS OR MINUS I", AND HAVE 2-4% AIR ENTRAINMENT
- 2. ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A-615 GRADE 60.
- 3. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH "THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" ACI 318-99.
- 4. ALL REINFORCING DETAILS SHALL CONFORM TO "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" ACI 315-80.
- 5. CONTRACTOR SHALL VERIFY LOCATIONS OF ALL OPENINGS, SLEEVES, ANCHOR BOLTS, INSERTS, ETC., AS REQUIRED BY OTHER TRADES BEFORE CONCRETE IS PLACED.
- 6. CONTRACTOR SHALL PROVIDE SPACERS, CHAIRS, BOLSTERS, ETC., NECESSARY TO SUPPORT REINFORCING STEEL.
- 1. ALL SLABS SHALL BE POURED MONOLITHICALLY, EXCEPT FOR REQUIRED CONSTRUCTION JOINTS.
- 8. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:
- 3"-----CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH. 1-1/2"-----ALL OTHER CASES.
- 9. HORIZONTAL SLAB BARS SHALL BE BENT 1'-6" AROUND CORNERS, OR PROVIDE CORNER BARS WITH A 2'-O" LAP ON EACH LEG.
- 10. TESTING LABORATORY SHALL SUBMIT ONE COPY OF ALL CONCRETE TEST REPORTS DIRECTLY TO THE POST TENSION ENGINEER ARCHITECT-SEE SPECS.

WOOD CONSTRUCTION

- I. WOOD CONSTRUCTION SHALL CONFORM TO THE NDS "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION", LATEST EDITION.
- 2. ALL WALL STUDS SHALL BE NO. 2 SOUTHERN YELLOW PINE FINGER JOINT OR SOLID WITH 9' PRE-CUT STUDS TO BE USED.

STUDS FOR ALL STUD WALLS SHALL BE AS FOLLOWS:

TWO STORY WALLS FIRST FLOOR SECOND FLOOR

THREE STORY WALLS

FIRST FLOOR *3×4'S @ 16"Ø.C. (U.N.O) SECOND FLOOR 2×4'S @ 16"O.C. (U.N.O) 2x4'S @ 16"O.C. (U.N.O) THIRD FLOOR

*IT IS ACCEPTABLE TO USE (2) 2x4 STUDS @ 16" O.C. IN LIEU OF 3x4 STUDS @ 16" O.C. NAIL DOUBLE STUDS TOGETHER W/ 10d NAILS @ 8" O.C.

2x4'S @ 16"O.C. (U.N.O)

2x4'S @ 16"O.C. (U.N.O)

ALL LOAD BEARING WALLS SHALL HAVE MIDSPAN HORIZONTAL BLOCKING SPACED AT 48" O.C. INSTALLED BEFORE WALLS ARE LOADED. ALL NON-LOAD BEARING PARTITIONS SHALL CONSIST OF 2x4 STUDS SPACED AT 24" O.C. 2x4 STUDS DO NOT NEED TO BE DOUBLED AT THE FIRST FLOOR FOR NON-LOAD BEARING WALLS.

- 3. PLACE A SINGLE PLATE AT THE BOTTOM AND A DOUBLE PLATE AT THE TOP OF ALL STUD WALLS. 2x SOLE PLATES AT THE EDGES OF SLABS SHALL BE ATTACHED TO THE SLAB WITH SIMPSON MAS MUDSILL ANCHORS (WITH 6 10d NAILS) AT 32" O.C., AT INTERIOR STUD WALLS PROVIDE EITHER HILTI ZF72 (WITH 2 7/8" LENGTH, 5/64" THICK WASHERS) POWDER DRIVEN FASTENERS AT 8" ON CENTER, OR 1/2" DIAMETER ANCHOR BOLTS WITH 6" EMBEDMENT, AT 32" ON CENTER. RED-HEAD FASTENERS OF EQUIVALENT SIZES MAY BE USED. ALL OTHER SUBSTITUTIONS MUST BE APPROVED BY ADVANCED STRUCTURAL ENGINEERING PRIOR TO INSTALLATION. SEE THE SHEAR WALL SCHEDULE BELOW FOR SPECIAL SOLE PLATE ATTACHMENT
- 4. STUDS SHALL BE PACKED AT ALL ANGLES, CORNERS, AROUND ALL OPENINGS AND AT SHEAR WALLS. SEE SCHEDULES, UL ASSEMBLIES AND ARCH. DETAILS FOR REQUIREMENTS.
- 5. WOOD LINTELS OVER OPENINGS SHALL BE AS NOTED PER DETAIL 4/56.03. NAIL MULTIPLE STUDS TOGETHER WITH 16d NAILS @ 12" O.C. EACH PLY.
- 6. WALL SHEATHING SHALL BE: (SEE SHEAR WALL SCHEDULE SI.02 FOR REQUIREMENTS AT SHEAR WALLS.)

AT INTERIOR WALLS PROVIDE 5/8" GYPSUM WALLBOARD (SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS) EACH SIDE OF STUDS, NAILED WITH 5d COOLER NAILS AT 7" O.C. (USE 6d COOLER NAILS FOR 5/8" WALLBOARD) AT ALL SUPPORTS. PROVIDE SOLID 2x BLOCKING AT ALL SHEET EDGES. BLOCKING IS NOT REQUIRED AT NON-LOAD BEARING PARTITIONS.

AT EXTERIOR WALLS SHEATH THE INTERIOR FACE OF WALLS WITH 5/8" GYPSUM WALLBOARD AS NOTED ABOVE FOR INTERIOR WALLS. SHEATH THE EXTERIOR FACE OF WALLS WITH

AND 8d NAILS AT 6" O.C. AT ALL INTERMEDIATE SUPPORTS. PROVIDE SOLID DOUBLE 2x BLOCKING AT ALL SHEET EDGES. SHEARWALL SCHEDULE NOTE 5.

- 1. ALL WOOD IN CONTACT WITH CONCRETE OR EXPOSED TO WEATHER SHALL BE TRESSUBE PREADED (OR 7/16" O.S.B.), NAILED WITH 8d NAILS AT 6" O.C. AT ALL EDGE SUPPORTS,
- 8. FLOOR SHEATHING IS 3/4" TONGUE AND GROOVE O.S.B., GLUED AND NAILED WITH 100 NAILS AT 6" O.C. AT SUPPORTED EDGES, AND 100 NAILS AT 10" O.C. AT INTERMEDIATE SUPPORTS.
- 9. ROOF SHEATHING SHALL BE 15/32" C D PLYWOOD OR 15/32" O.S.B. (SPAN RATING 32/16), NAILED TO TRUSSES BELOW. SEE ROOF SHEATHING NAILING SCHEDULE FOR NAIL PATTERN. PROVIDE ONE PLYWOOD CLIP PER SPAN BETWEEN SHEET EDGES FOR 24" SPAN. PROVIDE TWO PLYWOOD CLIP PER SPAN BETWEEN SHEET EDGES FOR 48" SPAN. PROVIDE SOLID 2x BLOCKING BETWEEN SUPPORTS AT ALL HIPS, RIDGES, VALLEYS AND CHANGES IN ROOF SLOPE.
- 10. ALL LUMBER SHALL BE STAMPED WITH THE GRADE MARK OF AN APPROVED GRADING AGENCY
- 11. NAILING SCHEDULE

CONNECTION	COMMON NAIL	NUMBER OR SPACING
SOLE PLATE TO TRUSS OR BLOCKING	16d	8" O.C.
STUD TO SOLE PLATE, TOE NAIL	8d	4
DOUBLE STUDS, FACE NAIL	16d	24" O.C.
DOUBLE TOP PLATES, FACE NAIL	160	6" O.C.
TOP PLATES LAPS AND INTERSECTIONS	16d	3
TRUSSES, LAPS OVER WALLS, FACE NAIL	160	4
BUILT-UP CORNER STUDS	160	12" O.C.
STUDS TO SOLE PLATE, END NAIL	160	2

- 12 WHERE WOOD BEAMS/HEADERS ABUT WOOD COLUMNS, PROVIDE SIMPSON "HHUC" CONNECTORS WITH ALL NAILS SPECIFIED BY THE MANUFACTURER.
- 13 AT ALL WALLS SUPPORTING ROOF TRUSSES PROVIDE UPLIFT STRAPPING/CONNECTORS SHOWN IN THE TYPICAL WALL ELEVATION 1/65.02. OR ALTERNATE: HURRY-BOLT SYSTEM OR EQUAL. - SUBMIT ENGINEERED SHOP DRAWINGS FOR REVIEW.
- 14. ALL PRESSURE TREATED AND FIRE RETARDANT LUMBER FASTENERS SHALL BE OF HOT-DIPPED ZINC COATED GALVANIZED STEEL
- 15. ALL SIMPSON HOLD DOWN COLLECTORS SHALL USE ALL THREAD ROD WITH DOUBLE BOLTS AT THE BOTTOM W/ A MIN. EMBEDMENT OF 9" AT INTERIOR FOOTING AND 12" AT EXTERIOR FOOTING W/2" DIA WASHERS.

PREFABRICATED WOOD TRUSSES

- 1. ALL PREFABRICATED WOOD TRUSSES SHALL BE SECURELY FASTENED AT EACH END TO THEIR SUPPORTING WALLS OR BEAMS AS SHOWN IN THE TYPICAL WALL ELEVATION 1/85.02
- 2. PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF THE "NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENERS" AS RECOMMENDED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.
- 3. TRUSS MEMBERS AND CONNECTIONS SHALL BE DESIGNED (WITH A MAXIMUM ALLOWABLE STRESS INCREASE FOR LOAD DURATION OF 25% FOR ROOF TRUSSES ONLY) TO WITHSTAND THE LIVE LOADS GIVEN IN THE NOTES AND TOTAL DEAD LOAD.
- 4. TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND DESIGN NOTES WITH A FLORIDA REGISTERED ENGINEER'S SEAL FOR APPROVAL BY THE ARCHITECT, DESIGN NOTES TO INCLUDE THE RATED LOAD CAPACITY OF THE CONNECTORS USED TO SECURE THE MEMBERS, CERTIFICATION OF THE CONNECTOR CAPACITIES AND MANUFACTURER'S LICENSE TO FABRICATE TRUSSES UTILIZING THE CONNECTOR SYSTEM PROPOSED.
- 5. THE CONTRACTOR SHALL APPROVE FABRICATION AND INSTALLATION DRAWINGS SHOWING SIZE, SHAPE AND LAYOUT PRIOR TO SUBMITTAL FOR REVIEW BY THE ARCHITECT AND BEFORE FABRICATION HAS BEGUN.
- 6. BRIDGING FOR PRE-ENGINEERED TRUSSES SHALL BE AS REQUIRED BY TRUSS MANUFACTURER, AND THE LOCAL BUILDING CODE, UNLESS NOTED ON PLANS, ALSO WHERE HEEL HEIGHT ON BOTTOM CHORD BEARING TRUSSES EXCEEDS II" PROVIDE CONTINUOUS 2x4 LET IN.

1. DESIGN LOADS - DEAD LOADS:

FLOOR TRUSSES BOTTOM CHORD 8 PSF TOP CHORD (APTS) 15 PSF 8 PSF OUTSIDE TRUSSES BOTTOM CHORD TOP CHORD 45 PSF

ROOF TRUSSES 10 PSF BOTTOM CHORD 10 PSF TOP CHORD TOP CHORD (AT OVERBUILT AREAS) 5 PSF ADDITIONAL

8. ALL SIMPSON TRUSS ANCHORS SHOWN ON DRAWINGS SHALL BE VERIFIED FOR LOADS SHOWN ON WOOD TRUSS DESIGN CALCULATIONS. THEREFORE, ANCHOR SIZES AND TYPES ARE SUBJECT TO CHANGE BY ADVANCED STRUCTURAL ENGINNERING II.

FASTENER SUBSTITUTIONS:

ALL NAILS ARE COMMON NAILS, UNLESS NOTED OTHERWISE. THE FOLLOWING FASTENERS ARE ACCEPTABLE SUBSTITUTIONS. THE ALTERNATE FASTENERS SHALL BE SPACED AT THE SAME SPACING AS THE SCHEDULED FASTENERS

ALTERNATE FASTENER SCHEDULED FASTENER 8d RING SHANK NAIL 8d COMMON NAIL

8d SCREW SHANK NAIL Ø.131 P-NAIL

100 RING SHANK NAIL 10d COMMON NAIL 10d SCREW SHANK NAIL

Ø.148 P-NAIL 160 RING SHANK NAIL 16d COMMON NAIL

16d SCREW SHANK NAIL

#6 x 1 1/4" TYPE S OR W DRYWALL SCREW 6d COOLER NAIL

WATERPROOFING FOR THIS BUILDING IS THE SOLE RESPONSIBILITY OF THE BUILDER/CONTRACTOR/ARCHITECT, HENCE NO DETAILS OF FLASHING, FLOOR, SLOPE AND STEP, NOR ANY OTHER WATERPROOFING MEASURES HAVE BEEN INCLUDED IN OUR DRAWINGS. IF SLOPE AND FLOOR STEPS ARE SHOWN ON OUR DRAWINGS, THEY SHALL BE USED ONLY AS A REFERENCE.

MASONRY WALL CONSTRUCTION

TYPE

⊢ − 1

H-2

H-3

H-4

- A. HOLLOW LOAD BEARING UNITS SHALL BE NORMAL WEIGHT, GRADE N, TYPE 2 CONFORMING TO ASTM C90, WITH A MINIMUM NET COMPRESSIVE STRENGTH OF 1900 PSI (f'm = 1500 PSI).
- B. MORTAR SHALL BE TYPE M OR S, CONFORMING TO ASTM C270.
- C. COURSE GROUT SHALL CONFORM TO ASTM C476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH OF 2.500 PSI.
- D. VERTICAL REINFORCEMENT SHALL BE AS NOTED ON THE DRAWINGS WITH CELLS FILLED WITH COARSE GROUT.
- E. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM AND AT A MAXIMUM SPACING OF 4'-0", REINFORCEMENT SHALL BE PLACED IN THE CENTER OF THE MASONRY CELL WITH DURO-WAL BAR POSITIONER D/A 811 TYPICAL UNLESS OTHERWISE NOTED. SEE TYPICAL GROUTING DETAILS FOR ADDITIONAL INFORMATION.
- F. REINFORCING STEEL SHALL BE LAPPED MINIMUM 48 BAR DIAMETERS WHERE SPLICED UNLESS NOTED OTHERWISE ON THE DRAWINGS.
- G. HORIZONTAL WALL REINFORCEMENT SHALL BE STANDARD LADDER TYPE DUR-O-WAL AT 16" O.C., UNLESS SHOWN OTHERWISE ON THE DRAWINGS.
- H. SPLICED WIRE REINFORCEMENT SHALL BE LAPPED AT LEAST 6" AND CONTAIN AT LEAST ONE CROSS WIRE OF EACH PIECE OF REINFORCEMENT WITHIN THE 6". LAP WITH STANDARD 'T' AND 'L' SHAPED PIECES AT INTERSECTIONS AND CORNERS.
- I. FOUNDATION DOWELS SHALL BE POSITIONED SUCH THAT IT IS ALIGNED WITH THE CENTER OF THE MASONRY UNIT'S VERTICAL CORE AND GROUTED SOLID. SHOULD THE FOUNDATION DOWEL NOT ALIGN WITH THE MASONRY UNIT'S VERTICAL CORE -NOTIFY THE ARCHITECT / STRUCTURAL ENGINEER FOR DIRECTION ON HOW TO CORRECT THE MIS ALIGNMENT.
- J. PROVIDE PRECAST CONCRETE LINTELS OVER ALL OPENINGS UNLESS NOTED OTHERWISE ON DRAWINGS. LINTELS SHALL BE OF SUFFICIENT SIZE AND REINFORCEMENT FOR THE GIVEN SPANS AND LOADING CONDITIONS. SUBMIT SHOP DRAWINGS WITH RATED LOAD CAPACITIES TO THE ARCHITECT FOR REVIEW.
- K. PROVIDE A KNOCK OUT BLOCK OR U-BLOCK REINFORCED WITH (1)-#5 CONTINUOUS AT THE SILL OF ALL WINDOW OPENINGS. EXTEND 8" BEYOND EACH SIDE OF THE OPENING TYPICALLY.

HEADER/BEAM

DOUBLE 2x8 FOR 4" WALL

TRIPLE 2x6 FOR 6" WALL

DOUBLE 2x10 FOR 4" WALL

TRIPLE 2x8 FOR 6" WALL

DOUBLE 2x12 FOR 4" WALL

TRIPLE 2x10 FOR 6" WALL

 $(2) 1 3/4" \times 11 7/8" LVL$

TRUSSES, AND H-1 AT OTHER WALLS.

SPACER BETWEEN MEMBERS.

1. PROVIDE WOOD HEADERS OVER ALL OPENINGS. IF NO HEADER IS

2. AT DOUBLE 2x HEADER/BEAMS PROVIDE A 3/8" PLYWOOD (OR 0.5.B.)

3. NAIL ALL MULTI-MEMBER HEADERS AND BEAMS TOGETHER WITH

16d NAILS AT 12" O.C. TOP AND BOTTOM, EACH SIDE, STAGGERED. 4. PROVIDE DOUBLE WOOD STUD UNDER WOOD HEADER EA END UN.O.

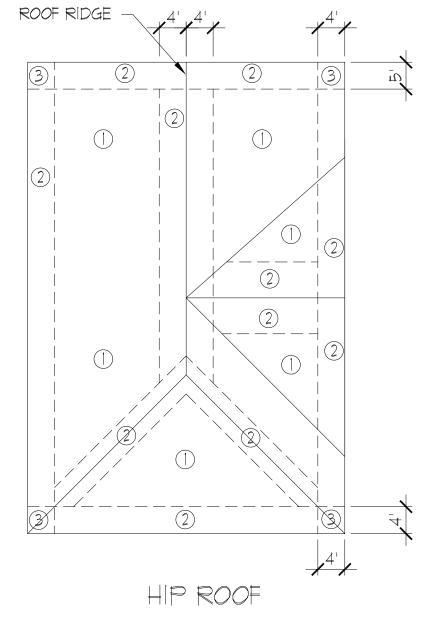
SPECIFIED, PROVIDE H-2 AT EXTERIOR WALLS AND WALLS SUPPORTING

HEADER/BEAM SCHEDULE

TYPE

H-6

ROOF FASTENING ZONES GABLE ROOF



ROOF SHEATHING FASTENING SCHEDULE NOTE: USE 8d RING SHANK NAILS IN ZONE

PANEL EDGES PANEL FIELD ① NAILS 6" O.C. (1) NAILS 6" O.C. 2 NAILS 6" O.C. 2 NAILS 6" O.C. (3) NAILS 4" O.C. 3) NAILS 4" O.C.

156 MPH WIND ZONE EXPOSURE B BUILDING CATAGORY II. BELOW VALUES BASED ON ALLOWABLE STRESS DESIGN (ASD)

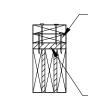
OPENING SIZE	INTERIOR ZONE	EXTERIOR ZONE**					
8' GARAGE DOOR	+25.31 / -28.64	+25.31 / -28.64					
16' GARAGE DOOR	+24.20 / -26.97	+24.20 / -26.97					
10 SQFT	+26.97 / -29.24	+26.97 / -36.10					
20 SQFT	+25.77 / -28.04	+25.77 / -33.63					
50 SQFT	+24.11 / -26.44	+24.11 / -30.44					
100 SQFT	+22.91 / -25.17	+22.91 / -28.04					
**END ZONE IS DEFINED AS AN AREA WITHIN 8'							

FROM THE EDGE OF THE BUILDING

STANDARD WINDOWS AND DOORS WILL GENERALLY FALL INTO THE 10 TO 20 SQ. FT. CATEGORY. STANDARD DOUBLE SLIDING GLASS DOOES WILL GENERALLY FALL INTO THE 20 TO 50 SQ. FT. CATEGORY. STANDARD DOUBLE GARAGE DOORS WILL GENERALLY FALL INTO THE 50 TO 100 SQ. FT. CATEGORY

WINDOW & DOOR WIND PRESSURES SCHEMATIC DESIGN DESIGN DEVELOPMENT 12/06/19 PERMIT REVIEW SET **REVISION HISTORY** Description

OPENING SIZE	INTERIOR ZONE	EXTERIOR ZONE**				
8' GARAGE DOOR	+25.31 / -28.64	+25.31 / -28.64				
16' GARAGE DOOR	+24.20 / -26.97	+24.20 / -26.97				
10 SQFT	+26.97 / -29.24	+26.97 / -36.10				
20 SQFT	+25.77 / -28.04	+25.77 / -33.63				
50 SQFT	+24.11 / -26.44	+24.11 / -3Ø.44				
100 SQFT	+22.91 / -25.17	+22.91 / -28.04				
**END ZONE IS DEFINED AS AN AREA WITHIN 8'						



HEADER/BEAM

 $(2) \mid 3/4" \times 9 \mid /2" \mid LVL$

 $(3) 1 3/4" \times 9 1/2" LVL$

 $(2) P.T. 2 \times 12$

CONNECTING TOP PLATE TO HEADER W/ 2-16D NAILS @ 8" O.C. TYP

-PLYWOOD SHIM

H-1 SECTION SCALE: N.T.S.



2-16D NAILS @ 8" O.C. TYP

H-2 SECTION SCALE: N.T.S.

12-09-19

ASE ENGINEERING SERVICES, INC. STRUCTURAL DESIGN GROUP

TO THE BEST OF MY KNOWLEDGE, THE BUILDING DESIGN PLANS AND SPECIFICATIONS
OMPLY WITH BUILDING STRUCTURAL DESIGN CODE. THE SIGNING AND SEALING OF THE PLAN
AND SPECIFICATIONS ARE ONLY FOR THE BUILDING'S STRUCTURAL COMPONENTS AFFECTED
BY WIND, LIVE AND GRAVITY LOADS. 10244 East Colonial Drive, Suite 202 Orlando, Florida 32817 — 407—677—5565 Fax 407—730—2999 Certificate of Authorization No. 25873

FUGLEBERG KOCH

CONSULTANT

2555 Temple Trail, Winter Park, FL 32789 (407) 629-0595

www.fuglebergkoch.com

PERMIT REVIEW STAME

ISSUE HISTORY

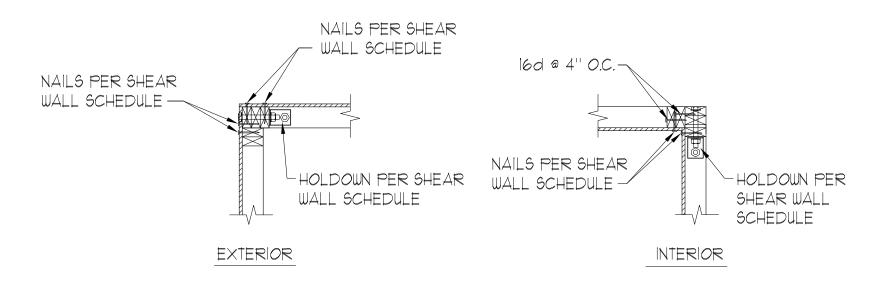
THE ROBERT 09/10/2019 FT. MYERS, FL Project #

> **GENERAL NOTES** BLDG TYPES 1, 2 & 3

> > **S0.01**

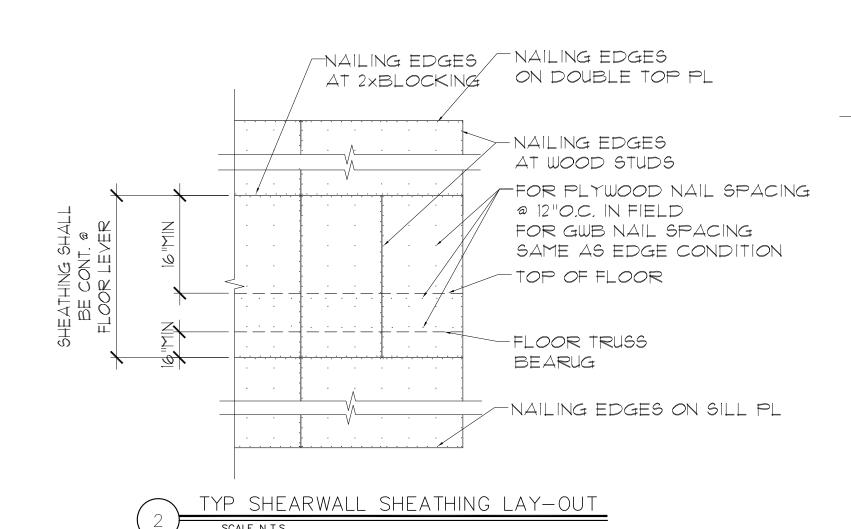
PLOTTED:

	(CONNECTOR EQUIVAL	LENT SCHEDULE	
SIMPSON TYP	FASTENERS	UPLIFT(16s.)	WOOD TO:	USP TYP
MTS16 OR MTS12	(14) 10d x 1-1/2	1000	WOOD	MTW16
HTS2Ø	(24) 10d x 1-1/2	1450	WOOD	HTW2Ø
LTS12	(12) 10d x 1-1/2	775	WOOD	LTW12
НЗ	(4)8d	455	WOOD	RTT
LTT2ØB	(10) 16d INTO STUDS	175Ø (NAILS)	CONCRETE, WOOD	LTS2ØB
HTT16	(18) 16d	4175 (NAILS)	CONCRETE, WOOD	HTT16
SP-1 SP-4	(10) 10d (6) 10d x 1-1/2	585 135	WOOD (BOTT OF STUD) WOOD (TOP OF STUD)	SPT22 SPT4
THA/THAC TYP	PER SIMPSON CATALOG	PER SIMPSON CATALOG	SELECTION BASE ON TRUSS WIDTH AND HEIGHT	MSH TYP
C\$16	(22) lØd	1650	II" MIN AT EA END FOR WOOD	RS15Ø
MAS	(6) 10d x 1-1/2	1005	SLAB, STEMWALL	FA3
HCP2 HCP4	(12) 10d x 1-1/2 (16) 10d	6 <i>0</i> 5	FOR 2× MEMBER FOR 4× MEMBER	N/A N/A
Д35	(12) 8d x1 1/2	450 SHEAR	WOOD	MPAI
CC44, ECCU44 CC66, ECCU66	(4) 5/8d BOLTS (6) 5/8d BOLTS	1465 (UPLIFT) 3660 (UPLIFT)	ECCU44 AT BEAM END ECCU66 AT BEAM END	KCC44, KECCU44 KCC66, KECCU66
PC/EPC TYP	PER SIMPSON CATALOG	PER SIMPSON CATALOG	SELECTION BASE ON POST AND BEAM WIDTH	PCM/EPCM TYP
ECCLL44 ECCLL66 ABU44	BOLTS AS REQ (12) 16d	1465 (UPLIFT) 3660 (UPLIFT)	ECCLLU44 FOR 4x4 POST ECCLLU66 FOR 6x6 POST ABU44 FOR 4x4 POST	KECCLL44 KECCLL66 PAU44
ABU66	5/8 ANCHOR BOLT	2200	ABU44 FOR 4x4 POST	PAU66
HUC410	(18) 16d FOR WOOD	1510	CONCRETE, MASONRY, WOOD FOR 4x10 BM	HD4IØIF
HUC412	(22) 16d FOR WOOD	1510	CONCRETE, MASONRY, WOOD FOR 4x12 BM	HD4121F
LUS TYP	PER SIMPSON CATALOG	PER SIMPSON CATALOG	LUS24 FOR 2x6, LUS26 FOR 2x8, LUS28 FOR 2x10, ETC.	JUS TYP



TYP. SHEARWALL CORNER HOLDOWN

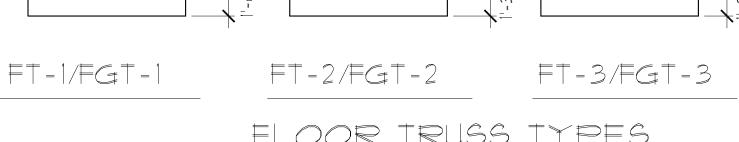
SCALE 3/4" = 1'-0"

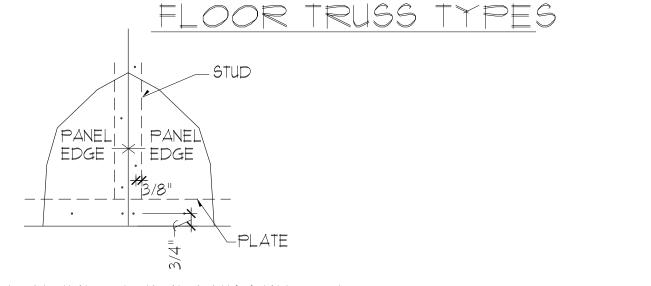


TRUSS NOTES

- 1. TRUSSES SHOWN FOR GENERAL CONFIGURATION ONLY, WEB MEMBERS ARE NOT SHOWN, BUT SHALL BE DESIGNED BY THE TRUSS MANUFACTURER IN ACCORDANCE WITH THE GENERAL NOTES. WORKING POINTS SHALL BE DETERMINED BY THE TRUSS MANUFACTURER.
- 2. TRUSS MANUFACTURER IS RESPONSIBLE FOR THE DESIGN OF ANY BRIDGING OR BRACING REQUIRED TO BRACE THE TRUSS BOTTOM CHORDS FOR WIND UPLIFT.
- 3. SEE THE ARCHITECTURAL DRAWINGS FOR ELEVATIONS, OVERHANGS AND BEARING CONDITIONS.
- 4. THE CONNECTION OF TRUSS TO TRUSS SHALL BE PROVIDED BY THE TRUSS MANUFACTUROR.
- 5. VERIFY TRUSS SLOPE WITH ARCH DRWG PRIOR TO CONSTRUCTION.

SEE ARCH FOR SLOPE





1. ALL HORIZONTAL & VERTICAL JOINTS SHALL OCCUR OVER FRAMING & SHALL BE ATTACHED IN THE SPACING SHOWN ON SHEARWALL SCHEDULE @ SHEARWALLS. SOLID 2X BLOCKING OR FRAMING MEMBER SHALL BE AT ALL PANEL EDGES.

2. FOLLOW SHEATHING ORIENTATION DIAGRAM (BELOW).
BOTTOM PLATE OF FIRST FLOOR FRAMING SHALL BE
ATTACHED TO FOUNDATION WITH MAS CONNECTORS OR
ANCHOR BOLTS AS SCHEDULED.

	SHEARWALL SCHEDULE FOR 3 STORY BUILDINGS																			
	SHEATHING NAILING FND CONN EACH END OF WALL				2nd FLOOR CONN EACH END OF WALL			¥LL	3rd FLOOR CONN EACH END OF WALL											
TYPE	FND-2nd FLR/ROOF 2nd-ROOF @ 2 STORY	1 3rd-ROOF @ 3 STORY	FND-2nd FLR/ROOF	2nd-ROOF @ 2 STORY	3rd-ROOF @ 3 STORY	CONNECTO	OR FND BOLT-12" EMBED DBL NUT EA END	ATTACH TO STUDS	REQ'D STUDS AT END OF WALL	FND SOLE PLATE ATTACHMENT	CONNECTOR	NAILS EACH END OF STRAP AT STU	THREADED ROD BTWN D CONN DBL NUT EA END	BOLTS TO STUDS	REQ'D STUDS AT END OF WALL	CONNECTOR	NAILS EACH END OF STRAP AT STUD	THREADED ROD BTWN CONN DBL NUT EA END	BOLTS TO STUDS	REQ'D STUDS AT END OF WALL
	1/16" C-D PLYWD *3 1/16" C-D PLYWD *3	3 1/16" C-D PLYWD *3	8d NAILS @ 4" O.C.	8d NAILS @ 6" O.C.	8d NAILS @ 6" O.C.	HD5B	5/8" DIA	2-3/4" DIA BOLTS	(3) 3x4	1/2" DIAx 7" EMB. A. BOLTS @ 32" O.C.	C\$20×40	(12) 1Ød	N/A	N/A	(4) 2×4	CS2Øx4Ø	(12) 1Ød	N/A	N/A	(3) 2×4
2	5/8" GYPSUM *2 5/8" GYPSUM 22 WALLBOARD	2 5/8" GYPSUM *2 WALLBOARD	6d COOLER NAILS @ 4" O.C.	6d COOLER NAILS @ 4" O.C.	6d COOLER NAILS @ 4" O.C.	HD5B	5/8" DIA	2-3/4" DIA BOLTS	(3) 3x4	1/2" DIAx 7" EMB. A. BOLTS @ 32" O.C.	CS2Øx4Ø	(12) 1Ød	N/A	N/A	(4) 2×4	C\$20×40	(12) 10d	N/A	N/A	(3) 2x4
3	1/16" C-D PLYWD *3 1/16" C-D PLYWD *3	3 1/16" C-D PLYWD *3	8d NAILS @ 4" O.C.	8d NAILS @ 6" O.C.	8d NAILS @ 6" O.C.	HD5B	5/8" DIA	2-3/4" DIA BOLTS	(3) 3x4	1/2" DIAx 7" EMB. A. BOLTS @ 32" O.C.	C520x40	(12) 1Ød	N/A	N/A	(4) 2×4	C\$2Øx4Ø	(12) 1Ød	N/A	N/A	(3) 2×4
4	1/16" C-D PLYWD *3 1/16" C-D PLYWD *3	N/A	8d NAILS @ 4" O.C.	8d NAILS @ 4" O.C.	N/A	HD5B	5/8" DIA	2-3/4" DIA BOLTS	(3) 2x4	1/2" DIAx 7" EMB. A. BOLTS @ 32" O.C.	C520x40	(12) 1Ød	N/A	N/A	(4) 2×4	N/A	N/A	N/A	N/A	N/A

- *| SHEATH BOTH FACES OF WOOD BEARING WALL. PROVIDE SOLID BLOCKING AT ALL SHEET EDGES, AND AT 4'-0" O.C. MAXIMUM BETWEEN STUDS.
- *2 THIS IS AN INTERIOR PARTY WALL, WITH SHEATHING ON ONE FACE ONLY. PROVIDE THE SHEARWALL SHEATHING SCHEDULED APPLIED DIRECTLY TO THE STUDS WITH THE SCHEDULED NAILING PATTERN.
- *3 SHEATH EXTERIOR FACE WITH SHEARWALL SCHEDULED, AND THE INTERIOR FACE OF STUDS AS SPECIFIED IN THE GENERAL NOTES FOR INTERIOR WALLS.
- * 4 ALL ANCHOR BOLTS SHALL HAVE 2" DIA WASHERS.

* SUBJECT TO COORDINATION WITH TRUSS ENGINEERING CRITERIA

NOTES:

- I ALL HD, MAS, AND CS FASTENERS ARE AS MANUFACTURED BY THE SIMPSON STRONG-TIE COMPANY, INC., OR EQUAL.
- 2 WHERE SCHEDULED ADDITIONAL STUDS AT THE ENDS OF SHEAR WALLS ARE INTERRUPTED AT TRUSS BEARING, PROVIDE SOLID BLOCKING IN THE TRUSS SPACE TO MATCH THE STUDS SCHEDULED FOR THE LOWER FLOOR.
- 3 WHERE SCHEDULED SHEATHING CANNOT EXTEND FULL HEIGHT OF WALL, PROVIDE SHEATHING OR BLOCKING BETWEEN FLOOR TRUSSES AT ALL SHEARWALLS.
- 4 SHEATHING SHALL EXTEND FULL HEIGHT OF SHEARWALLS. STAGGER SHEATHING HORIZONTAL JOINTS SO THEY DO NOT FALL AT HORIZONTAL JOINT BETWEEN DOUBLE TOP PLATES.
- 5 WHEREVER THE SHEATHING OF A SHEAR WALL IS INTERRUPTED (I.E. BY AN INTERSECTING WALL) IN THE VERTICAL PLANE, THE SCHEDULED "ADD'L STUD AT END OF WALL" AND HOLDDOWN ANCHORS SHALL BE PROVIDED AT THE END OF THE SHEATHING (ONE EACH SIDE OF THE INTERRUPTION). THE QUANTITY OF SHEARWALL CALLOUTS ON THE PLANS MAY NOT ACCURATELY REFLECT THE NUMBER OF HOLDDOWNS REQUIRED BECAUSE OF THIS. THE CONTRACTOR MUST FIRST DETERMINE WHERE THE SHEARWALL SHEATHING WILL BE INTERRUPTED BEFORE DETERMINING THE NUMBER HOLDDOWNS REQUIRED.
- 6 WHERE BOLTS ARE CALLED OUT FOR HOLDDOWN ANCHORS, THE BOLTS SHALL BE THROUGH BOLTS CONFORMING TO ASTM A3ØT.
- 1 1/16" O.S.B. MAY BE USED IN LIEU OF THE 1/16" C-DX PLYWOOD SHEATHING EXCEPT FOR ROOF TRUSS SHEATHING.
- 8 ANCHOR BOLTS SHALL ALL HAVE 3" HOOKS. A36 ALL THREAD RODS, DRILLED AND EPOXY SET INTO
- FOOTINGS, MAY BE SUBSTITUTED FOR ANCHOR BOLTS CAST IN PLACE. ALL THREAD RODS SHALL HAVE SAME EMBEDMENT DEPTH AS ANCHOR BOLTS.

PERMIT REVIEW STAMP

ISSUE HISTORY

11/22/19 SCHEMATIC DESIGN

12/06/19 DESIGN DEVELOPMENT
02/28/20 PERMIT REVIEW SET

2555 Temple Trail, Winter Park, FL 32789 (407) 629-0595 www.fuglebergkoch.com

FUGLEBERG KOCH

CONSULTANT

ASE ENGINEERING SERVICES, INC.

STRUCTURAL DESIGN GROUP

TO THE BEST OF MY KNOWLEDGE, THE BUILDING DESIGN PLANS AND SPECIFICATIONS
COMPLY WITH BUILDING STRUCTURAL DESIGN CODE. THE SIGNING AND SEALING OF THE PLANS
AND SPECIFICATIONS ARE ONLY FOR THE BUILDING'S STRUCTURAL COMPONENTS AFFECTED
BY WIND, LIVE AND GRAVITY LOADS.

10244 East Colonial Drive, Suite 202
Orlando, Florida 32817 - 407-677-5565 Fax 407-730-2999
Certificate of Authorization No. 25873

THE ROBERT

Checked:

Approval:

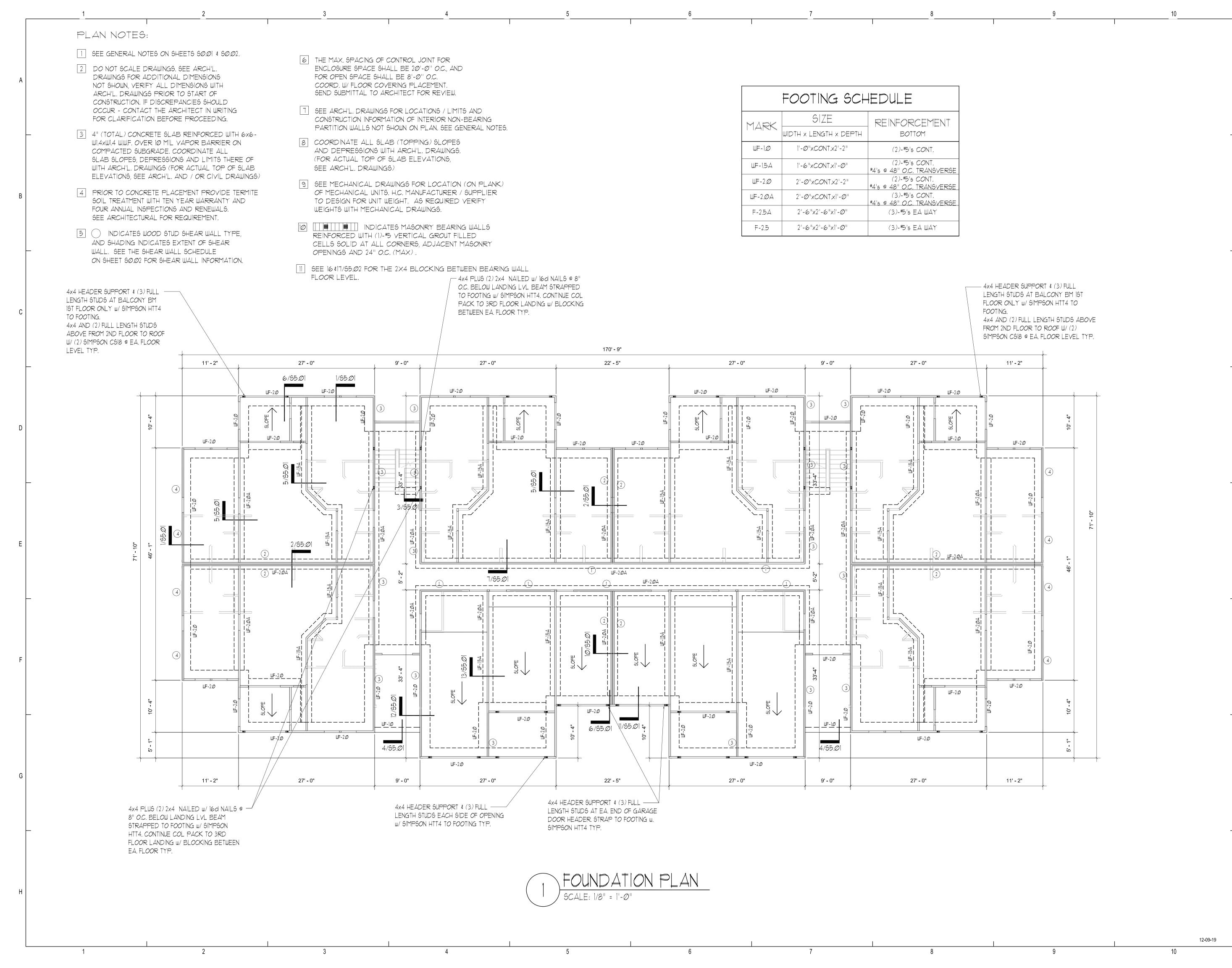
Date:

Project #:

GENERAL NOTES BLDG TYPES 1, 2 & 3

09/10/2019

S0.02



PERMIT REVIEW STAMP **ISSUE HISTORY** 11/22/19 SCHEMATIC DESIGN 12/06/19 DESIGN DEVELOPMENT 02/28/20 PERMIT REVIEW SET REVISION HISTORY Description FUGLEBERG KOCH 2555 Temple Trail, Winter Park, FL 32789 (407) 629-0595 www. fuglebergkoch.com CONSULTANT ASE ENGINEERING SERVICES, INC. 10244 East Colonial Drive, Suite 202 Orlando, Florida 32817 — 407—677—5565 Fax 407—730—2999 Certificate of Authorization No. 25873

S1.01

BUILDING TYPE 1

FOUNDATION PLAN

THE ROBERT

FT. MYERS, FL

PLOTTED:

- SEE GENERAL NOTES ON SHEET SO.O1 & SO.O2.
- DO NOT SCALE DRAWINGS. SEE ARCH'L.

 DRAWINGS FOR ADDITIONAL DIMENSIONS

 NOT SHOWN, VERIFY ALL DIMENSIONS WITH

 ARCH'L. DRAWINGS PRIOR TO START OF

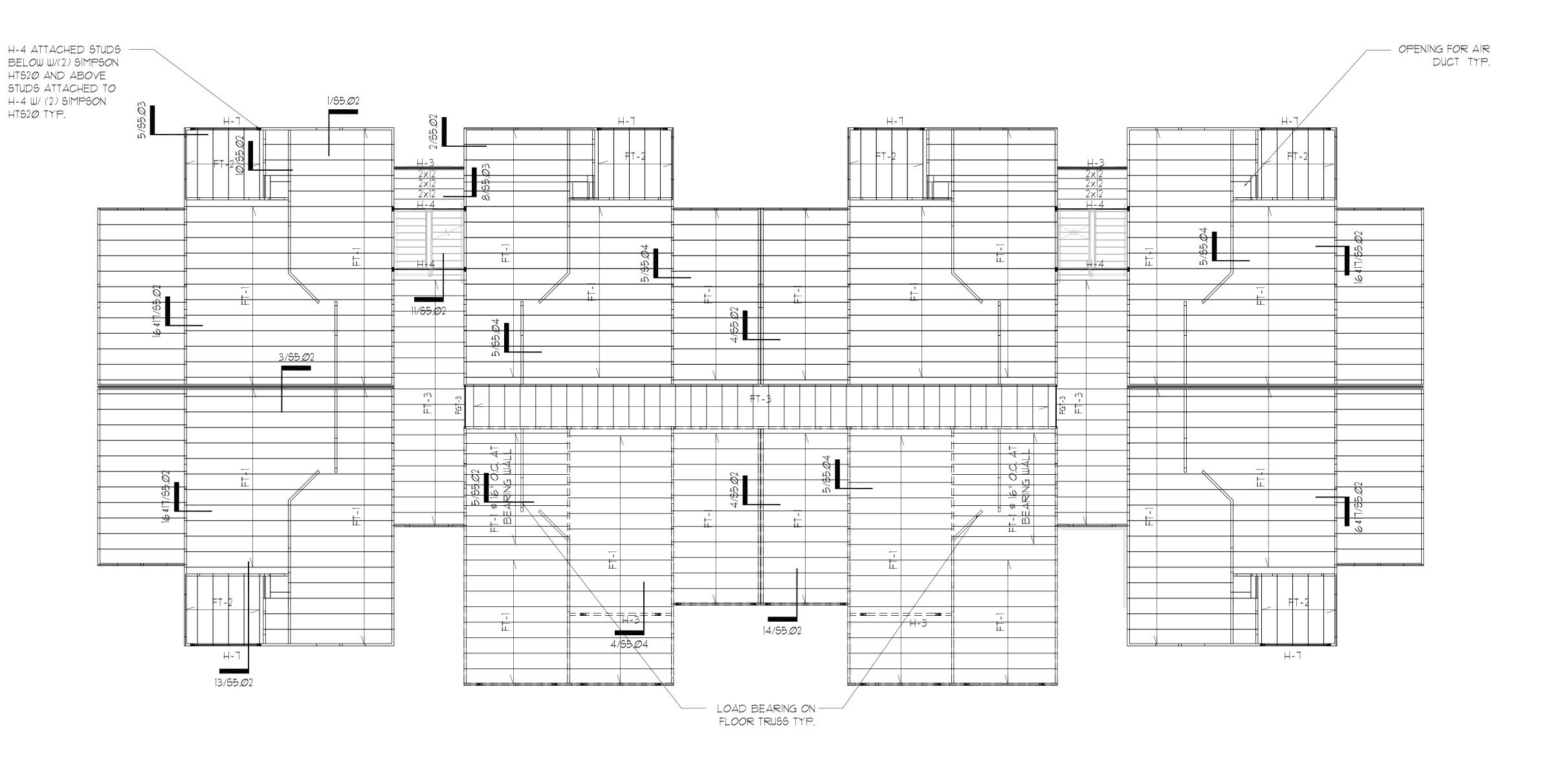
 CONSTRUCTION. IF DISCREPANCIES SHOULD

 OCCUR CONTACT THE ARCHITECT IN WRITING

 FOR CLARIFICATION BEFORE PROCEEDING.
- 3 INDICATES WOOD STUD SHEAR WALL TYPE, AND SHADING INDICATES EXTENT OF SHEAR WALL. SEE THE SHEAR WALL SCHEDULE ON SHEET SØ. Ø2 FOR SHEAR WALL INFORMATION.
- 4 SEE ARCH'L. DRAWINGS FOR LOCATIONS/LIMITS
 AND CONSTRUCTION INFORMATION OF INTERIOR
 NON-BEARING PARTITION WALLS NOT SHOWN ON
 PLAN. SEE GENERAL NOTES (SHEET SØ.Ø1) FOR
 ADDITIONAL WALL FRAMING INFORMATION.
- PROVIDE WOOD HEADER OVER ALL OPENINGS
 IN WOOD WALLS (COORD. EXACT SIZE, LOCATION
 AND ELEVATIONS WITH ARCH'L. DRAWINGS) IF NO
 HEADER TYPE HAS BEEN CALL-OUT ON PLAN,
 PROVIDE WOOD HEADER BASED ON HEADER
 SCHEDULE SHOWN ON SO.01 SHEET.
- PRE-ENGINEERED 18" DEEP WOOD TRUSSES AT 24" O.C. (MAX) U.N.O., 15" DEEP WOOD TRUSSES AT 24" O.C. (MAX) U.N.O AT BALCONY/BREEZEWAY WITH 3/4" P.T. C-D PLYWOOD SHEATHING.

- COORDINATE LOCATION OF FLOOR TRUSSES W/MECH. DWGS FOR LOCATION OF EXHAUST FAN, AIR HANDLING, AND DUCTS.
- 8 F.G.T. INDICATES FLOOR GIRDER TRUSS.
- THE REQUIRED NUMBER OF FASTENERS FOR CONNECTORS SUCH AS NAILS TO WOOD MEMBERS, SEE SIMPSON CATALOG.
- SEE THE GENERAL NOTES FOR FLOOR SHEATHING INFORMATION, AND FOR WOOD TRUSS TYPES.
- PROVIDE DOUBLE WOOD STUDS UNDER WOOD HEADER,
 WOOD BEAM, F.G.T., G.T. CONTINUOUS TO THE FTG. U.N.O.
 (PROVIDE (2) 2X4 WOOD BLOCKING BETWEEN TRUSS TOP
 AND BOTTOM CORDS BELOW)
- 12 AT DOORWAYS FOR INTERIOR BEARING WALLS, ADD A (2) 2x BLOCKING BETWEEN FLOOR LEVELS.
- 13 SEE ARCH DRAWINGS FOR SPECS ON LIGHTWEIGHT CONCRETE OVER WOOD JOIST TYP.
- 14 INDICATES MASONRY BEARING WALLS REINFORCED WITH (1)-#5 VERTICAL GROUT FILLED CELLS SOLID AT ALL CORNERS, ADJACENT MASONRY OPENINGS AND 24" O.C. (MAX).
- PROVIDE DOUBLE KNOCK OUT BLOCK BOND BEAM AT EA FLOOR LEVEL WITH #5 BARS AT EA COURSE.

PROVIDE PRECAST "U" LINTELS OVER ALL OPENINGS IN MASONRY WALL (COORD. EXACT SIZE, LOCATIONS AND ELEVATIONS WITH ARCH'L. DRAWINGS). IF NO LINTEL TYPE HAS BEEN CALL-OUT ON PLAN, PROVIDE MASONRY LINTEL TYPE: 8FI6-IB/IT, AND 8FI4-IB/IT FOR DOOR SEE "LINTEL SCHEDULE" (ON SHEET S5.05) FOR LINTEL TYPE SIZES AND REINFORCING. PROVIDE TEMPORARY SHORING DURING CONSTRUCTION IF LINTEL SPAN IS GREATER THAN 6 (SIX) FEET.





ISSUE HISTORY

No. Date Description

1 11/22/19 SCHEMATIC DESIGN
2 12/06/19 DESIGN DEVELOPMENT
3 02/28/20 PERMIT REVIEW SET

REVISION HISTORY

No. Date Description
- - - -

ASE ENGINEERING SERVICES, INC.

STRUCTURAL DESIGN GROUP

TO THE BEST OF MY KNOWLEDGE, THE BUILDING DESIGN PLANS AND SPECIFICATIONS COMPLY WITH BUILDING STRUCTURAL DESIGN CODE. THE SIGNING AND SEALING OF THE PLANS AND SPECIFICATIONS ARE ONLY FOR THE BUILDING'S STRUCTURAL COMPONENTS AFFECTED BY WIND, LIVE AND GRAVITY LOADS.

10244 East Colonial Drive, Suite 202

Orlando, Florida 32817 — 407-677-5565 Fax 407-730-2999

Certificate of Authorization No. 25873

Minsheng Xie P.E.

FUGLEBERG KOCH

CONSULTANT

2555 Temple Trail, Winter Park, FL 32789 (407) 629-0595

www. fuglebergkoch.com

BUILDING TYPE 1
2ND FLOOR FRAMING PLAN

S1.02

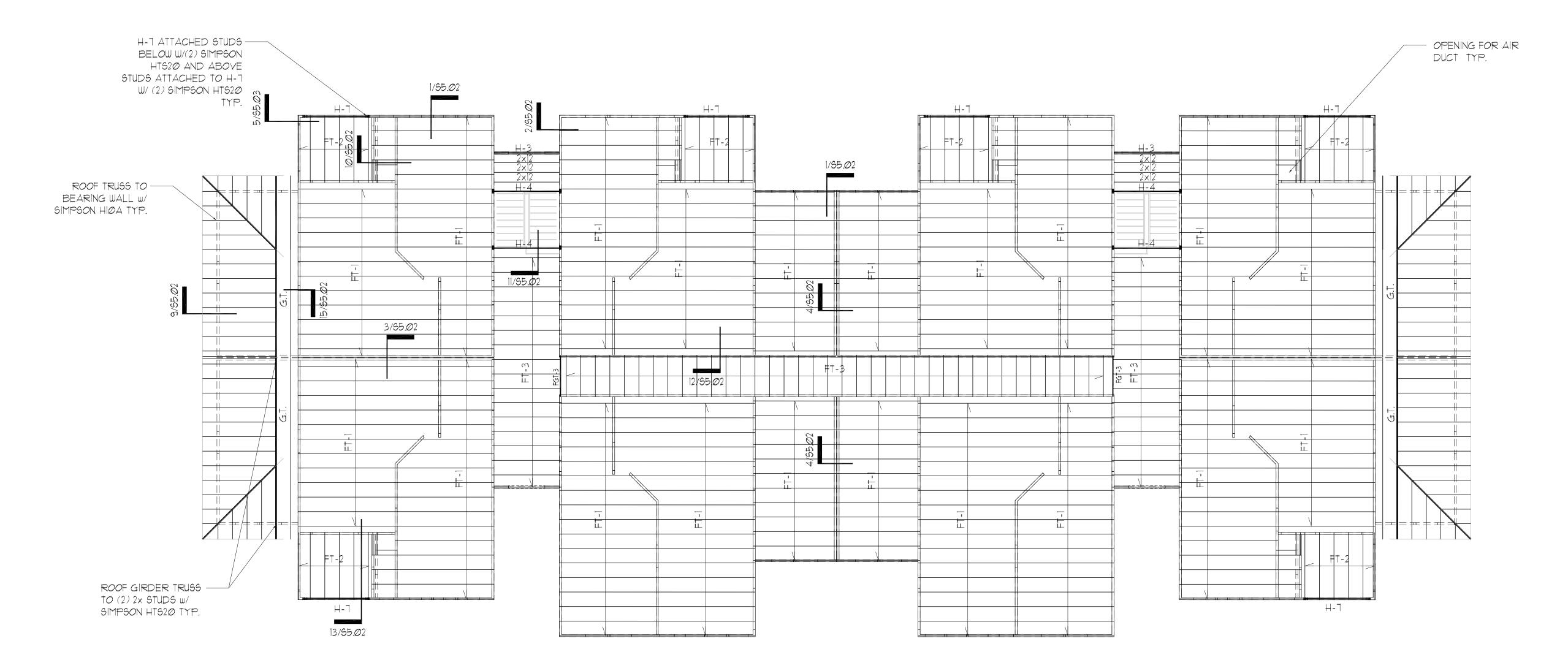
PLOTTED:

12-09-19

- SEE GENERAL NOTES ON SHEET SØ.Ø1 & SØ.Ø2.
- DO NOT SCALE DRAWINGS. SEE ARCH'L.
 DRAWINGS FOR ADDITIONAL DIMENSIONS
 NOT SHOWN, VERIFY ALL DIMENSIONS WITH
 ARCH'L. DRAWINGS PRIOR TO START OF
 CONSTRUCTION. IF DISCREPANCIES SHOULD
 OCCUR CONTACT THE ARCHITECT IN WRITING
 FOR CLARIFICATION BEFORE PROCEEDING.
- 3 INDICATES WOOD STUD SHEAR WALL TYPE,
 AND SHADING INDICATES EXTENT OF SHEAR
 WALL. SEE THE SHEAR WALL SCHEDULE
 ON SHEET SO.02 FOR SHEAR WALL INFORMATION.
- 4 SEE ARCH'L. DRAWINGS FOR LOCATIONS/LIMITS
 AND CONSTRUCTION INFORMATION OF INTERIOR
 NON-BEARING PARTITION WALLS NOT SHOWN ON
 PLAN. SEE GENERAL NOTES (SHEET SØ.Ø1) FOR
 ADDITIONAL WALL FRAMING INFORMATION.
- PROVIDE WOOD HEADER OVER ALL OPENINGS
 IN WOOD WALLS (COORD. EXACT SIZE, LOCATION
 AND ELEVATIONS WITH ARCH'L. DRAWINGS) IF NO
 HEADER TYPE HAS BEEN CALL-OUT ON PLAN,
 PROVIDE WOOD HEADER BASED ON HEADER
 SCHEDULE SHOWN ON SØ.ØI SHEET.
- PRE-ENGINEERED 18" DEEP WOOD TRUSSES AT 24" O.C. (MAX) U.N.O., 15" DEEP WOOD TRUSSES AT 24" O.C. (MAX) U.N.O AT BALCONY/BREEZEWAY WITH 3/4" P.T. C-D PLYWOOD SHEATHING.

- TOORDINATE LOCATION OF FLOOR TRUSSES W/MECH. DWGS FOR LOCATION OF EXHAUST FAN, AIR HANDLING, AND DUCTS.
- 8 F.G.T. INDICATES FLOOR GIRDER TRUSS.
- THE REQUIRED NUMBER OF FASTENERS FOR CONNECTORS SUCH AS NAILS TO WOOD MEMBERS, SEE SIMPSON CATALOG.
- SEE THE GENERAL NOTES FOR FLOOR SHEATHING INFORMATION, AND FOR WOOD TRUSS TYPES.
- PROVIDE DOUBLE WOOD STUDS UNDER WOOD HEADER, WOOD BEAM, F.G.T., G.T. CONTINUOUS TO THE FTG. U.N.O. (PROVIDE (2) 2X4 WOOD BLOCKING BETWEEN TRUSS TOP AND BOTTOM CORDS BELOW)
- 12 AT DOORWAYS FOR INTERIOR BEARING WALLS, ADD A (2) 2x BLOCKING BETWEEN FLOOR LEVELS.
- 13 SEE ARCH DRAWINGS FOR SPECS ON LIGHTWEIGHT CONCRETE OVER WOOD JOIST TYP.
- 14 INDICATES MASONRY BEARING WALLS REINFORCED WITH (1)-#5 VERTICAL GROUT FILLED CELLS SOLID AT ALL CORNERS, ADJACENT MASONRY OPENINGS AND 24" O.C. (MAX).
- PROVIDE DOUBLE KNOCK OUT BLOCK BOND BEAM AT EA FLOOR LEVEL WITH #5 BARS AT EA COURSE.

PROVIDE PRECAST "U" LINTELS OVER ALL OPENINGS IN MASONRY WALL (COORD. EXACT SIZE, LOCATIONS AND ELEVATIONS WITH ARCH'L. DRAWINGS). IF NO LINTEL TYPE HAS BEEN CALL-OUT ON PLAN, PROVIDE MASONRY LINTEL TYPE: 8FI6-IB/IT, AND 8FI4-IB/IT FOR DOOR SEE "LINTEL SCHEDULE" (ON SHEET 95.05) FOR LINTEL TYPE SIZES AND REINFORCING. PROVIDE TEMPORARY SHORING DURING CONSTRUCTION IF LINTEL SPAN IS GREATER THAN 6 (SIX) FEET.





PERMIT REVIEW STAMP **ISSUE HISTORY** No. Date 11/22/19 SCHEMATIC DESIGN 2 12/06/19 DESIGN DEVELOPMENT 3 02/28/20 PERMIT REVIEW SET **REVISION HISTORY** Date Description FUGLEBERG KOCH 2555 Temple Trail, Winter Park, FL 32789 (407) 629-0595 www.fuglebergkoch.com CONSULTANT

ASE ENGINEERING SERVICES, INC.

STRUCTURAL DESIGN GROUP

TO THE BEST OF MY KNOWLEDGE, THE BUILDING 1996 PLANS AND SPECIFICATIONS COMPLY WITH BUILDING STRUCTURAL DESIGN CODE. THE SIGNING AND SEALING OF THE PLAND SPECIFICATIONS ARE ONLY FOR THE BUILDING'S STRUCTURAL COMPONENTS AFFECTE BY WIND, LIVE AND GRAVITY LOADS.

10244 East Colonial Drive, Suite 202

Orlando, Florida 32817 — 407-677-5565 Fox 407-730-2999

Certificate of Authorization No. 25873

THE ROBERT

12-09-19

FT. MYERS, FL

BUILDING TYPE 1 3RD FLOOR FRAMING PLAN

S1.03

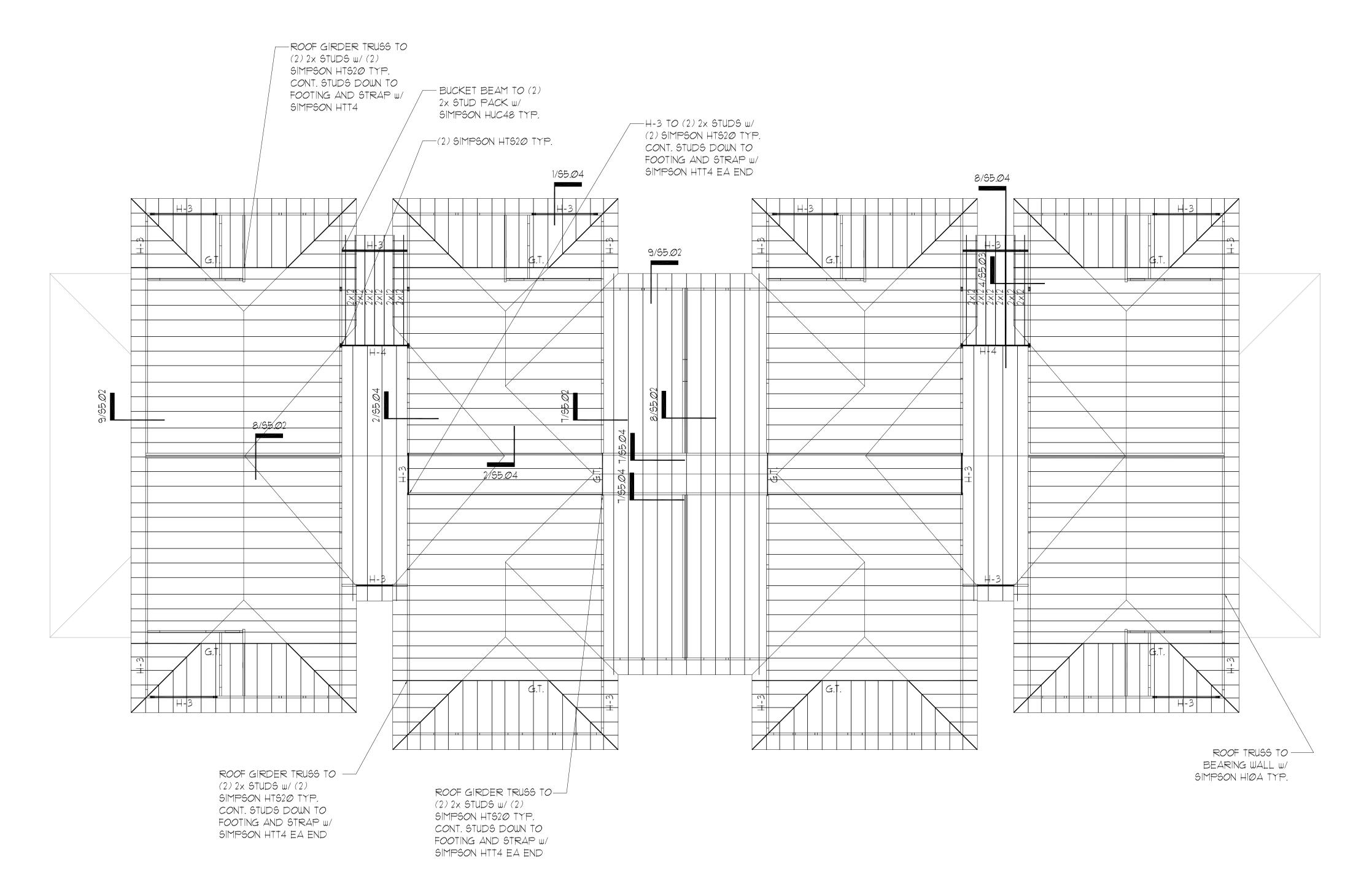
09/10/2019

Project #:



- SEE GENERAL NOTES ON SHEET SØ.Ø1 & SØ.Ø2.
- DO NOT SCALE DRAWINGS. SEE ARCH'L.
 DRAWINGS FOR ADDITIONAL DIMENSIONS
 NOT SHOWN, VERIFY ALL DIMENSIONS WITH
 ARCH'L. DRAWINGS PRIOR TO START OF
 CONSTRUCTION. IF DISCREPANCIES SHOULD
 OCCUR CONTACT THE ARCHITECT IN WRITING
 FOR CLARIFICATION BEFORE PROCEEDING.
- 3 SEE ARCH'L. DRAWINGS FOR LOCATIONS/LIMITS
 AND CONSTRUCTION INFORMATION OF INTERIOR
 NON-BEARING PARTITION WALLS NOT SHOWN ON
 PLAN. SEE GENERAL NOTES (SHEET SØ.ØI) FOR
 ADDITIONAL WALL FRAMING INFORMATION.
- PROVIDE WOOD HEADER OVER ALL OPENINGS
 IN WOOD WALLS (COORD. EXACT SIZE, LOCATION
 AND ELEVATIONS WITH ARCH'L. DRAWINGS) IF NO
 HEADER TYPE HAS BEEN CALL-OUT ON PLAN,
 PROVIDE WOOD HEADER BASED ON HEADER
 SCHEDULE SHOWN ON SO.OI SHEET.

- 5 PRE-ENGINEERED WOOD TRUSSES & MISCELLANEOUS WOOD FRAMING SPACED AT 24"O.C.(MAX).
- 6 O.B. INDICATES OVER-BUILT PRE-ENG. TRUSS.
-] G.T. INDICATES GIRDER TRUSS.
- THE REQUIRED NUMBER OF FASTENERS FOR CONNECTORS SUCH AS NAILS TO WOOD MEMBERS, SEE SIMPSON CATALOG.
- 9 SEE THE GENERAL NOTES FOR ROOF SHEATHING INFORMATION, AND FOR WOOD TRUSS TYPES.
- TRUSS ENGINEER TO DESIGN TRUSS TO INCORPORATE LOADS FROM MECHANICAL UNITS.
- PROVIDE DOUBLE WOOD STUDS UNDER WOOD HEADER, WOOD BEAM, F.G.T., G.T. TO THE FTG. U.N.O.





PERMIT REVIEW STAMP **ISSUE HISTORY** No. Date 1 11/22/19 SCHEMATIC DESIGN 2 12/06/19 DESIGN DEVELOPMENT 3 02/28/20 PERMIT REVIEW SET REVISION HISTORY Date Description FUGLEBERG KOCH 2555 Temple Trail, Winter Park, FL 32789 (407) 629-0595 www.fuglebergkoch.com CONSULTANT

ASE ENGINEERING SERVICES, INC.

STRUCTURAL DESIGN GROUP

TO THE BEST OF HY KNOWLEDGE, THE BUILDING DESIGN PLANS AND SPECIFICATIONS
COMPLY WITH BUILDING STRUCTURAL DESIGN CODE. THE SIGNING AND SEALING OF THE PLAND SPECIFICATIONS ARE ONLY FOR THE BUILDING'S STRUCTURAL COMPONENTS AFFECTE
BY WIND, LIVE AND GRAVITY LOADS.

10244 East Colonial Drive, Suite 202
Orlando, Florida 32817 - 407-677-5565 Fax 407-730-2999
Certificate of Authorization No. 25873

Minsheng Xie P.E.
Florida No. 51161

THE ROBERT

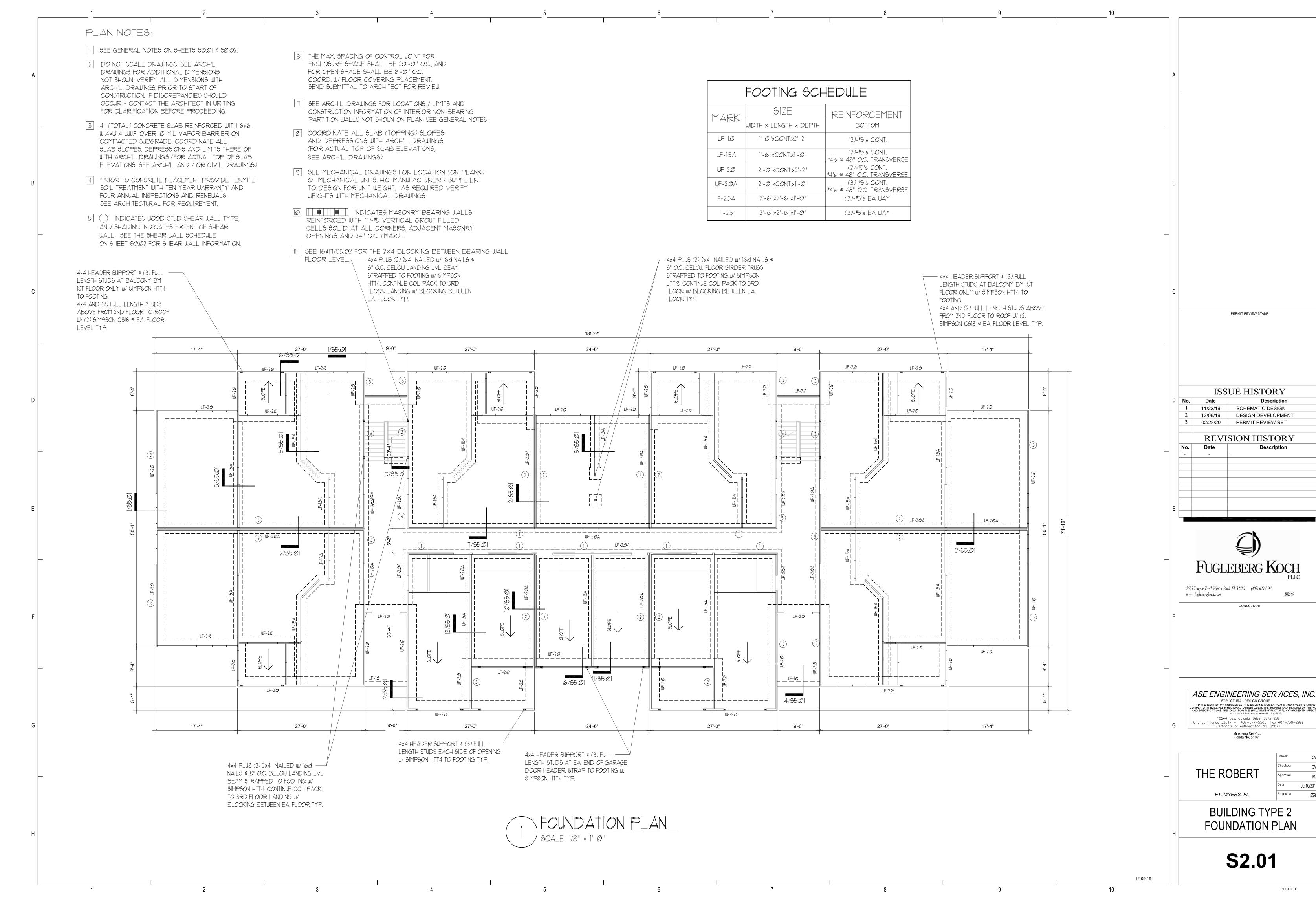
12-09-19

FT. MYERS, FL Project #:

09/10/2019

BUILDING TYPE 1 ROOF FRAMING PLAN

S1.04



Description

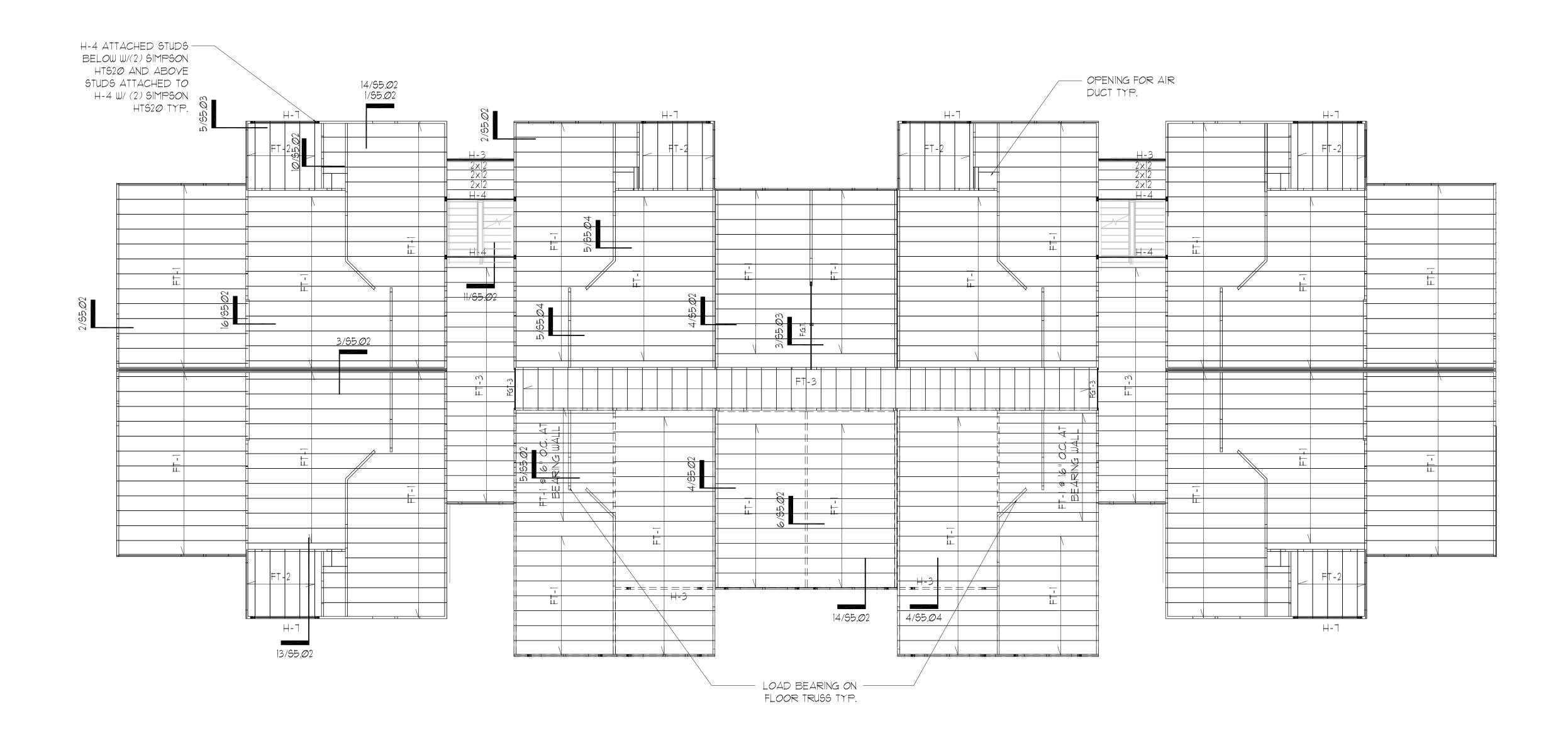
09/10/2019

PLOTTED:

- SEE GENERAL NOTES ON SHEET SØ.Ø1 & SØ.Ø2.
- 2 DO NOT SCALE DRAWINGS. SEE ARCH'L. DRAWINGS FOR ADDITIONAL DIMENSIONS NOT SHOWN, VERIFY ALL DIMENSIONS WITH ARCH'L. DRAWINGS PRIOR TO START OF CONSTRUCTION. IF DISCREPANCIES SHOULD OCCUR - CONTACT THE ARCHITECT IN WRITING FOR CLARIFICATION BEFORE PROCEEDING.
- 3 () INDICATES WOOD STUD SHEAR WALL TYPE, AND SHADING INDICATES EXTENT OF SHEAR WALL. SEE THE SHEAR WALL SCHEDULE ON SHEET SØ.Ø2 FOR SHEAR WALL INFORMATION.
- 4 SEE ARCH'L. DRAWINGS FOR LOCATIONS/LIMITS AND CONSTRUCTION INFORMATION OF INTERIOR NON-BEARING PARTITION WALLS NOT SHOWN ON PLAN. SEE GENERAL NOTES (SHEET SO.01) FOR ADDITIONAL WALL FRAMING INFORMATION.
- PROVIDE WOOD HEADER OVER ALL OPENINGS IN WOOD WALLS (COORD, EXACT SIZE, LOCATION AND ELEVATIONS WITH ARCH'L, DRAWINGS) IF NO HEADER TYPE HAS BEEN CALL-OUT ON PLAN, PROVIDE WOOD HEADER BASED ON HEADER SCHEDULE SHOWN ON SO.01 SHEET.
- 6 PRE-ENGINEERED 18" DEEP WOOD TRUSSES AT 24" O.C. (MAX) U.N.O., 15" DEEP WOOD TRUSSES AT 24" O.C. (MAX) U.N.O AT BALCONY/BREEZEWAY WITH 3/4" P.T. C-D PLYWOOD SHEATHING.

- COORDINATE LOCATION OF FLOOR TRUSSES W/ MECH. DWGS FOR LOCATION OF EXHAUST FAN, AIR HANDLING, AND DUCTS.
- 8 F.G.T. INDICATES FLOOR GIRDER TRUSS.
- THE REQUIRED NUMBER OF FASTENERS FOR CONNECTORS SUCH AS NAILS TO WOOD MEMBERS, SEE SIMPSON CATALOG.
- SEE THE GENERAL NOTES FOR FLOOR SHEATHING INFORMATION, AND FOR WOOD TRUSS TYPES.
- 11 PROVIDE DOUBLE WOOD STUDS UNDER WOOD HEADER, WOOD BEAM, F.G.T., G.T. CONTINUOUS TO THE FTG. U.N.O. (PROVIDE (2) 2X4 WOOD BLOCKING BETWEEN TRUSS TOP AND BOTTOM CORDS BELOW)
- 12 AT DOORWAYS FOR INTERIOR BEARING WALLS, ADD A (2) 2x BLOCKING BETWEEN FLOOR LEVELS.
- 13 SEE ARCH DRAWINGS FOR SPECS ON LIGHTWEIGHT CONCRETE OVER WOOD JOIST TYP.
- 14 INDICATES MASONRY BEARING WALLS REINFORCED WITH (1)-#5 VERTICAL GROUT FILLED CELLS SOLID AT ALL CORNERS, ADJACENT MASONRY OPENINGS AND 24" O.C. (MAX).
- 15 PROVIDE DOUBLE KNOCK OUT BLOCK BOND BEAM AT EA FLOOR LEVEL WITH #5 BARS AT EA COURSE.

PROVIDE PRECAST "U" LINTELS OVER ALL OPENINGS IN MASONRY WALL (COORD. EXACT SIZE, LOCATIONS AND ELEVATIONS WITH ARCH'L. DRAWINGS). IF NO LINTEL TYPE HAS BEEN CALL-OUT ON PLAN, PROVIDE MASONRY LINTEL TYPE: 8F16-1B/IT, AND 8F14-1B/IT FOR DOOR SEE "LINTEL SCHEDULE" (ON SHEET S5.05) FOR LINTEL TYPE SIZES AND REINFORCING. PROVIDE TEMPORARY SHORING DURING CONSTRUCTION IF LINTEL SPAN IS GREATER THAN 6 (SIX) FEET.





PERMIT REVIEW STAMP **ISSUE HISTORY** No. Date 11/22/19 SCHEMATIC DESIGN 2 12/06/19 DESIGN DEVELOPMENT 3 02/28/20 PERMIT REVIEW SET **REVISION HISTORY** Date Description



www.fuglebergkoch.com CONSULTANT

ASE ENGINEERING SERVICES, INC. 10244 East Colonial Drive, Suite 202 Orlando, Florida 32817 — 407—677—5565 Fax 407—730—2999 Certificate of Authorization No. 25873

THE ROBERT

12-09-19

Project #: FT. MYERS, FL

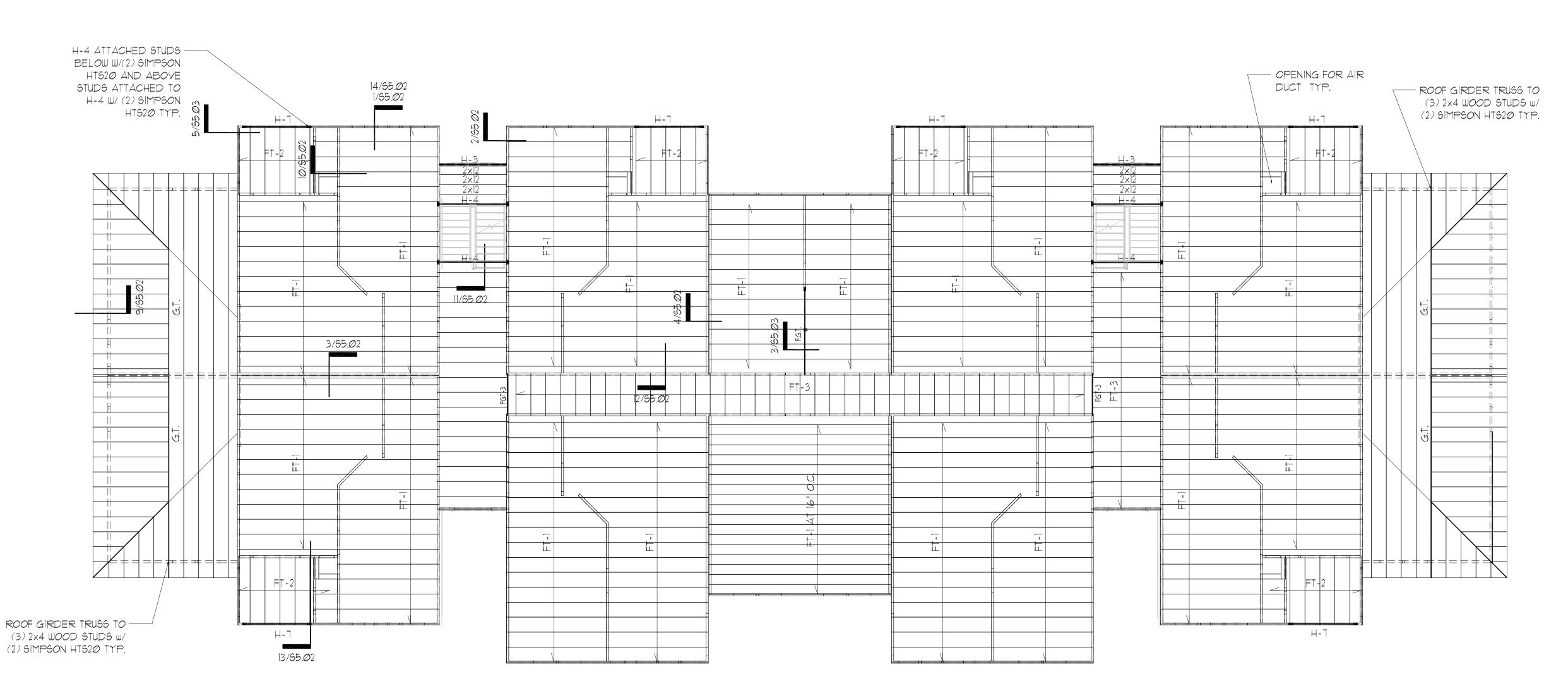
BUILDING TYPE 2 2ND FLOOR FRAMING PLAN

S2.02

- SEE GENERAL NOTES ON SHEET SØ.Ø1 & SØ.Ø2.
- DO NOT SCALE DRAWINGS. SEE ARCH'L.
 DRAWINGS FOR ADDITIONAL DIMENSIONS
 NOT SHOWN, VERIFY ALL DIMENSIONS WITH
 ARCH'L. DRAWINGS PRIOR TO START OF
 CONSTRUCTION. IF DISCREPANCIES SHOULD
 OCCUR CONTACT THE ARCHITECT IN WRITING
 FOR CLARIFICATION BEFORE PROCEEDING.
- 3 INDICATES WOOD STUD SHEAR WALL TYPE,
 AND SHADING INDICATES EXTENT OF SHEAR
 WALL. SEE THE SHEAR WALL SCHEDULE
 ON SHEET SO.02 FOR SHEAR WALL INFORMATION.
- 4 SEE ARCH'L. DRAWINGS FOR LOCATIONS/LIMITS
 AND CONSTRUCTION INFORMATION OF INTERIOR
 NON-BEARING PARTITION WALLS NOT SHOWN ON
 PLAN. SEE GENERAL NOTES (SHEET SØ.Ø1) FOR
 ADDITIONAL WALL FRAMING INFORMATION.
- PROVIDE WOOD HEADER OVER ALL OPENINGS
 IN WOOD WALLS (COORD. EXACT SIZE, LOCATION
 AND ELEVATIONS WITH ARCH'L. DRAWINGS) IF NO
 HEADER TYPE HAS BEEN CALL-OUT ON PLAN,
 PROVIDE WOOD HEADER BASED ON HEADER
 SCHEDULE SHOWN ON SO.OI SHEET.
- PRE-ENGINEERED 18" DEEP WOOD TRUSSES AT 24" O.C. (MAX) U.N.O., 15" DEEP WOOD TRUSSES AT 24" O.C. (MAX) U.N.O AT BALCONY/BREEZEWAY WITH 3/4" P.T. C-D PLYWOOD SHEATHING.

- COORDINATE LOCATION OF FLOOR TRUSSES W/ MECH. DWGS FOR LOCATION OF EXHAUST FAN, AIR HANDLING, AND DUCTS.
- 8 F.G.T. INDICATES FLOOR GIRDER TRUSS.
- THE REQUIRED NUMBER OF FASTENERS FOR CONNECTORS SUCH AS NAILS TO WOOD MEMBERS, SEE SIMPSON CATALOG.
- SEE THE GENERAL NOTES FOR FLOOR SHEATHING INFORMATION, AND FOR WOOD TRUSS TYPES.
- PROVIDE DOUBLE WOOD STUDS UNDER WOOD HEADER,
 WOOD BEAM, F.G.T., G.T. CONTINUOUS TO THE FTG. U.N.O.
 (PROVIDE (2) 2X4 WOOD BLOCKING BETWEEN TRUSS TOP
 AND BOTTOM CORDS BELOW)
- 12 AT DOORWAYS FOR INTERIOR BEARING WALLS, ADD A (2) 2x BLOCKING BETWEEN FLOOR LEVELS.
- 13 SEE ARCH DRAWINGS FOR SPECS ON LIGHTWEIGHT CONCRETE OVER WOOD JOIST TYP.
- 14 INDICATES MASONRY BEARING WALLS REINFORCED WITH (1)-#5 VERTICAL GROUT FILLED CELLS SOLID AT ALL CORNERS, ADJACENT MASONRY OPENINGS AND 24" O.C. (MAX).
- PROVIDE DOUBLE KNOCK OUT BLOCK BOND BEAM AT EA FLOOR LEVEL WITH #5 BARS AT EA COURSE.

PROVIDE PRECAST "U" LINTELS OVER ALL OPENINGS IN MASONRY WALL (COORD. EXACT SIZE, LOCATIONS AND ELEVATIONS WITH ARCH'L. DRAWINGS). IF NO LINTEL TYPE HAS BEEN CALL-OUT ON PLAN, PROVIDE MASONRY LINTEL TYPE: 8FI6-IB/IT, AND 8FI4-IB/IT FOR DOOR SEE "LINTEL SCHEDULE" (ON SHEET 95.05) FOR LINTEL TYPE SIZES AND REINFORCING. PROVIDE TEMPORARY SHORING DURING CONSTRUCTION IF LINTEL SPAN IS GREATER THAN 6 (SIX) FEET.



PERMIT REVIEW STAMP **ISSUE HISTORY** No. Date 11/22/19 SCHEMATIC DESIGN 2 12/06/19 DESIGN DEVELOPMENT 3 02/28/20 PERMIT REVIEW SET **REVISION HISTORY** Date Description FUGLEBERG KOCH 2555 Temple Trail, Winter Park, FL 32789 (407) 629-0595 www.fuglebergkoch.com CONSULTANT ASE ENGINEERING SERVICES, INC. 10244 East Colonial Drive, Suite 202 Orlando, Florida 32817 — 407—677—5565 Fax 407—730—2999 Certificate of Authorization No. 25873 THE ROBER Project #: FT. MYERS, FL **BUILDING TYPE 2** 3RD FLOOR FRAMING PLAN

3RD FLOOR PLAN

5CALE: 1/8" = 1'-0"

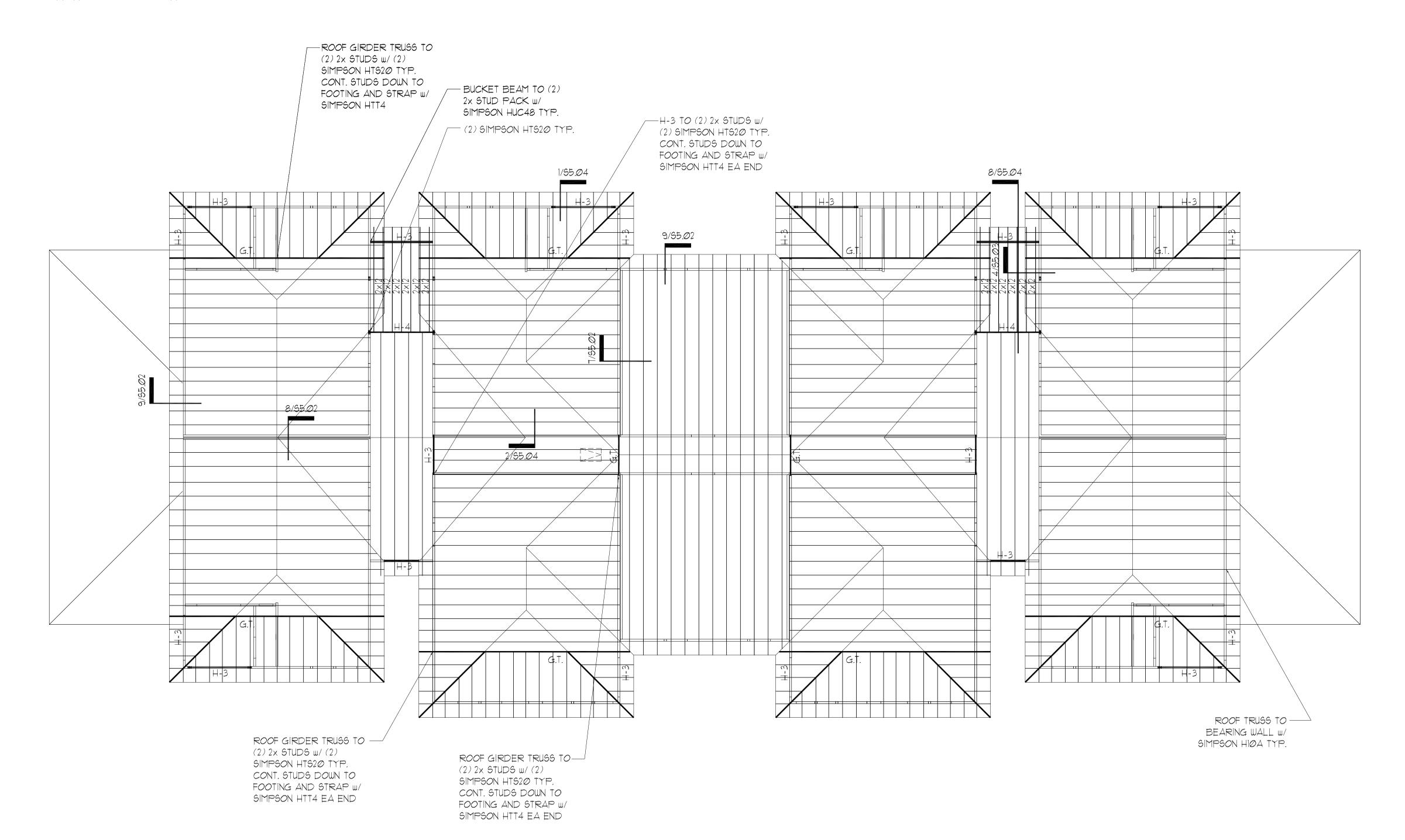
S2.03

12-09-19



- 1 SEE GENERAL NOTES ON SHEET SØ.Ø1 & SØ.Ø2.
- DO NOT SCALE DRAWINGS. SEE ARCH'L.
 DRAWINGS FOR ADDITIONAL DIMENSIONS
 NOT SHOWN, VERIFY ALL DIMENSIONS WITH
 ARCH'L. DRAWINGS PRIOR TO START OF
 CONSTRUCTION. IF DISCREPANCIES SHOULD
 OCCUR CONTACT THE ARCHITECT IN WRITING
 FOR CLARIFICATION BEFORE PROCEEDING.
- 3 SEE ARCH'L. DRAWINGS FOR LOCATIONS/LIMITS
 AND CONSTRUCTION INFORMATION OF INTERIOR
 NON-BEARING PARTITION WALLS NOT SHOWN ON
 PLAN. SEE GENERAL NOTES (SHEET SØ.ØI) FOR
 ADDITIONAL WALL FRAMING INFORMATION.
- PROVIDE WOOD HEADER OVER ALL OPENINGS
 IN WOOD WALLS (COORD. EXACT SIZE, LOCATION
 AND ELEVATIONS WITH ARCH'L. DRAWINGS) IF NO
 HEADER TYPE HAS BEEN CALL-OUT ON PLAN,
 PROVIDE WOOD HEADER BASED ON HEADER
 SCHEDULE SHOWN ON SO.OI SHEET.

- 5 PRE-ENGINEERED WOOD TRUSSES & MISCELLANEOUS WOOD FRAMING SPACED AT 24"O.C.(MAX).
- 6 O.B. INDICATES OVER-BUILT PRE-ENG. TRUSS.
-] G.T. INDICATES GIRDER TRUSS.
- THE REQUIRED NUMBER OF FASTENERS FOR CONNECTORS SUCH AS NAILS TO WOOD MEMBERS, SEE SIMPSON CATALOG.
- 9 SEE THE GENERAL NOTES FOR ROOF SHEATHING INFORMATION, AND FOR WOOD TRUSS TYPES.
- MODER TO DESIGN TRUSS TO INCORPORATE LOADS FROM MECHANICAL UNITS.
- PROVIDE DOUBLE WOOD STUDS UNDER WOOD HEADER, WOOD BEAM, F.G.T., G.T. TO THE FTG. U.N.O.





PERMIT REVIEW STAMP **ISSUE HISTORY** No. Date 1 11/22/19 SCHEMATIC DESIGN 2 12/06/19 DESIGN DEVELOPMENT 3 02/28/20 PERMIT REVIEW SET **REVISION HISTORY** Date Description



CONSULTANT

2555 Temple Trail, Winter Park, FL 32789 (407) 629-0595 www. fuglebergkoch.com

ASE ENGINEERING SERVICES, INC.

STRUCTURAL DESIGN GROUP

TO THE BEST OF MY KNOWLEDGE, THE BUILDING DESIGN PLANS AND SPECIFICATIONS COMPLY WITH BUILDING STRUCTURAL DESIGN CODE. THE SIGNING AND SEALING OF THE PLAND SPECIFICATIONS ARE ONLY FOR THE BUILDING'S STRUCTURAL COMPONENTS AFFECTED BY WIND, LIVE AND GRAVITY LOADS.

10244 East Colonial Drive, Suite 202

Orlando, Florida 32817 — 407-677-5565 Fax 407-730-2999

Certificate of Authorization No. 25873

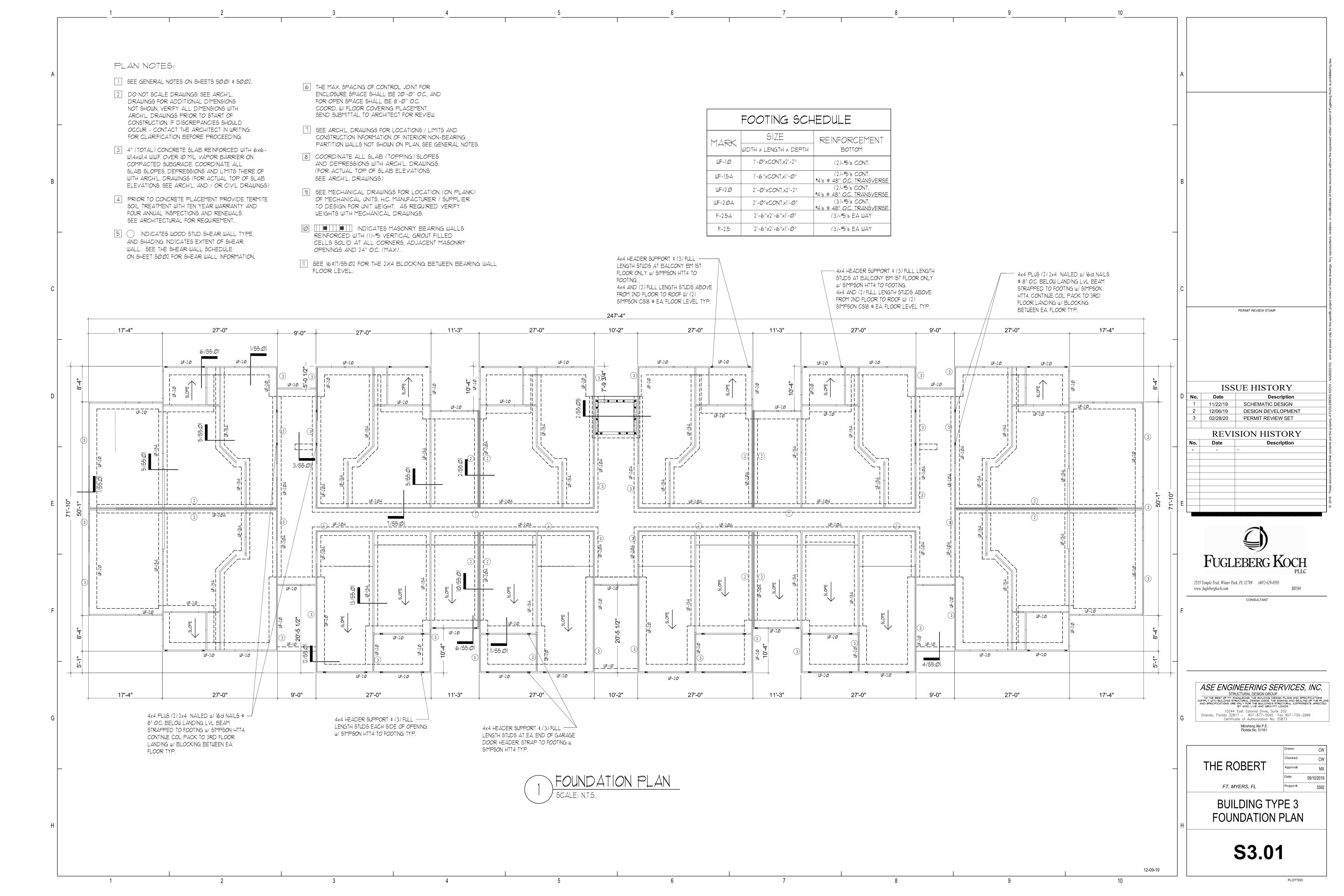
Minsheng Xie P.E.
Florida No. 51161

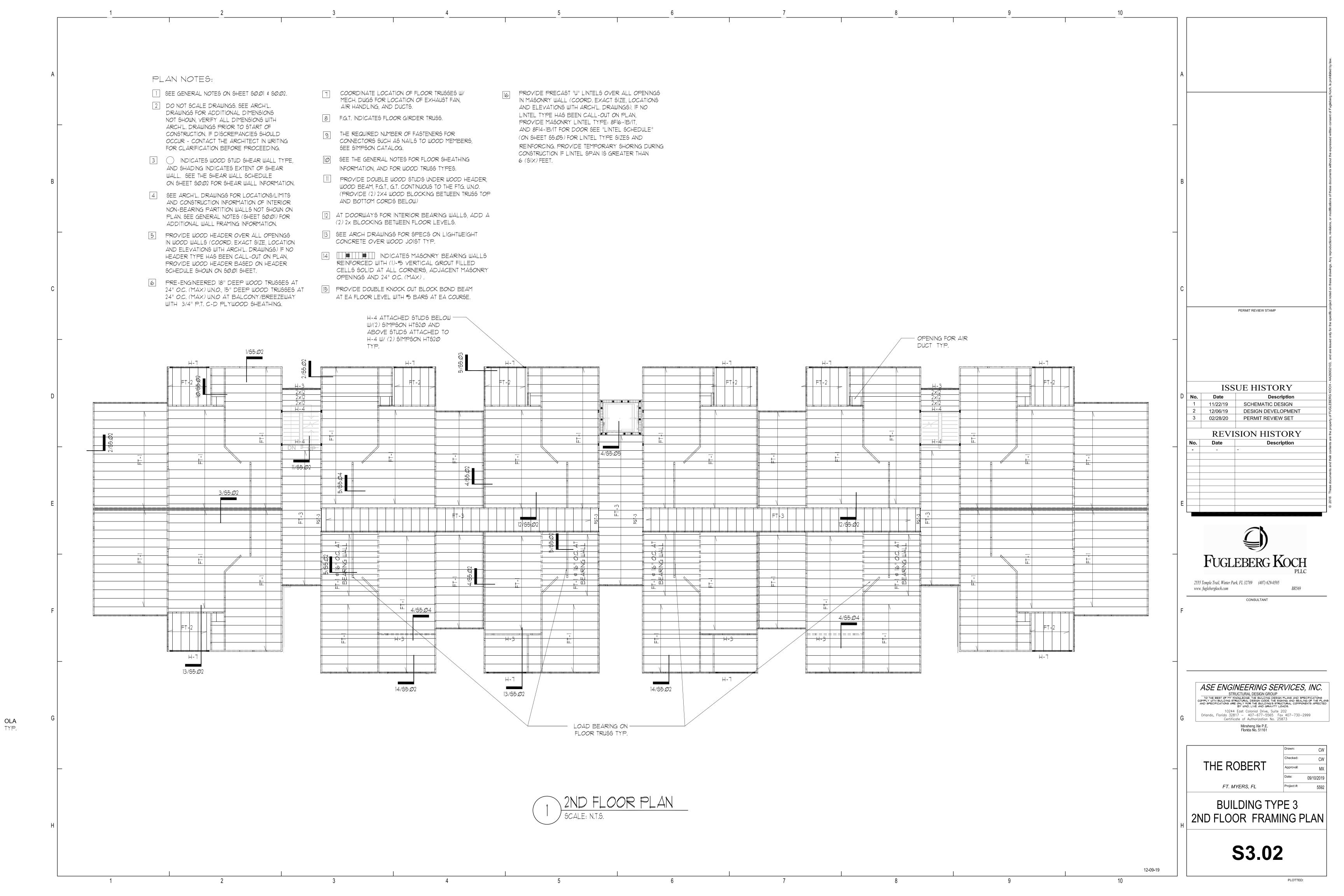
THE ROBERT

BUILDING TYPE 2 ROOF FRAMING PLAN

S2.04

12-09-19



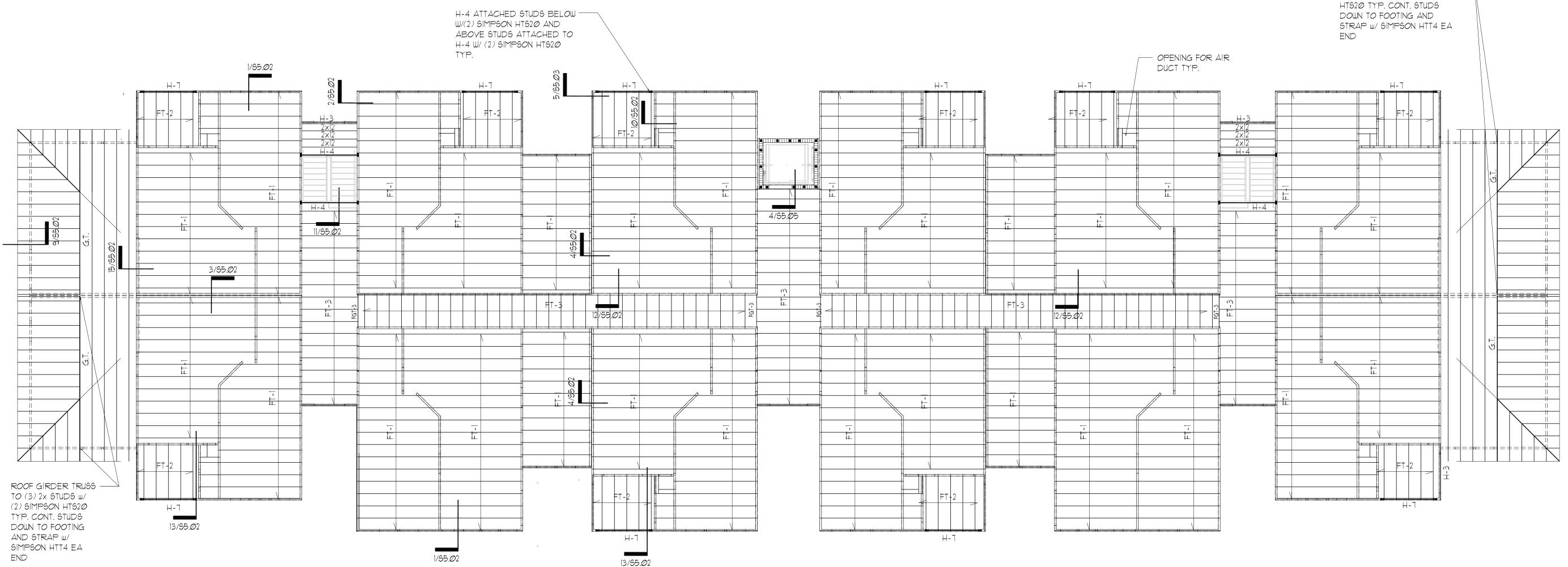




- SEE GENERAL NOTES ON SHEET SO.01 & SO.02.
- DO NOT SCALE DRAWINGS. SEE ARCH'L.
 DRAWINGS FOR ADDITIONAL DIMENSIONS
 NOT SHOWN, VERIFY ALL DIMENSIONS WITH
 ARCH'L. DRAWINGS PRIOR TO START OF
 CONSTRUCTION. IF DISCREPANCIES SHOULD
 OCCUR CONTACT THE ARCHITECT IN WRITING
 FOR CLARIFICATION BEFORE PROCEEDING.
- 3 INDICATES WOOD STUD SHEAR WALL TYPE,
 AND SHADING INDICATES EXTENT OF SHEAR
 WALL. SEE THE SHEAR WALL SCHEDULE
 ON SHEET SO.02 FOR SHEAR WALL INFORMATION.
- SEE ARCH'L. DRAWINGS FOR LOCATIONS/LIMITS
 AND CONSTRUCTION INFORMATION OF INTERIOR
 NON-BEARING PARTITION WALLS NOT SHOWN ON
 PLAN. SEE GENERAL NOTES (SHEET SØ.Ø1) FOR
 ADDITIONAL WALL FRAMING INFORMATION.
- PROVIDE WOOD HEADER OVER ALL OPENINGS
 IN WOOD WALLS (COORD. EXACT SIZE, LOCATION
 AND ELEVATIONS WITH ARCH'L. DRAWINGS) IF NO
 HEADER TYPE HAS BEEN CALL-OUT ON PLAN,
 PROVIDE WOOD HEADER BASED ON HEADER
 SCHEDULE SHOWN ON SOOI SHEET.
- PRE-ENGINEERED 18" DEEP WOOD TRUSSES AT 24" O.C. (MAX) U.N.O., 15" DEEP WOOD TRUSSES AT 24" O.C. (MAX) U.N.O AT BALCONY/BREEZEWAY WITH 3/4" P.T. C-D PLYWOOD SHEATHING.

- COORDINATE LOCATION OF FLOOR TRUSSES W/ MECH. DWGS FOR LOCATION OF EXHAUST FAN, AIR HANDLING, AND DUCTS.
- 8 F.G.T. INDICATES FLOOR GIRDER TRUSS.
- THE REQUIRED NUMBER OF FASTENERS FOR CONNECTORS SUCH AS NAILS TO WOOD MEMBERS, SEE SIMPSON CATALOG.
- SEE THE GENERAL NOTES FOR FLOOR SHEATHING INFORMATION, AND FOR WOOD TRUSS TYPES.
- PROVIDE DOUBLE WOOD STUDS UNDER WOOD HEADER, WOOD BEAM, F.G.T., G.T. CONTINUOUS TO THE FTG. U.N.O. (PROVIDE (2) 2X4 WOOD BLOCKING BETWEEN TRUSS TOP AND BOTTOM CORDS BELOW)
- 12 AT DOORWAYS FOR INTERIOR BEARING WALLS, ADD A (2) 2x BLOCKING BETWEEN FLOOR LEVELS.
- 13 SEE ARCH DRAWINGS FOR SPECS ON LIGHTWEIGHT CONCRETE OVER WOOD JOIST TYP.
- 14 INDICATES MASONRY BEARING WALLS
 REINFORCED WITH (1)-#5 VERTICAL GROUT FILLED
 CELLS SOLID AT ALL CORNERS, ADJACENT MASONRY
 OPENINGS AND 24" O.C. (MAX).
- PROVIDE DOUBLE KNOCK OUT BLOCK BOND BEAM AT EA FLOOR LEVEL WITH #5 BARS AT EA COURSE.

PROVIDE PRECAST "U" LINTELS OVER ALL OPENINGS IN MASONRY WALL (COORD. EXACT SIZE, LOCATIONS AND ELEVATIONS WITH ARCH'L. DRAWINGS). IF NO LINTEL TYPE HAS BEEN CALL-OUT ON PLAN, PROVIDE MASONRY LINTEL TYPE: 8FI6-IB/IT, AND 8FI4-IB/IT FOR DOOR SEE "LINTEL SCHEDULE" (ON SHEET \$5.05) FOR LINTEL TYPE SIZES AND REINFORCING. PROVIDE TEMPORARY SHORING DURING CONSTRUCTION IF LINTEL SPAN IS GREATER THAN 6 (SIX) FEET.



3RD FLOOR PLAN SCALE: N.T.S. D ISSUE HISTORY

D No. Date Description
1 11/22/19 SCHEMATIC DESIGN
2 12/06/19 DESIGN DEVELOPMENT
3 02/28/20 PERMIT REVIEW SET

REVISION HISTORY
No. Date Description
- - -

ASE ENGINEERING SERVICES, INC.

STRUCTURAL DESIGN GROUP

TO THE BEST OF THE PUBLICING DESIGN PLANS AND SPECIFICATIONS COMPLY WITH BUILDING STRUCTURAL DESIGN CODE. THE SIGNING AND SEALING OF THE PLANS AND SPECIFICATIONS ARE ONLY FOR THE BUILDING'S STRUCTURAL COMPONENTS AFFECTED BY WIND, LIVE AND GRAVITY LOADS.

10244 East Colonial Drive, Suite 202

Orlando, Florida 32817 - 407-677-5565 Fax 407-730-2999

Certificate of Authorization No. 25873

FUGLEBERG KOCH

CONSULTANT

2555 Temple Trail, Winter Park, FL 32789 (407) 629-0595

www.fuglebergkoch.com

THE ROBERT

FT. MYERS, FL

BUILDING TYPE 3 3RD FLOOR FRAMING PLAN

S3.03

12-09-19

ROOF GIRDER TRUSS TO (3) ---

2x STUDS w/ (2) SIMPSON

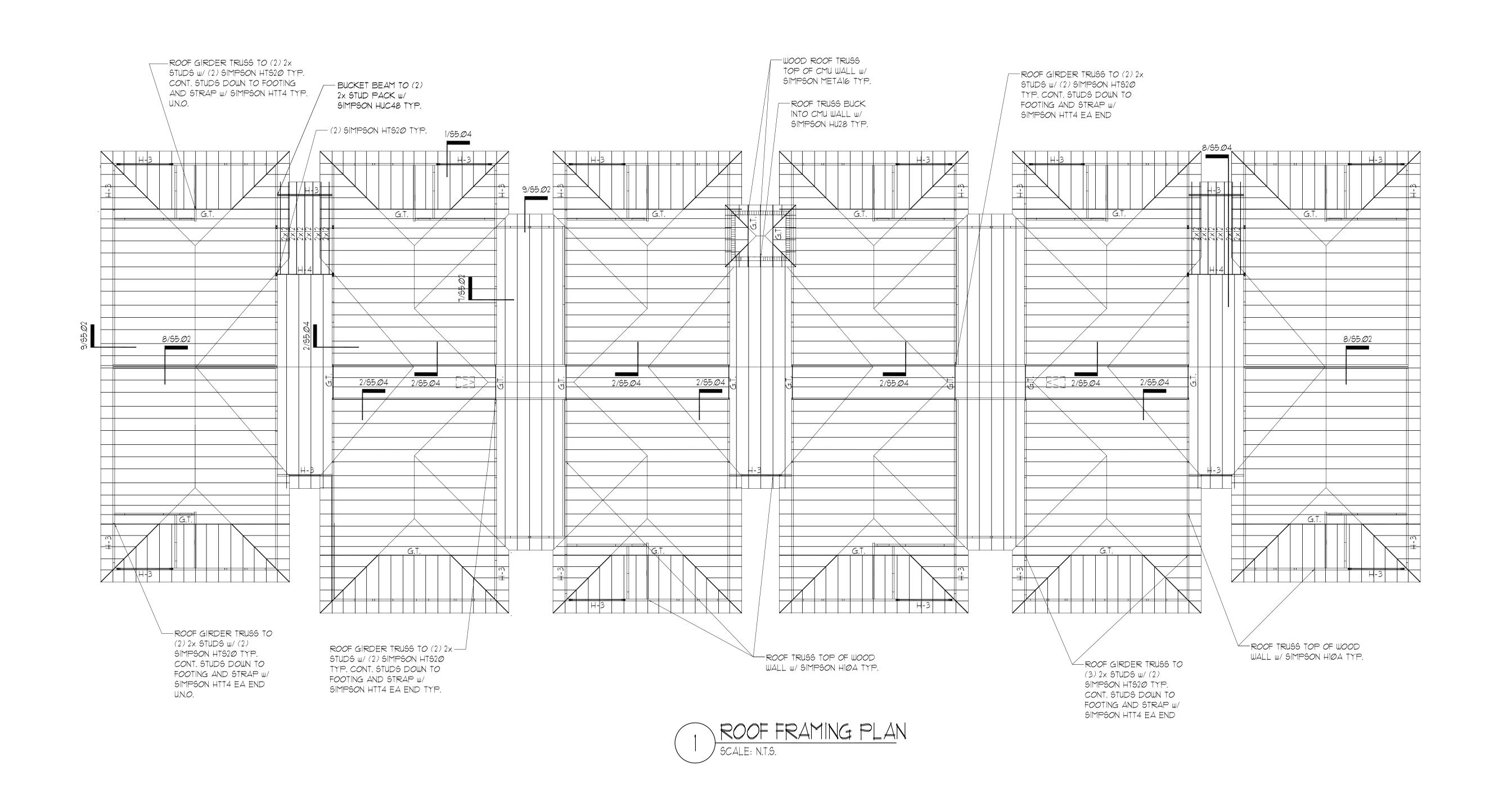
PLOTTED

Project #:



- SEE GENERAL NOTES ON SHEET SO.01 & SO.02.
- DO NOT SCALE DRAWINGS. SEE ARCH'L.
 DRAWINGS FOR ADDITIONAL DIMENSIONS
 NOT SHOWN, VERIFY ALL DIMENSIONS WITH
 ARCH'L. DRAWINGS PRIOR TO START OF
 CONSTRUCTION. IF DISCREPANCIES SHOULD
 OCCUR CONTACT THE ARCHITECT IN WRITING
 FOR CLARIFICATION BEFORE PROCEEDING.
- 3 SEE ARCH'L. DRAWINGS FOR LOCATIONS/LIMITS
 AND CONSTRUCTION INFORMATION OF INTERIOR
 NON-BEARING PARTITION WALLS NOT SHOWN ON
 PLAN. SEE GENERAL NOTES (SHEET SØ.Ø1) FOR
 ADDITIONAL WALL FRAMING INFORMATION.
- PROVIDE WOOD HEADER OVER ALL OPENINGS
 IN WOOD WALLS (COORD. EXACT SIZE, LOCATION
 AND ELEVATIONS WITH ARCH'L. DRAWINGS) IF NO
 HEADER TYPE HAS BEEN CALL-OUT ON PLAN,
 PROVIDE WOOD HEADER BASED ON HEADER
 SCHEDULE SHOWN ON SO.OI SHEET.

- 5 PRE-ENGINEERED WOOD TRUSSES & MISCELLANEOUS WOOD FRAMING SPACED AT 24"O.C.(MAX).
- 6 O.B. INDICATES OVER-BUILT PRE-ENG. TRUSS.
- G.T. INDICATES GIRDER TRUSS.
- THE REQUIRED NUMBER OF FASTENERS FOR CONNECTORS SUCH AS NAILS TO WOOD MEMBERS, SEE SIMPSON CATALOG.
- 9 SEE THE GENERAL NOTES FOR ROOF SHEATHING INFORMATION, AND FOR WOOD TRUSS TYPES.
- TRUSS ENGINEER TO DESIGN TRUSS TO INCORPORATE LOADS FROM MECHANICAL UNITS.
- PROVIDE DOUBLE WOOD STUDS UNDER WOOD HEADER, WOOD BEAM, F.G.T., G.T. TO THE FT.G. U.N.O.



PERMIT REVIEW STAMP **ISSUE HISTORY** No. Date 11/22/19 SCHEMATIC DESIGN 2 12/06/19 DESIGN DEVELOPMENT 3 02/28/20 PERMIT REVIEW SET REVISION HISTORY Date Description FUGLEBERG KOCH 2555 Temple Trail, Winter Park, FL 32789 (407) 629-0595 www.fuglebergkoch.com CONSULTANT

ASE ENGINEERING SERVICES, INC.

STRUCTURAL DESIGN GROUP

TO THE BEST OF MY KNOWLEDGE, THE BUILDING DESIGN PLANS AND SPECIFICATIONS COMPLY WITH BUILDING STRUCTURAL DESIGN CODE. THE SIGNING AND SEALING OF THE PLANS AND SPECIFICATIONS AFFECTED BY WIND, LIVE AND GRAVITY LOADS.

10244 East Colonial Drive, Suite 202

Orlando, Florida 32817 — 407-677-5565 Fax 407-730-2999

Certificate of Authorization No. 25873

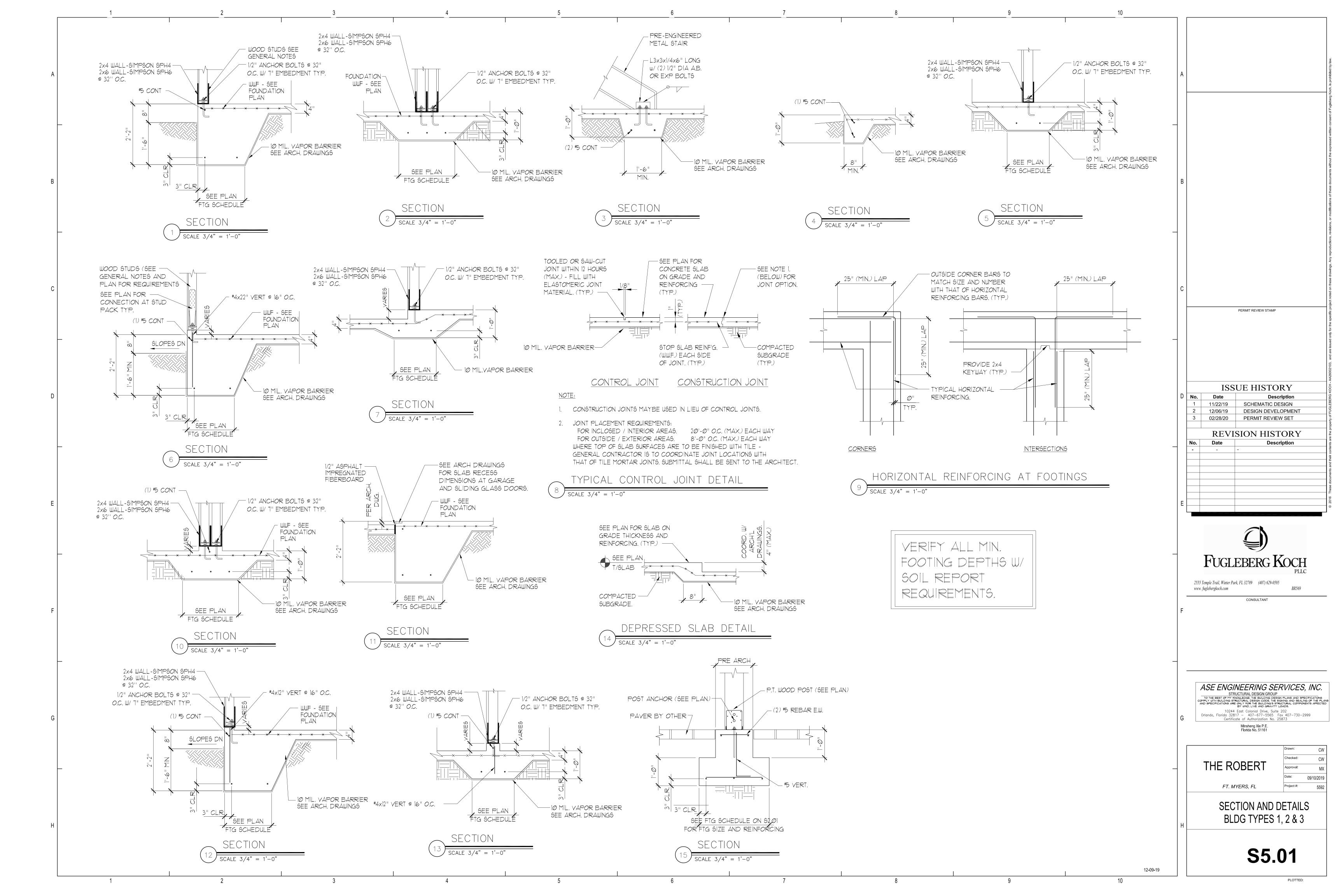
THE ROBERT

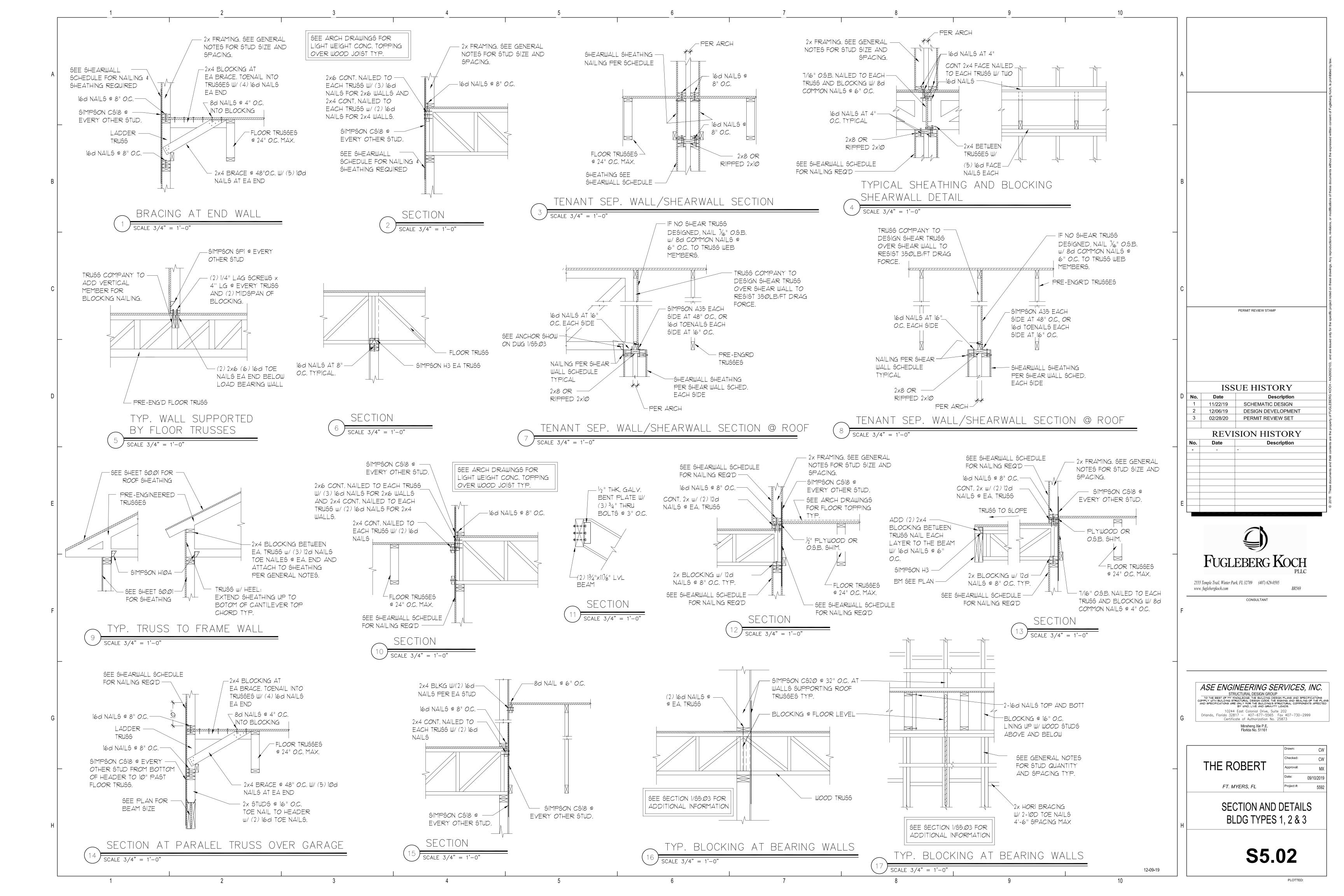
BUILDING TYPE 3 ROOF FRAMING PLAN

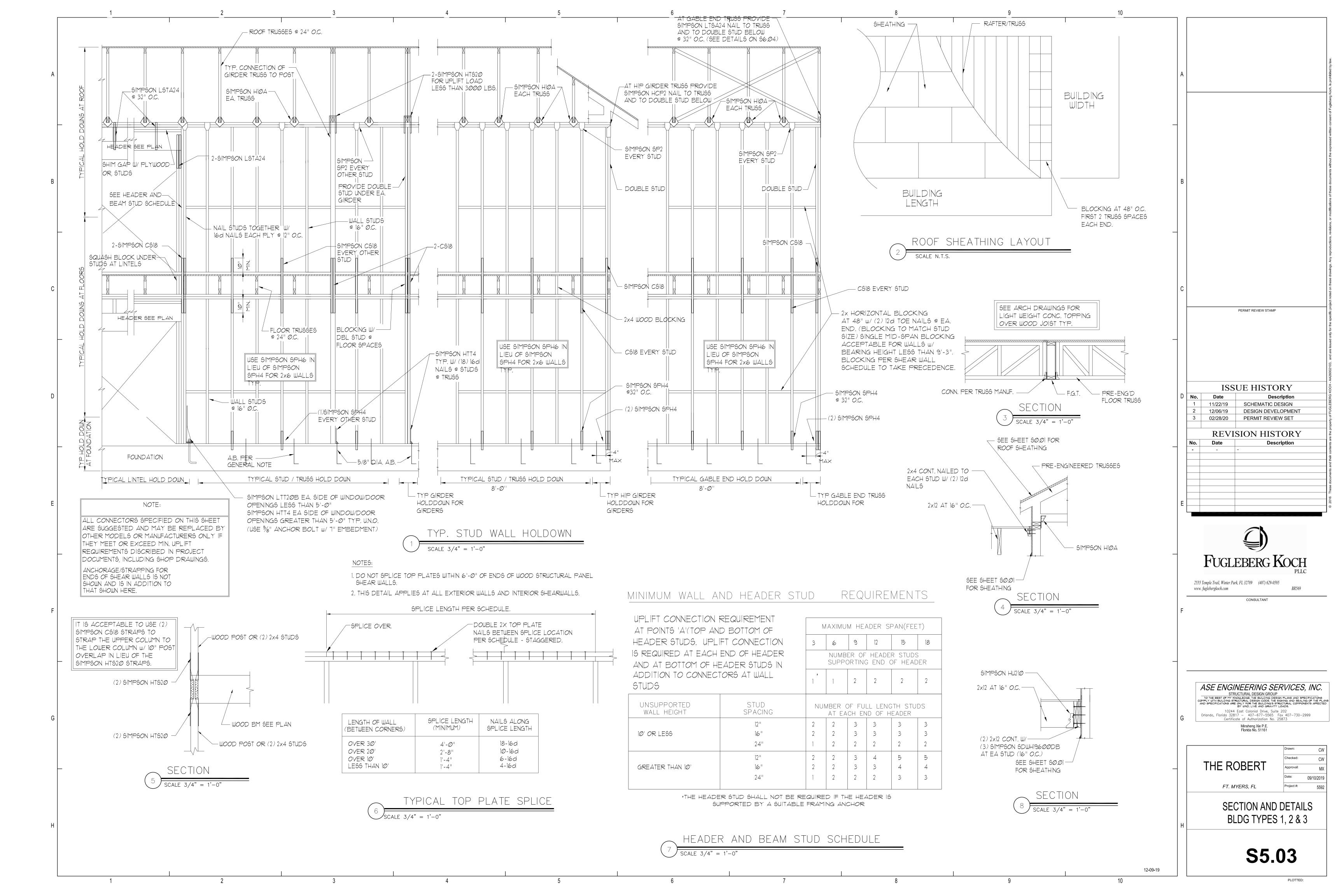
S3.04

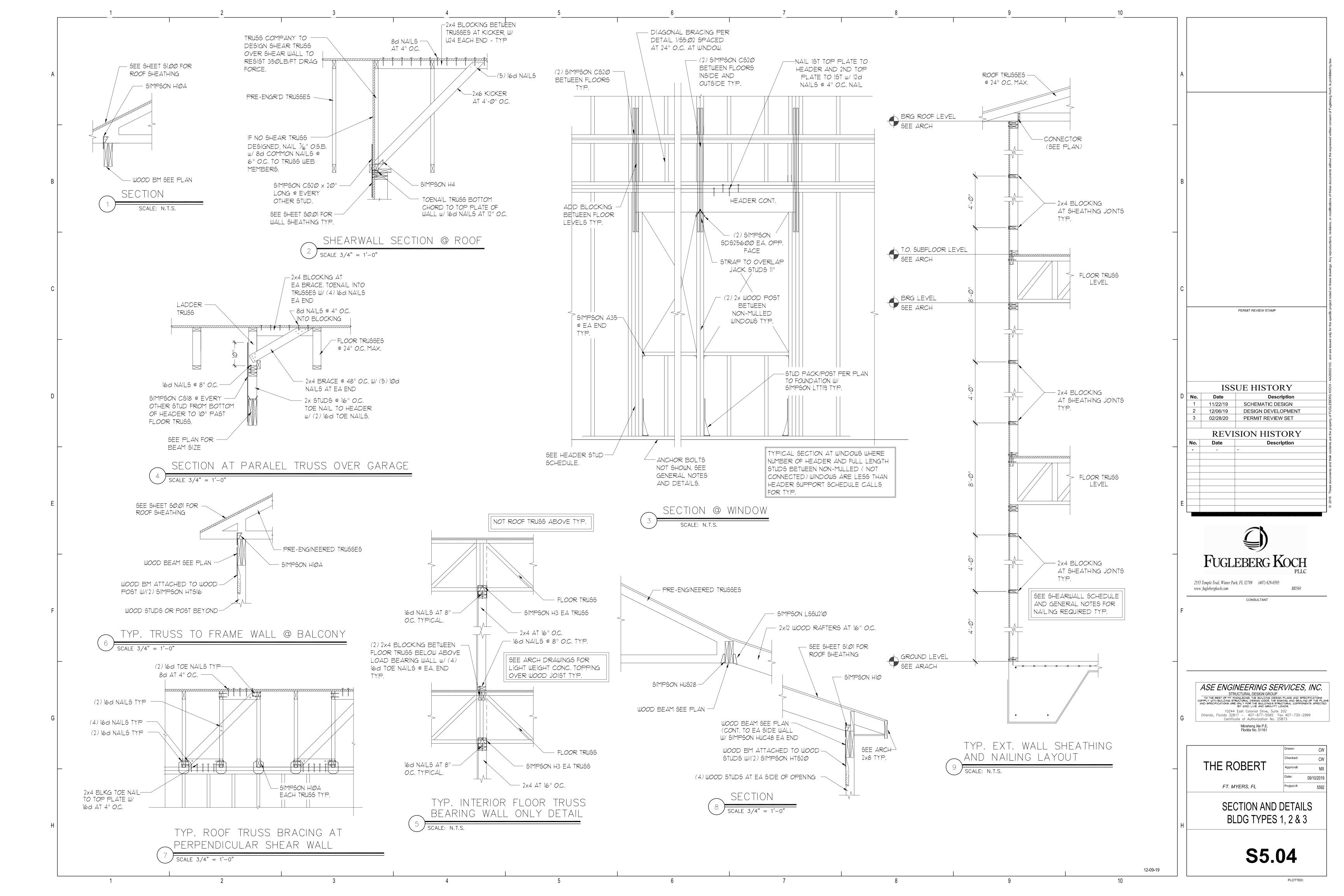
12-09-19

PLOTTED:



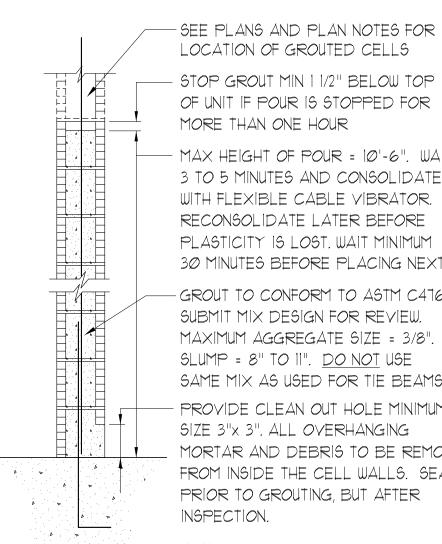






MASONRY WALL CONSTRUCTION

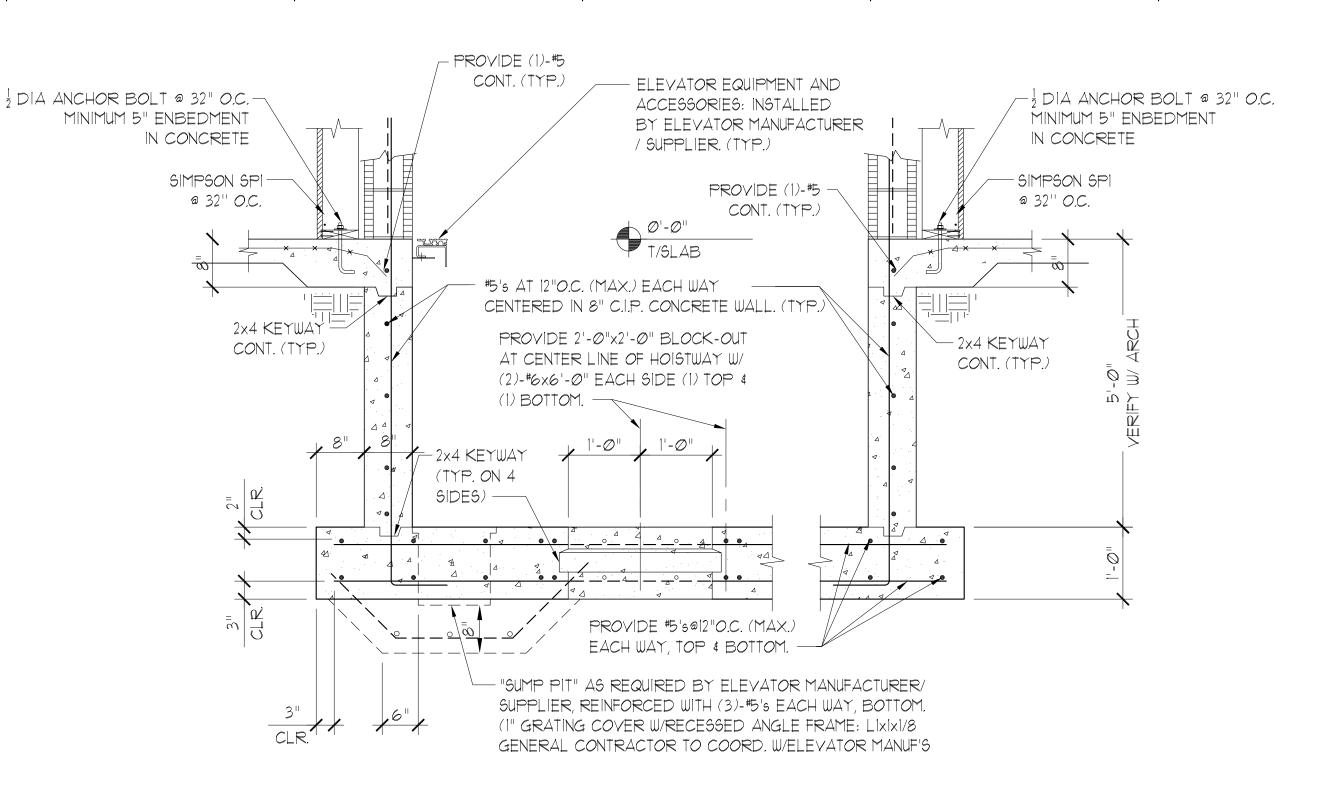
- A. HOLLOW LOAD BEARING UNITS SHALL BE NORMAL WEIGHT, GRADE N, TYPE 2, CONFORMING TO ASTM C90, WITH A MINIMUM NET COMPRESSIVE STRENGTH OF 1900 PSI (f'm = 1500 PSI).
- B. MORTAR SHALL BE TYPE M OR S, CONFORMING TO ASTM C270.
- C. COURSE GROUT SHALL CONFORM TO ASTM C476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI
- D. VERTICAL REINFORCEMENT SHALL BE AS NOTED ON THE DRAWINGS WITH CELLS FILLED WITH COARSE GROUT.
- E. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM AND AT A MAXIMUM SPACING OF 4'-0". REINFORCEMENT SHALL BE PLACED IN THE CENTER OF THE MASONRY CELL WITH DURO-WAL BAR POSITIONER D/A 811 TYPICAL UNLESS OTHERWISE NOTED. SEE TYPICAL GROUTING DETAILS FOR ADDITIONAL INFORMATION.
- F. REINFORCING STEEL SHALL BE LAPPED MINIMUM 48 BAR DIAMETERS WHERE SPLICED UNLESS NOTED OTHERWISE ON THE DRAWINGS.
- G. HORIZONTAL WALL REINFORCEMENT SHALL BE STANDARD LADDER TYPE DUR-0-WAL AT 16" O.C., UNLESS SHOWN OTHERWISE ON THE DRAWINGS.
- H. SPLICED WIRE REINFORCEMENT SHALL BE LAPPED AT LEAST 6" AND CONTAIN AT LEAST ONE CROSS WIRE OF EACH PIECE OF REINFORCEMENT WITHIN THE 6". LAP WITH STANDARD 'T' AND 'L' SHAPED PIECES AT INTERSECTIONS AND CORNERS.
- I. FOUNDATION DOWELS SHALL BE POSITIONED SUCH THAT IT IS ALIGNED WITH THE CENTER OF THE MASONRY UNIT'S VERTICAL CORE AND GROUTED SOLID. SHOULD THE FOUNDATION DOWEL NOT ALIGN WITH THE MASONRY UNIT'S VERTICAL CORE. NOTIFY THE ARCHITECT / STRUCTURAL ENGINEER FOR DIRECTION ON HOW TO CORRECT THE MIS ALIGNMENT.
- J. PROVIDE PRECAST CONCRETE LINTELS OVER ALL OPENINGS UNLESS NOTED OTHERWISE ON DRAWINGS. LINTELS SHALL BE OF SUFFICIENT SIZE AND REINFORCEMENT FOR THE GIVEN SPANS AND LOADING CONDITIONS. SUBMIT SHOP DRAWINGS WITH RATED LOAD CAPACITIES TO THE ARCHITECT FOR REVIEW.
- K. PROVIDE A KNOCK OUT BLOCK OR U-BLOCK REINFORCED WITH (1)-#5 CONTINUOUS AT THE SILL OF ALL WINDOW OPENINGS. EXTEND 8" BEYOND EACH SIDE OF THE OPENING TYPICALLY.



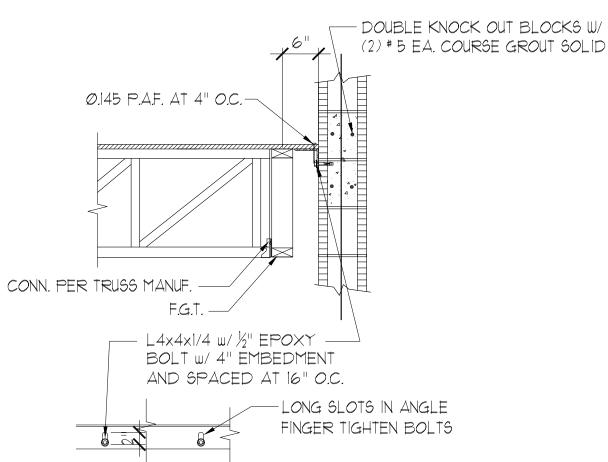
- STOP GROUT MIN 1 1/2" BELOW TOP OF UNIT IF POUR IS STOPPED FOR MORE THAN ONE HOUR - MAX HEIGHT OF POUR = 10'-6". WAIT 3 TO 5 MINUTES AND CONSOLIDATE WITH FLEXIBLE CABLE VIBRATOR. RECONSOLIDATE LATER BEFORE PLASTICITY IS LOST, WAIT MINIMUM 30 MINUTES BEFORE PLACING NEXT LIFT. -GROUT TO CONFORM TO ASTM C476. SUBMIT MIX DESIGN FOR REVIEW. MAXIMUM AGGREGATE SIZE = 3/8". SLUMP = 8" TO 11". DO NOT USE SAME MIX AS USED FOR TIE BEAMS. - PROVIDE CLEAN OUT HOLE MINIMUM SIZE 3"x 3". ALL OVERHANGING MORTAR AND DEBRIS TO BE REMOVED FROM INSIDE THE CELL WALLS. SEAL PRIOR TO GROUTING, BUT AFTER INSPECTION.

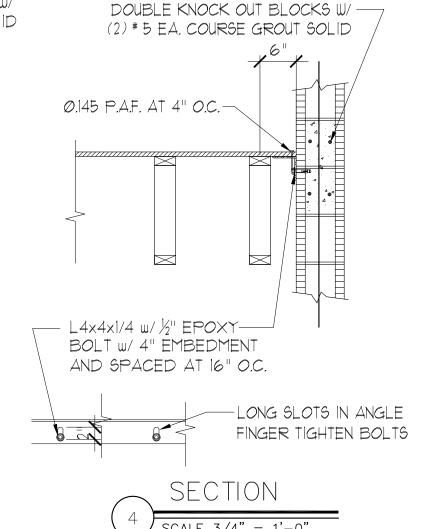
DO NOT GROUT UNTIL MORTAR HAS SET SUFFICIENTLY TO WITHSTAND THE PRESSURE OF THE GROUT, WAIT NOT LESS THAN 24 HOURS.

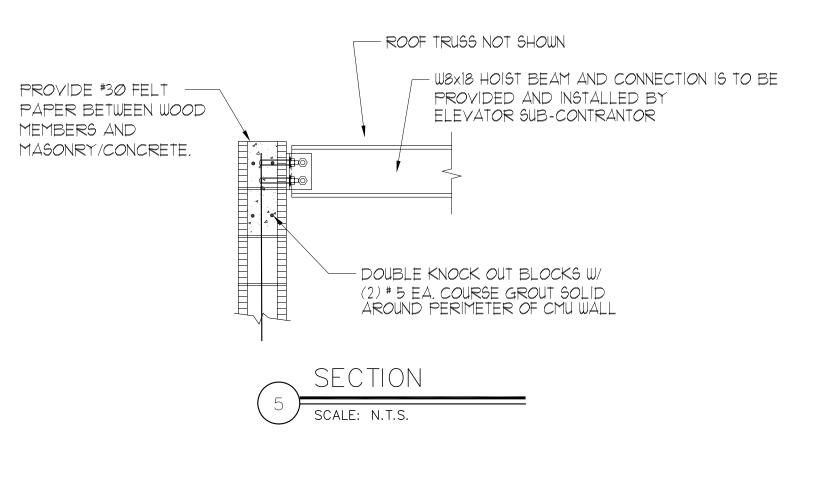
GROUTING SECTION TYPICAL HIGH LIFT



ELEVATOR PIT DETAIL







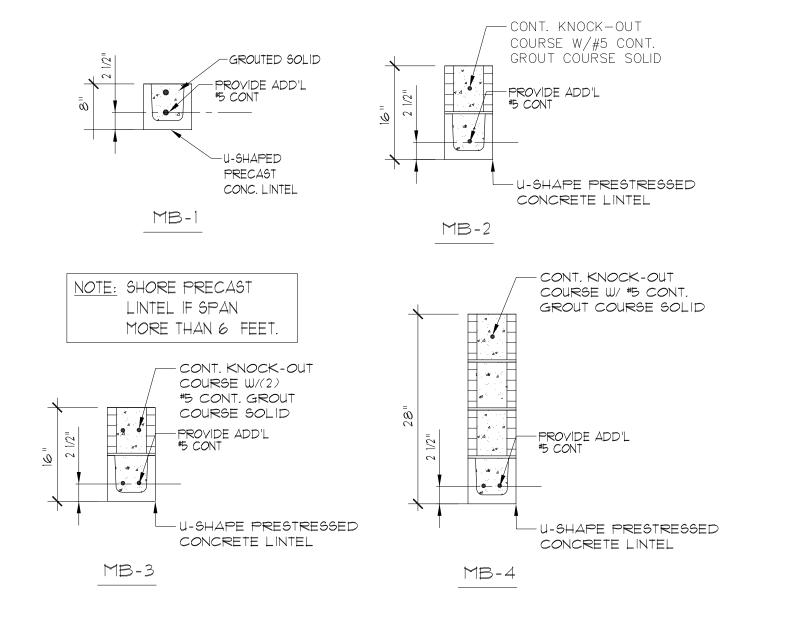
MASONRY LINTEL SCHEDULE (CAST-CRETE LINTEL)

MARK	LENGTH (L)	CAST-CRETE MARK	REMARK				
MB-1	2' -10"\$ L \$ 5"-10"	8F16-ØB/IT PRECAST					
MB-2	5' -10"\$ L <u>\$</u> 14'-0"	8F16-1B/IT PRECAST					
MB-3	14' -∅"⊈ L ⊈ 21'-4"	8F16 - IB/IT PRESTRESSED					
MB-4	14' -0"\$ L \$ 21'-4"	8F28-2B/IT PRESTRESSED					

- 1. PROVIDE MASONRY LINTEL OVER ALL OPENINGS. IF NO LINTEL IS SPECIFIED, PROVIDE MB-2.
- 2. PROVIDE MINIMUM END BEARING OF 8". CUT OUT BOTTOM OF LINTEL AT END TO ALLOW CONTINUATION OF FILLED CELL REINFORCING.
- 3. MASONRY LINTEL SUBSTITUTIONS MUST BE APPROVED BY "ADVANCED STRUCTURAL ENGINEERING" PRIOR TO INSTALLATION.

NOTE: SHORE PRECAST "U" LINTELS PER MANUFACTURER'S RECOMMENDATIONS.

12-09-19



MASONRY LINTEL SCHEDULI SCALE: N.T.S.

02/28/20 PERMIT REVIEW SET REVISION HISTORY Description FUGLEBERG KOCH

PERMIT REVIEW STAMP

ISSUE HISTORY

11/22/19 SCHEMATIC DESIGN

12/06/19 DESIGN DEVELOPMENT

2555 Temple Trail, Winter Park, FL 32789 (407) 629-0595 www. fuglebergkoch.com

CONSULTANT

ASE ENGINEERING SERVICES, INC. STRUCTURAL DESIGN GROUP

TO THE BEST OF MY KNOWLEDGE, THE BUILDING DESIGN PLANS AND SPECIFICATIONS

COMPLY WITH BUILDING STRUCTURAL DESIGN CODE. THE SIGNING AND SEALING OF THE PLANS

AND SPECIFICATIONS ARE ONLY FOR THE BUILDINGS STRUCTURAL COMPONENTS AFFECTED

BY WIND, LIVE AND GRAVITY LOADS. 10244 East Colonial Drive, Suite 202 Orlando, Florida 32817 - 407-677-5565 Fax 407-730-2999 Certificate of Authorization No. 25873

THE ROBERT Approval: FT. MYERS, FL Project #

> SECTION AND DETAILS BLDG TYPES 1, 2 & 3

> > **S5.05**

PLOTTED:

GENERAL NOTES

1. IT IS THE CONTRACTORS SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENT PARTS DURING ERECTION. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF WHATEVER TEMPORARY BRACING, GUYS OR TIE-DOWNS MAY BE NECESSARY.

CODES AND SPECIFICATIONS

A. GENERAL BUILDING CODE:

1. FLORIDA BUILDING CODE 2017 (6TH EDITION)

B. CONCRETE:

1. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE (ACI 318-89) 2. SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301-84).

DESIGN LOADS

A. DESIGN LIVE LOADS

20 PSF ROOF....

B. WIND LOADS:

1) BASIC WIND SPEED = 156 MPH (ULTIMATE) 121 MPH (NOMINAL)

2) RISK CATAGORY II

3) WIND EXPOSURE B.

- 4) INTERNAL PRESSURE COEFFICIENT: Ø.18 FULLY ENCLOSED STRUCTURE 5) COMPONENTS AND CLADDING (ASD): +24.11 PSF AND -30.44 PSF FOR

DESIGN WIND PRESSURES.

FOUNDATIONS

- 1. FOUNDATION DESIGN IS BASED ON AN ALLOWABLE SOIL BEARING PRESSURE OF 2500 PSF. WHICH SHALL BE VERIFIED BY A REGISTERED GEOTECHNICAL ENGINEER PRIOR TO THE START OF WORK.
- 2. PLACE FOOTINGS/SLAB ON COMPACTED SOIL. FOLLOW RECOMMENDATIONS OF SOILS REPORT.
- 3. VERIFY BEARING CAPACITY BY TESTING PRIOR TO SLAB PLACEMENT, SUBMIT REPORTS TO ARCHITECT

CAST-IN-PLACE CONCRETE

- I. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS, HAVE A SLUMP OF 4" PLUS OR MINUS I", AND HAVE 2-4% AIR ENTRAINMENT.
- 2. ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A-615 GRADE 60.
- 3. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH "THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" ACI 318-99.
- 4. ALL REINFORCING DETAILS SHALL CONFORM TO "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" ACI 315-80.
- 5. CONTRACTOR SHALL VERIFY LOCATIONS OF ALL OPENINGS, SLEEVES, ANCHOR BOLTS, INSERTS, ETC., AS REQUIRED BY OTHER TRADES BEFORE CONCRETE IS PLACED.
- 6. CONTRACTOR SHALL PROVIDE SPACERS, CHAIRS, BOLSTERS, ETC., NECESSARY TO SUPPORT REINFORCING STEEL.
- 1. ALL SLABS SHALL BE POURED MONOLITHICALLY, EXCEPT FOR REQUIRED CONSTRUCTION JOINTS.
- 8. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:

3"-----CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH. 1-1/2"-----ALL OTHER CASES.

- 9. HORIZONTAL SLAB BARS SHALL BE BENT I'-6" AROUND CORNERS, OR PROVIDE CORNER BARS WITH A 2'-O" LAP ON EACH LEG.
- 10. TESTING LABORATORY SHALL SUBMIT ONE COPY OF ALL CONCRETE TEST REPORTS DIRECTLY TO THE POST TENSION ENGINEER ARCHITECT-SEE SPECS.

WOOD CONSTRUCTION

- 1. WOOD CONSTRUCTION SHALL CONFORM TO THE NDS "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION", LATEST EDITION.
- 2. ALL WALL STUDS AND MISC. WOOD FRAMING SHALL BE NO.2 SOUTHERN YELLOW PINE. STUDS FOR LOAD BEARING WALLS SHALL BE AS FOLLOWS:

EXTERIOR WALLS INTERIOR WALLS

2x6'S @ 16"Ø.C. (U.N.O) 2x6'S @ 16"O.C. (U.N.O)

ALL LOAD BEARING WALLS SHALL HAVE MIDSPAN HORIZONTAL BLOCKING SPACED AT 48" O.C. INSTALLED BEFORE WALLS ARE LOADED

ALL NON-LOAD BEARING PARTITIONS SHALL CONSIST OF 2x4 STUDS SPACED AT 24" O.C. 2x4 STUDS DO NOT NEED TO BE DOUBLED AT THE FIRST FLOOR FOR NON-LOAD BEARING WALLS.

- 3. PLACE A SINGLE PLATE AT THE BOTTOM AND A DOUBLE PLATE AT THE TOP OF ALL STUD WALLS, 2x SOLE PLATES AT THE EDGES OF SLABS SHALL BE ATTACHED TO THE SLAB WITH SIMPSON MAS MUDSILL ANCHORS (WITH 6 100 NAILS) AT 32" O.C. AT INTERIOR STUD WALLS PROVIDE EITHER HILTI ZF72 (WITH 2 7/8" LENGTH, 5/64" THICK WASHERS) POWDER DRIVEN FASTENERS AT 8" ON CENTER, OR 1/2" DIAMETER ANCHOR BOLTS WITH 6" EMBEDMENT, AT 32" ON CENTER. RED-HEAD FASTENERS OF EQUIVALENT SIZES MAY BE USED. ALL OTHER SUBSTITUTIONS
- MUST BE APPROVED BY ADVANCED STRUCTURAL ENGINEERING PRIOR TO INSTALLATION. SEE THE SHEAR WALL SCHEDULE BELOW FOR SPECIAL SOLE PLATE ATTACHMENT
- 4. STUDS SHALL BE PACKED AT ALL ANGLES, CORNERS, AROUND ALL OPENINGS AND AT SHEAR WALLS. SEE SCHEDULES, UL ASSEMBLIES AND ARCH. DETAILS FOR REQUIREMENTS
- 5. WOOD LINTELS OVER OPENINGS SHALL BE AS NOTED PER DETAIL 1/95.02 . NAIL MULTIPLE STUDS TOGETHER WITH 16d NAILS @ 12" O.C. EACH PLY.
- 6. WALL SHEATHING SHALL BE: (SEE SHEAR WALL SCHEDULE SI,02 FOR REQUIREMENTS AT SHEAR WALLS.)

AT INTERIOR WALLS PROVIDE 5/8" GYPSUM WALLBOARD (SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS) EACH SIDE OF STUDS, NAILED WITH 5d COOLER NAILS AT 1" O.C. (USE 6d COOLER NAILS FOR 5/8" WALLBOARD) AT ALL SUPPORTS. PROVIDE SOLID 2x BLOCKING AT ALL SHEET EDGES. BLOCKING IS NOT REQUIRED AT NON-LOAD BEARING PARTITIONS.

AT EXTERIOR WALLS SHEATH THE INTERIOR FACE OF WALLS WITH 5/8" GYPSUM WALLBOARD AS NOTED ABOVE FOR INTERIOR WALLS. SHEATH THE EXTERIOR FACE OF WALLS WITH 1/16" C-DX PLYWOOD (OR 1/16" O.S.B.), NAILED WITH 8d NAILS AT 6" O.C. AT ALL EDGE SUPPORTS, AND 8d NAILS AT 6" O.C. AT ALL INTERMEDIATE SUPPORTS. PROVIDE SOLID DOUBLE 2x BLOCKING AT ALL SHEET EDGES SHEARWALL SCHEDULE NOTE 5.

- 1. ALL WOOD IN CONTACT WITH CONCRETE OR EXPOSED TO WEATHER SHALL BE PRESSURE TREATED.
- 8. ROOF SHEATHING SHALL BE 15/32" C D PLYWOOD OR 15/32" O.S.B., NAILED WITH 8d NAILS AT 6" O.C. AT SUPPORTED EDGES, AND 8d NAILS AT 12" O.C. AT INTERMEDIATE SUPPORTS. PROVIDE ONE PLYWOOD CLIP PER SPAN BETWEEN SHEET EDGES. PROVIDE SOLID 2x BLOCKING BETWEEN SUPPORTS AT ALL HIPS, RIDGES, VALLEYS, AND CHANGES IN ROOF SLOPE.
- 9. ALL LUMBER SHALL BE STAMPED WITH THE GRADE MARK OF AN APPROVED GRADING AGENCY

10. NAILING SCHEDULE:

CONNECTION	COMMON NAIL	NUMBER OR SPACING
SOLE PLATE TO TRUSS OR BLOCKING	160	8" O.C.
STUD TO SOLE PLATE, TOE NAIL	8d	4
DOUBLE STUDS, FACE NAIL	160	24" O.C.
DOUBLE TOP PLATES, FACE NAIL	16d	16" O.C.
TOP PLATES LAPS AND INTERSECTIONS	16d	3
TRUSSES, LAPS OVER WALLS, FACE NAIL	16d	4
BUILT-UP CORNER STUDS	16d	12" O.C.
STUDS TO SOLE PLATE, END NAIL	16d	2

WHERE WOOD BEAMS/HEADERS ABUT WOOD COLUMNS, PROVIDE SIMPSON "HHUC" CONNECTORS WITH ALL NAILS SPECIFIED BY THE MANUFACTURER.

AT ALL WALLS SUPPORTING ROOF TRUSSES PROVIDE UPLIFT STRAPPING/CONNECTORS SHOWN IN THE TYPICAL WALL ELEVATION 1/55.02, OR ALTERNATE: HURRY-BOLT 11. SYSTEM OR EQUAL. - SUBMIT ENGINEERED SHOP DRAWINGS FOR REVIEW.

ALL LOAD BEARING WALLS SHALL HAVE MIDSPAN HORIZONTAL BLOCKING 12. SPACED AT 48" O.C. INSTALLED BEFORE WALLS ARE LOADED. ALL NON-LOAD BEARING PARTITIONS SHALL CONSIST OF 2x4 STUDS SPACED AT 24" O.C. 2x4 STUDS DO NOT NEED TO BE DOUBLED AT THE FIRST FLOOR FOR NON-LOAD BEARING WALLS.

PREFABRICATED WOOD TRUSSES

- 1. ALL PREFABRICATED WOOD TRUSSES SHALL BE SECURELY FASTENED AT EACH END TO THEIR SUPPORTING WALLS OR BEAMS AS SHOWN IN THE TYPICAL WALL ELEVATION 1/95.02
- 2. PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF THE "NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENERS" AS RECOMMENDED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.
- 3. TRUSS MEMBERS AND CONNECTIONS SHALL BE DESIGNED (WITH A MAXIMUM ALLOWABLE STRESS INCREASE FOR LOAD DURATION OF 25% FOR ROOF TRUSSES ONLY) TO WITHSTAND THE LIVE LOADS GIVEN IN THE NOTES AND TOTAL DEAD LOAD.
- 4. TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND DESIGN NOTES WITH A FLORIDA REGISTERED ENGINEER'S SEAL FOR APPROVAL BY THE ARCHITECT. DESIGN NOTES TO INCLUDE THE RATED LOAD CAPACITY OF THE CONNECTORS USED TO SECURE THE MEMBERS, CERTIFICATION OF THE CONNECTOR CAPACITIES AND MANUFACTURER'S LICENSE TO FABRICATE TRUSSES UTILIZING THE CONNECTOR SYSTEM PROPOSED.
- 5. THE CONTRACTOR SHALL APPROVE FABRICATION AND INSTALLATION DRAWINGS SHOWING SIZE, SHAPE AND LAYOUT PRIOR TO SUBMITTAL FOR REVIEW BY THE ARCHITECT AND BEFORE FABRICATION HAS BEGUN.
- 6. BRIDGING FOR PRE-ENGINEERED TRUSSES SHALL BE AS REQUIRED BY TRUSS MANUFACTURER, AND THE LOCAL BUILDING CODE, UNLESS NOTED ON PLANS. ALSO WHERE HEEL HEIGHT ON BOTTOM CHORD BEARING TRUSSES EXCEEDS II" PROVIDE CONTINUOUS 2x4 LET IN.

7. DESIGN LOADS - DEAD LOADS:

FLOOR TRUSSES BOTTOM CHORD 8 PSF TOP CHORD (APTS) 15 PSF BOTTOM CHORD 8 PSF OUTSIDE TRUSSES TOP CHORD 45 PSF

ROOF TRUSSES BOTTOM CHORD 10 PSF 10 PSF (SHINGLE) TOP CHORD TOP CHORD 12 PSF (TILE) TOP CHORD (AT OVERBUILT AREAS) 5 PSF ADDITIONAL

8. ALL SIMPSON TRUSS ANCHORS SHOWN ON DRAWINGS SHALL BE VERIFIED FOR LOADS SHOWN ON WOOD TRUSS DESIGN CALCULATIONS, THEREFORE, ANCHOR SIZES AND TYPES ARE SUBJECT TO CHANGE BY ADVANCED STRUCTURAL ENGINNERING II.

FASTENER SUBSTITUTIONS:

ALL NAILS ARE COMMON NAILS, UNLESS NOTED OTHERWISE. THE FOLLOWING FASTENERS ARE ACCEPTABLE SUBSTITUTIONS. THE ALTERNATE FASTENERS SHALL BE SPACED AT THE SAME SPACING AS THE SCHEDULED FASTENERS.

ALTERNATE FASTENER SCHEDULED FASTENER

8d RING SHANK NAIL 8d COMMON NAIL 8d SCREW SHANK NAIL

Ø.131 P-NAIL

100 RING SHANK NAIL 10d COMMON NAIL 100 SCREW SHANK NAIL

Ø.148 P-NAIL

160 RING SHANK NAIL 16d COMMON NAIL 16d SCREW SHANK NAIL

*6 x 1 1/4" TYPE S OR W DRYWALL SCREW 6d COOLER NAIL

WATERPROOFING FOR THIS BUILDING IS THE SOLE RESPONSIBILITY OF THE BUILDER/CONTRACTOR/ARCHITECT, HENCE NO DETAILS OF FLASHING, FLOOR, SLOPE AND STEP, NOR ANY OTHER WATERPROOFING MEASURES HAVE BEEN INCLUDED IN OUR DRAWINGS. IF SLOPE AND FLOOR STEPS ARE SHOWN ON OUR DRAWINGS, THEY SHALL BE USED ONLY AS A REFERENCE.

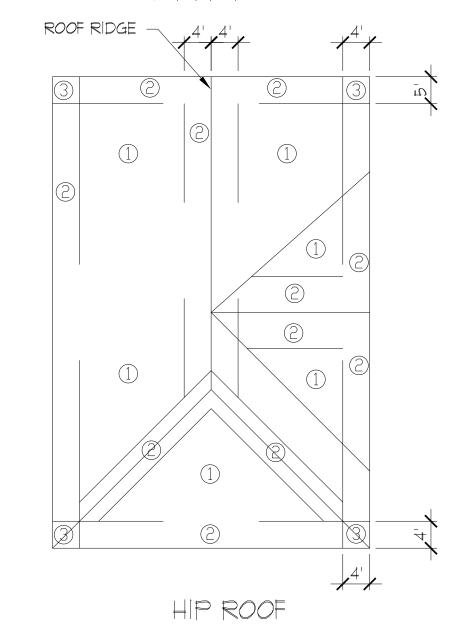
SHOP DRAWING SUBMITTALS

- A. ALL SHOP DRAWINGS SHALL BE REVIEWED AND STAMPED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL. ALL SUBMITTALS SHALL BE PROVIDED FOR THE ENGINEER REVIEW A MINIMUM OF TWO WEEKS PRIOR TO CONSTRUCTION OR REQUIRED DELIVERY OF MATERIALS. THE ENGINEER SHALL BE PROVIDED A MINIMUM OF (10) BUSINESS DAY TO REVIEW SUBMITTALS. THE CONTRACTOR SHALL MAKE NO CLAIMS FOR DELAY FOR SUBMITTALS NOT PROVIDED IN ACCORDANCE WITH THIS REQUIRED REVIEW PERIOD OR NOT OTHERWISE SUBMITTED IN A TIMELY MANNER.
- B. THE GENERAL CONTRACTOR SHALL SUBMIT FOR ENGINEER REVIEW SHOP
- DRAWINGS FOR THE FOLLOWING ITEMS:
- 1. STRUCTURAL STEEL
- 2. REINFORCING STEEL (*). 3. PRE-ENGINEERED WOOD TRUSSES (*).
- 4. CONCRETE MIX DESIGNS
- 5. POST-TENSION CABLE SHOP DRAWINGS WITH CALCULATIONS (*). 6. CMU WALL.

1. PRE-ENGINEERED STAIRS WITH CALCULATIONS.

- ITEMS MARKED (*) SHALL HAVE SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA. ITEMS MARKED (#) SHALL BE SUBMITTED FOR ENGINEERS RECORD ONLY.
- C. MANUFACTURER'S LITERATURE: SUBMIT TWO COPIES OF MANUFACTURER'S LITERATURE FOR ALL MATERIALS AND PRODUCTS USED IN CONSTRUCTION ON THE PROJECT.

ROOF FASTENING ZONES GABLE ROOF



ROOF SHEATHING FASTENING SCHEDULE NOTE: USE 8d RING SHANK NAILS IN ZONE

PANEL EDGES PANEL FIELD (1) NAILS 6" O.C. ① NAILS 6" O.C. (2) NAILS 6" O.C. ② NAILS 6" O.C. ③ NAILS 4" O.C. 3 NAILS 4" O.C.

WINDOW & DOOR WIND PRESSURES 156 MPH WIND ZONE EXPOSURE B BUILDING CATAGORY II. BELOW VALUES

OPENING SIZE	INTERIOR ZONE	EXTERIOR ZONE**
8' GARAGE DOOR	+25.31 / -28.64	+25.31 / -28.64
16' GARAGE DOOR	+24.20 / -26.97	+24.20 / -26.97
10 SQFT	+26.97 / -29.24	+26.97 / -36.10
20 SQFT	+25.77 / -28 <i>.</i> 04	+25.77 / -33.63
50 SQFT	+24.11 / -26.44	+24.11 / -3Ø.44
100 SQFT	+22.91 / -25.17	+22.91 / -28.04

STANDARD WINDOWS AND DOORS WILL GENERALLY FALL INTO THE 10 TO 20 SQ. FT. CATEGORY. STANDARD DOUBLE SLIDING GLASS DOOES WILL GENERALLY FALL INTO THE 20 TO 50 SQ. FT. CATEGORY, STANDARD DOUBLE GARAGE DOORS WILL GENERALLY

BASED ON ALLOWABLE STRESS DESIGN (ASD)

OPENING SIZE	INTERIOR ZONE	EXTERIOR ZONE**
8' GARAGE DOOR	+25.31 / -28.64	+25.31 / -28.64
16' GARAGE DOOR	+24.20 / -26.97	+24.20 / -26.97
10 SQFT	+26.97 / -29.24	+26.97 / -36.10
20 SQFT	+25.77 / -28.04	+25.77 / -33.63
50 SQFT	+24.11 / -26.44	+24.11 / -30.44
100 SQFT	+22.91 / -25.17	+22.91 / -28.04

**END ZONE IS DEFINED AS AN AREA WITHIN 8" FROM THE EDGE OF THE BUILDING

FALL INTO THE 50 TO 100 SQ. FT. CATEGORY

	HEADER/BEAM	SCHEI)ULE
TYPE	HEADER/BEAM	TYPE	HEADER/BEAM
H-1	TRIPLE 2x6	H-5	(2) 1 3/4" × 11 7/8" LVL
H-2	TRIPLE 2×8	H-6	(2) 1 3/4" × 9 1/4" LVL
H-3	TRIPLE 2×10	H-7	(3) 1 3/4" × 11 7/8" LVL
H-4	DOUBLE 2×12		

- I. PROVIDE WOOD HEADERS OVER ALL OPENINGS. IF NO HEADER IS SPECIFIED, PROVIDE H-3 AT WALLS SUPPORTING TRUSSES, AND H-2 AT OTHER WALLS.
- 2. AT DOUBLE OR TRIPLE 2x HEADER/BEAMS PROVIDE A 1/2" PLYWOOD (OR 0.5.B.) SPACER BETWEEN MEMBERS (EXCEPT LVL BEAM).
- 3. NAIL ALL MULTI-MEMBER HEADERS AND BEAMS TOGETHER WITH 16d NAILS AT 12" O.C. TOP AND BOTTOM, EACH SIDE, STAGGERED.

FUGLEBERG KOCH 2555 Temple Trail, Winter Park, FL 32789 (407) 629-0595 www. fuglebergkoch.com ASE ENGINEERING SERVICES, INC. STRUCTURAL DESIGN GROUP

TO THE BEST OF MY KNOWLEDGE, THE BUILDING DESIGN PLANS AND SPECIFICATIONS
COMPLY WITH BUILDING STRUCTURAL DESIGN CODE. THE SIGNING AND SEALING OF THE PL
AND SPECIFICATIONS ARE ONLY FOR THE BUILDING'S STRUCTURAL COMPONENTS AFFECTE
BY WIND, LIVE AND GRAVITY LOADS. 10244 East Colonial Drive, Suite 202 Orlando, Florida 32817 — 407—677—5565 Fax 407—730—2999 Certificate of Authorization No. 25873

PERMIT REVIEW STAM

ISSUE HISTORY

REVISION HISTORY

SCHEMATIC DESIGN

PERMIT REVIEW SET

DESIGN DEVELOPMENT

Description

No.

11/22/19

12/06/19

02/28/20

Date

THE ROBERT 09/10/19 FT. MYERS, FL **GENERAL NOTES**

CS1.01

			CONNECTOR SCHEDULI	Ε	
#	SIMPSON Or Equal	FASTENERS	UPLIFT(lbs.)	WOOD TO:	USP
	H10	(8) 8d	990	WOOD	N/A
2	HTS20	(24) 10d x 1-1/2	1450	WOOD	HTW20
3	LTS12	(12) 10d x 1-1/2	775	WOOD	LTW12
4	H3	(4) 8d	455	WOOD	RT7
5	LTT20B	(10) 16d INTO STUDS	1750 (NAILS)	CONCRETE, WOOD	LTS20B
6	HTT16	(18) 16d	4175 (NAILS)	CONCRETE, WOOD	HTT16
7	SP-1 SP-4	(10) 10d (6) 10d x 1-1/2	585 735	WOOD (BOTT OF STUD) WOOD (TOP OF STUD)	SPT22 SPT4
	THA/THAC TYP	PER SIMPSON CATALOG	PER SIMPSON CATALOG	SELECTION BASE ON TRUSS WIDTH AND HEIGHT	MSH TYP
8	CS16	(22) 10d	1650	11" MIN AT EA END FOR WOO.	D RS150
(D)	MAS	(6) 10d x 1-1/2	1005	SLAB, STEMWALL	FA3
10	HCP2 HCP4	(12) 10d x 1-1/2 (16) 10d	605 1000	FOR 2× MEMBER FOR 4× MEMBER	N/A N/A
	A35	(12) 8d x1 1/2	450 SHEAR	WOOD	MPA1
12	CC44, ECCU44 CC66, ECCU66	(4) 5/8d BOLTS (6) 5/8d BOLTS	1465 (UPLIFT) 3660 (UPLIFT)	ECCU44 AT BEAM END ECCU66 AT BEAM END	KCC44, KECCU44 KCC66, KECCU66
13	PC/EPC TYP	PER SIMPSON CATALOG	PER SIMPSON CATALOG	SELECTION BASE ON POST AND BEAM WIDTH	PCM/EPCM TYP
14	ECCLL44 ECCLL66	BOLTS AS REQ	1465 (UPLIFT) 3660 (UPLIFT)	ECCLLU44 FOR 4×4 POST ECCLLU66 FOR 6×6 POST	KECCLL44 KECCLL66
(15)	ABU44 ABU66	(12) 16d 5/8 ANCHOR BOLT	2200	ABU44 FOR 4x4 POST ABU44 FOR 4x4 POST	PAU44 PAU66
16	HUC410	(18) 16d FOR WOOD	1510	CONCRETE, MASONRY, WOOD FOR 4×10 BM	HD410IF
	HUC412	(22) 16d FOR WOOD	1510	CONCRETE, MASONRY, WOOD FOR 4×12 BM	HD412IF
(18)	LUS TYP	PER SIMPSON CATALOG	PER SIMPSON CATALOG	LUS24 FOR 2x6, LUS26 FOR 2x8, LUS28 FOR 2x10, ETC.	JUS TYP

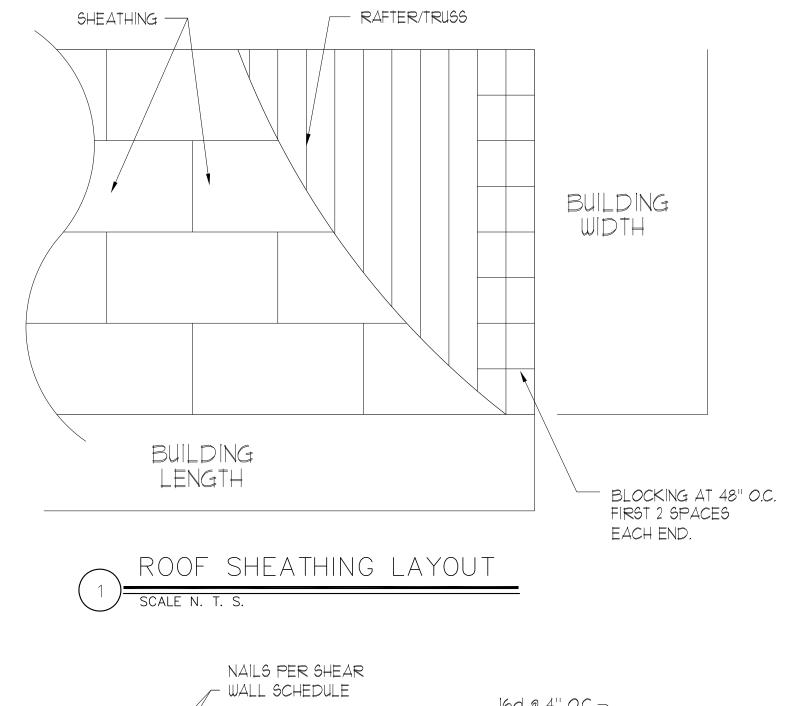
NOTE: USE TRUSS SHOP DRAWINGS TO DETERMINE UPLIFT REQUIREMENTS. PROVIDE TRUSS ANCHOR THAT PROVIDES AN UPLIFT CAPACITY EQUAL TO OR GREATER THAN THE UPLIFT INDICATED ON THE TRUSS SHOP DRAWINGS. INSTALL ALL TRUSS ANCHORS PER THE MANUFACTURER'S REQUIREMENTS.

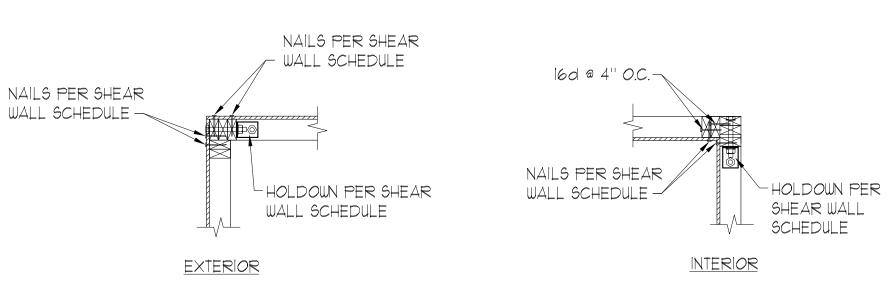
SHEARWALL SCHEDULE										
TYPE	SHEATHING	SHEATHING	NAILING	FND CONN EACH END OF WALL				FND SOLE PLATE		
	FND-ROOF	FND-ROOF		CONNECTOR	FND BOLT-12" EMBED DBL NUT EA END	ATTACH TO STUDS	REQ'D STUDS AT END OF WALL	ATTACHMENT		
	1/2" C-DX *1 PLYWOOD OR OSB	8d NAILS @ 4" D.C.		HD5B	3/4″ DIA	(2) 3/4″ DIA	(3) 2×6 OR WD POST	1/2" DIA× 7" EMB. A. BOLTS @ 32" O.C.		
(5)	5/8″ GYPSUM WALLBOARD	6d COOLER NAILS @ 7″ O.C.		HD5B	3/4″ DIA	(2) 3/4″ DIA	(2) 2x4	1/2" DIA× 7" EMB. A. BOLTS @ 32" O.C.		

*1 SHEATH EXTERIOR FACE WITH PLYWOOD SCHEDULED, AND THE INTERIOR FACE OF STUDS AS SPECIFIED IN THE GENERAL NOTES FOR INTERIOR WALLS.

NOTES:

- 1 ALL HD AND CS FASTENERS ARE AS MANUFACTURED BY THE SIMPSON STRONG-TIE COMPANY, INC, OR EQUAL.
- 2 WHERE SCHEDULED ADDITIONAL STUDS AT THE ENDS OF SHEAR WALLS ARE INTERRUPTED AT TRUSS BEARING, PROVIDE SOLID BLOCKING IN THE TRUSS SPACE TO MATCH THE STUDS SCHEDULED FOR THE LOWER FLOOR.
- 3 WHERE SCHEDULED SHEATHING CANNOT EXTEND FULL HEIGHT OF WALL, PROVIDE SHEATHING OR BLOCKING BETWEEN FLOOR TRUSSES AT ALL SHEARWALLS.
- 4 SHEATHING SHALL EXTEND FULL HEIGHT OF SHEARWALLS, STAGGER SHEATHING HORIZONTAL JOINTS SO THEY DO NOT FALL AT HORIZONTAL JOINT BETWEEN DOUBLE TOP PLATES.
- 5 WHEREVER THE SHEATHING OF A SHEAR WALL IS INTERRUPTED (I.E. BY AN INTERSECTING WALL) IN THE VERTICAL PLANE, THE SCHEDULED "ADD'L STUD AT END OF WALL" AND HOLDDOWN ANCHORS SHALL BE PROVIDED AT THE END OF THE SHEATHING (ONE EACH SIDE OF THE INTERRUPTION). THE QUANTITY OF SHEARWALL CALLOUTS ON THE PLANS MAY NOT ACCURATELY REFLECT THE NUMBER OF HOLDDOWNS REQUIRED BECAUSE OF THIS. THE CONTRACTOR MUST FIRST DETERMINE WHERE THE SHEARWALL SHEATHING WILL BE INTERRUPTED BEFORE DETERMINING THE NUMBER HOLDDOWNS REQUIRED.
- 6 WHERE BOLTS ARE CALLED OUT FOR HOLDDOWN ANCHORS, THE BOLTS SHALL BE THROUGH BOLTS CONFORMING TO ASTM A307.
- 7 7/16" D.S.B. MAY BE USED IN LIEU OF THE 1/2" C-DX PLYWOOD SHEATHING.
- 8 ANCHOR BOLTS SHALL ALL HAVE 3" HOOKS. A36 ALL THREAD RODS, DRILLED AND EPOXY GROUTED INTO FOOTINGS, MAY BE SUBSTITUTED FOR ANCHOR BOLTS CAST IN PLACE. ALL THREAD RODS SHALL HAVE SAME EMBEDMENT DEPTH AS ANCHOR BOLTS.





TYP. HOLDOWN ARRAGEMENTS AT CORNER

HAILS PER SHEAR

JALL SCHEDULE

SHEAR WALL
SCHEDULE

INTERIOR

MENTS AT CORNER

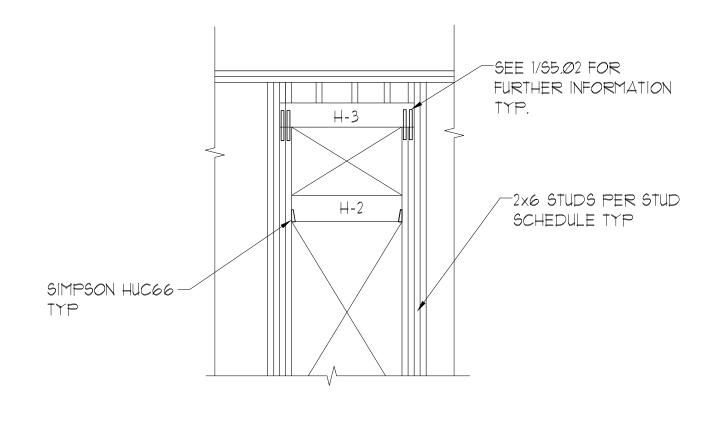
(2) IØd NAILS TOENAIL —

OVER FRAMING

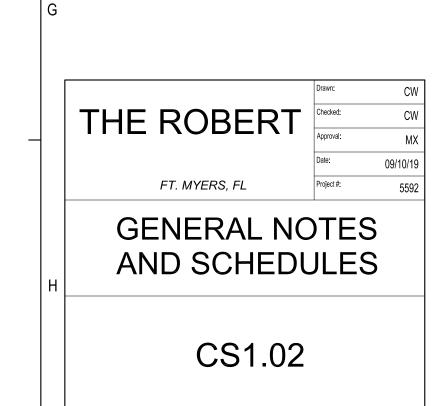
SIMPSON MTS 12 @ EACH —END SECURE TO ROOF TRUSSES —

STRAP OVER FRAMING

a EA TRUSS TYP



TYP TRANSON DETAIL



PERMIT REVIEW STAMP

ISSUE HISTORY

REVISION HISTORY

FUGLEBERG KOCH

CONSULTANT

ASE ENGINEERING SERVICES, INC.

STRUCTURAL DESIGN GROUP

TO THE BEST OF MY KNOWLEDGE, THE BUILDING DESIGN PLANS AND SPECIFICATIONS
COMPLY WITH BUILDING STRUCTURAL DESIGN CODE. THE SIGNING AND SEALING OF THE PLANS
AND SPECIFICATIONS ARE ONLY FOR THE BUILDING'S STRUCTURAL COMPONENTS AFFECTED
BY WIND, LIVE AND GRAVITY LOADS.

10244 East Colonial Drive, Suite 202 Orlando, Florida 32817 — 407—677—5565 Fax 407—730—2999 Certificate of Authorization No. 25873

2555 Temple Trail, Winter Park, FL 32789 (407) 629-0595

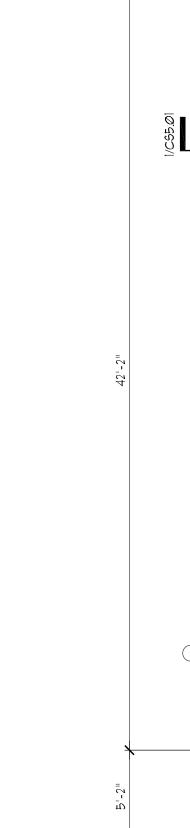
www. fuglebergkoch.com

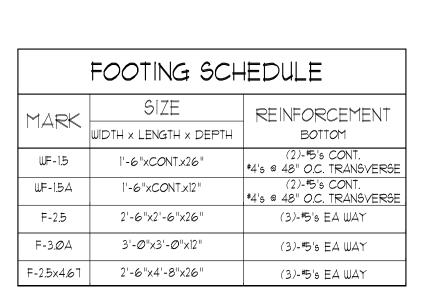
12/06/19

3 02/28/20

SCHEMATIC DESIGN
DESIGN DEVELOPMENT

PERMIT REVIEW SET





1 SEE GENERAL NOTES ON SHEET CSI.ØI.

DO NOT SCALE DRAWINGS. SEE ARCH'L.
DRAWINGS FOR ADDITIONAL DIMENSIONS
NOT SHOWN, VERIFY ALL DIMENSIONS WITH
ARCH'L. DRAWINGS PRIOR TO START OF
CONSTRUCTION. IF DISCREPANCIES SHOULD
OCCUR - CONTACT THE ARCHITECT IN WRITING
FOR CLARIFICATION BEFORE PROCEEDING.

3 4" (TOTAL) CONCRETE SLAB REINFORCED WITH 6x6-WI.4xWI.4 W.W.F. OVER 8 MIL VAPOR BARRIER ON COMPACTED SUBGRADE. COORDINATE ALL SLAB SLOPES, DEPRESSIONS AND LIMITS THERE OF WITH ARCH'L. DRAWINGS (FOR ACTUAL TOP OF SLAB

ELEVATIONS, SEE ARCH'L. AND / OR CIVIL DRAWINGS)

4 PRIOR TO CONCRETE PLACEMENT PROVIDE TERMITE

SEE ARCHITECTURAL SPECIFICATIONS.

MIDICATES WOOD STUD SHEAR WALL TYPE,
AND SHADING INDICATES EXTENT OF SHEAR
WALL. SEE THE SHEAR WALL SCHEDULE

ON SHEET SI.ØI FOR SHEAR WALL INFORMATION.

THE MAX. SPACING OF CONTROL JOINT FOR ENCLOSURE SPACE SHALL BE 20'-0" O.C., AND

SEE ARCH'L. DRAWINGS FOR LOCATIONS / LIMITS AND

CONSTRUCTION INFORMATION OF INTERIOR NON-BEARING PARTITION WALLS NOT SHOWN ON PLAN. SEE GENERAL NOTES.

9 SEE MECHANICAL DRAWINGS FOR LOCATION (ON PLANK)
OF MECHANICAL UNITS. H.C. MANUFACTURER / SUPPLIER
TO DESIGN FOR UNIT WEIGHT. AS REQUIRED VERIFY

USE HURRI-BOLT SYSTEM. SEE TYPICAL DETAIL 1/C56.03 FOR

CONNECTION OF FOOTING TO WOOD STUD, TRUSS TO STUD

FOR OPEN SPACE SHALL BE 8'-0" O.C.

8 COORDINATE ALL SLAB (TOPPING) SLOPES AND DEPRESSIONS WITH ARCH'L. DRAWINGS.

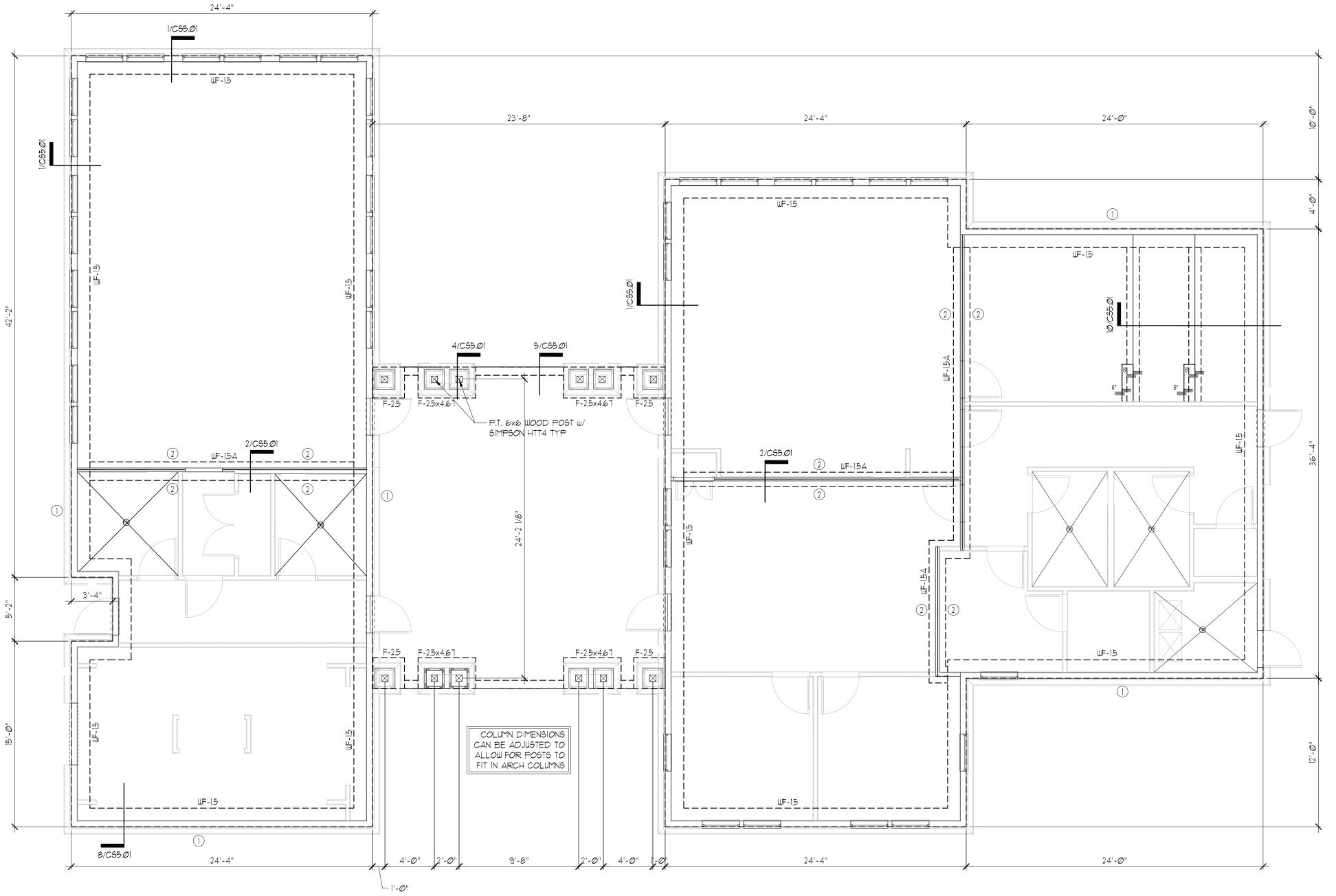
(FOR ACTUAL TOP OF SLAB ELEVATIONS,

WEIGHTS WITH MECHANICAL DRAWINGS.

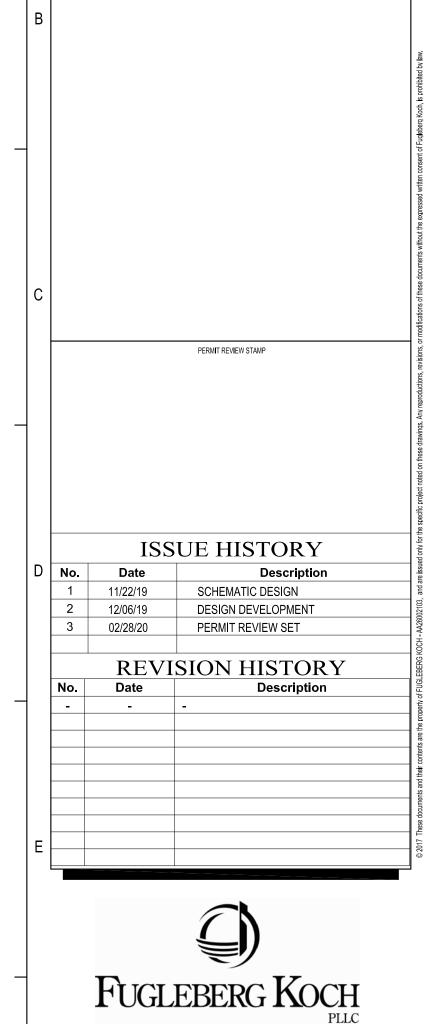
SEE ARCH'L. DRAWINGS)

WALL AND, POST U.N.O.

SOIL TREATMENT WITH ONE YEAR WARRANTY AND FOUR ANNUAL INSPECTIONS AND RENEWALS.







ASE ENGINEERING SERVICES, INC.

STRUCTURAL DESIGN GROUP

TO THE BEST OF MY KNOWLEDGE, THE BUILDING DESIGN PLANS AND SPECIFICATIONS COMPLY WITH BUILDING STRUCTURAL DESIGN CODE. THE SIGNING AND SEALING OF THE PLANS AND SPECIFICATIONS ARE ONLY FOR THE BUILDING'S STRUCTURAL COMPONENTS AFFECTED BY WIND, LIVE AND GRAVITY LOADS.

10244 East Colonial Drive, Suite 202

Orlando, Florida 32817 - 407-677-5565 Fax 407-730-2999

Certificate of Authorization No. 25873

Minsheng Xie P.E.
Florida No. 51161

CONSULTANT

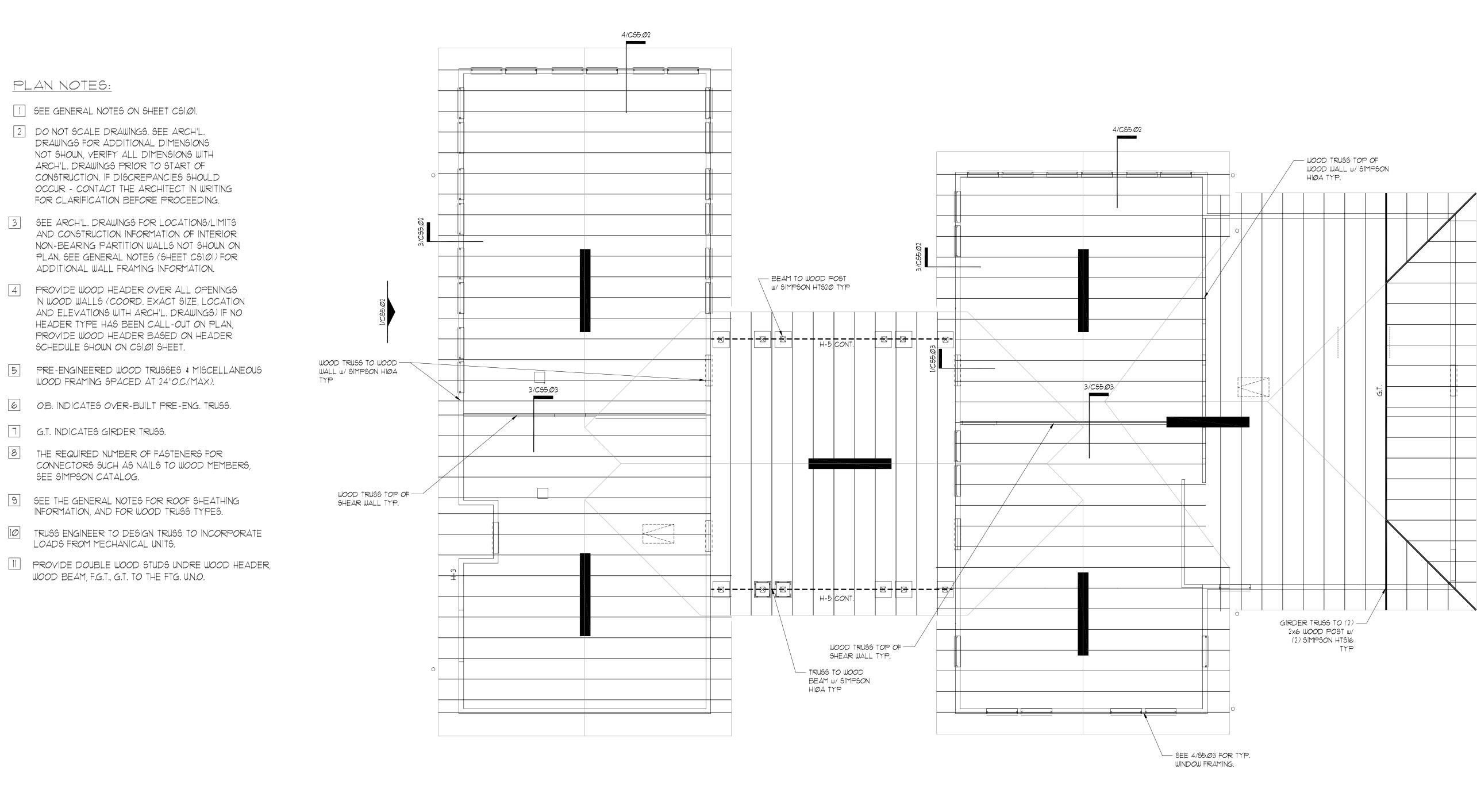
2555 Temple Trail, Winter Park, FL 32789 (407) 629-0595

www. fuglebergkoch.com

FOUNDATION PLAN								
FT. MYERS, FL	Project #:	5592						
	Date:	09/10/19						
THE ROBERT	Approval:	MX						
THE ROBERT	Checked:	CW						
		CVV						

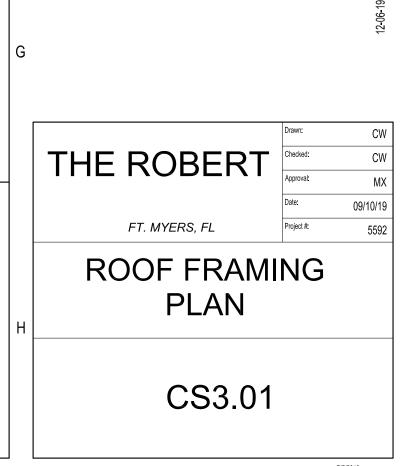
CS 2.01

5/2018



ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"



ISSUE HISTORY

SCHEMATIC DESIGN

PERMIT REVIEW SET

REVISION HISTORY

FUGLEBERG KOCH

ASE ENGINEERING SERVICES, INC.

TO THE BEST OF MY KNOWLEDGE, THE BUILDING DESIGN PLANS AND SPECIFICATIONS COMPLY WITH BUILDING STRUCTURAL DESIGN CODE. THE SIGNING AND SEALING OF THE PLANS AND SPECIFICATIONS ARE ONLY FOR THE BUILDING'S STRUCTURAL COMPONENTS AFFECTED BY WIND, LIVE AND GRAVITY LOADS.

10244 East Colonial Drive, Suite 202
Orlando, Florida 32817 - 407-677-5565 Fax 407-730-2999
Certificate of Authorization No. 25873

2555 Temple Trail, Winter Park, FL 32789 (407) 629-0595

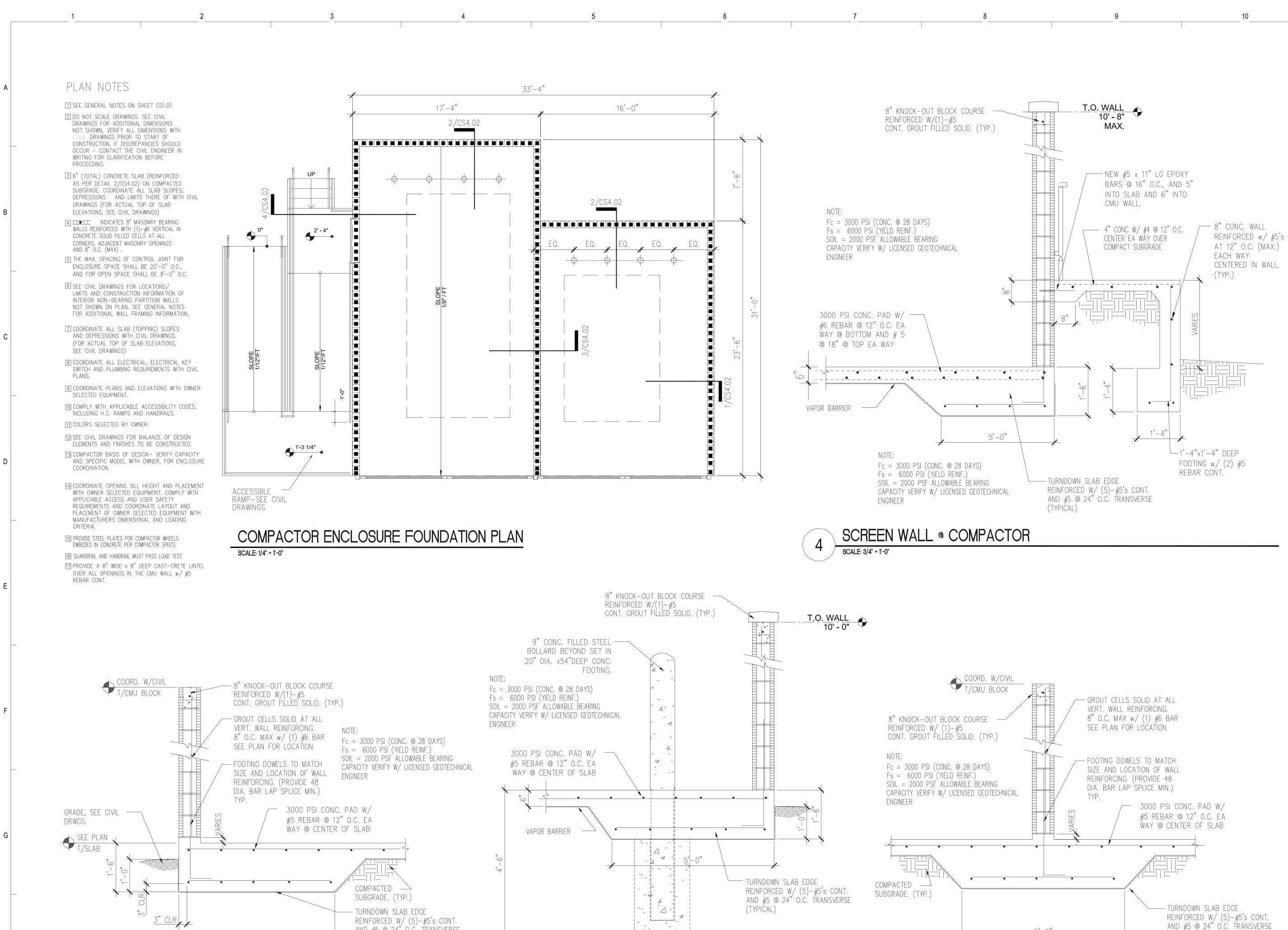
www. fuglebergkoch.com

DESIGN DEVELOPMENT

Description

11/22/19

2 12/06/19 3 02/28/20



AND #5 @ 24" O.C. TRANSVERSE

SECTION

SCALE: 3/4" = 1'-0"

(TYPICAL)

5'-0"

THICKENED SLAB/FOOTING

SCALE: 3/4" = 1'-0"

THE ROBERT 09/10/19 FT. MYERS, FL TRASH ENCLOSURE FOUNDATION PLAN

PERMIT REVIEW STAMP

ISSUE HISTORY

REVISION HISTORY

SCHEMATIC DESIGN

PERMIT REVIEW SET

DESIGN DEVELOPMENT

Description

11/22/19

Date

2555 Temple Trail, Winter Park, FL 32789 (407) 629-0595

CONSULTANT

ASE ENGINEERING SERVICES, INC.
STRUCTURAL DESIGN GROUP

TO THE BEST OF MY KNOWLEDGE, THE BUILDING DESIGN PLANS AND SPECIFICATIONS COMPLY WITH BUILDING STRUCTURAL DESIGN CODE. THE SIGNING AND SEALING OF THE PLANS AND SPECIFICATIONS ARE ONLY FOR THE BUILDING'S STRUCTURAL COMPONENTS AFFECTED BY WIND, LIVE AND GRAVITY LOADS.

10244 East Colonial Drive, Suite 202 Orlando, Florida 32817 — 407—677—5565 Fax 407—730—2999 Certificate of Authorization No. 25873

www.fuglebergkoch.com

5'-0"

THICKENED SLAB/FOOTING

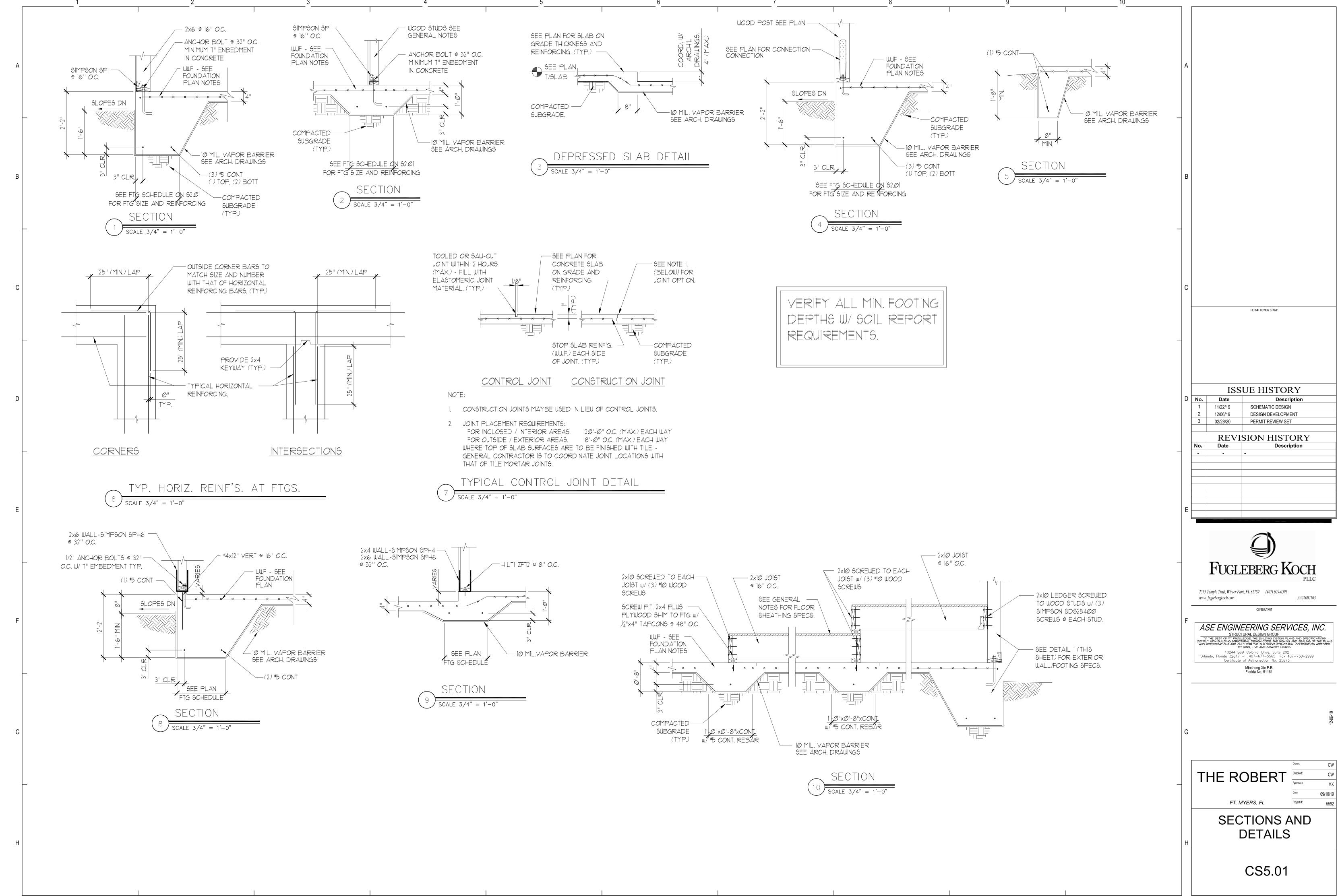
SCALE: 3/4" = 1'-0"

(TYPICAL)

12/06/19

3 02/28/20

CS4.02



7/5/2018 9:55:20 AM

