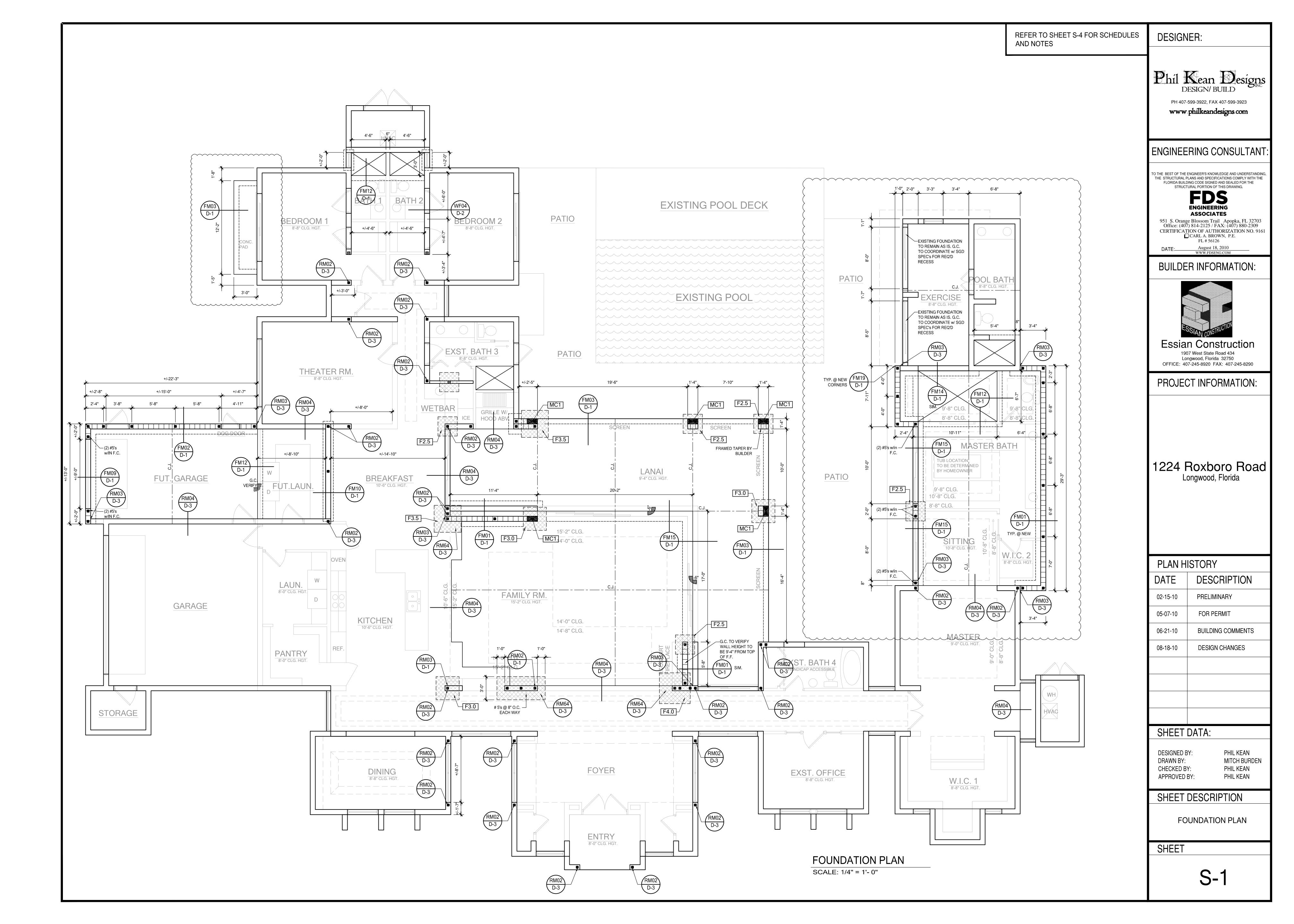
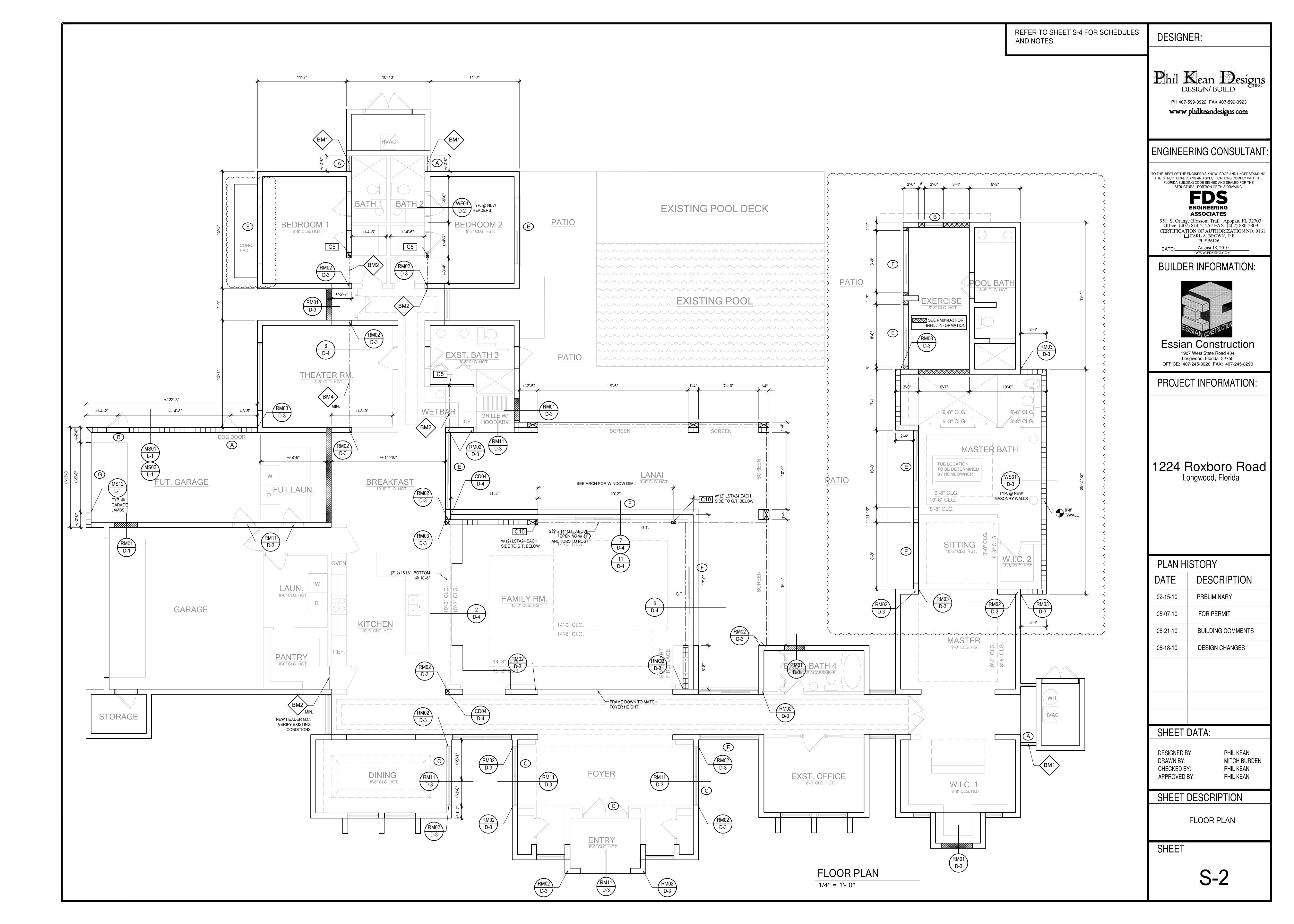
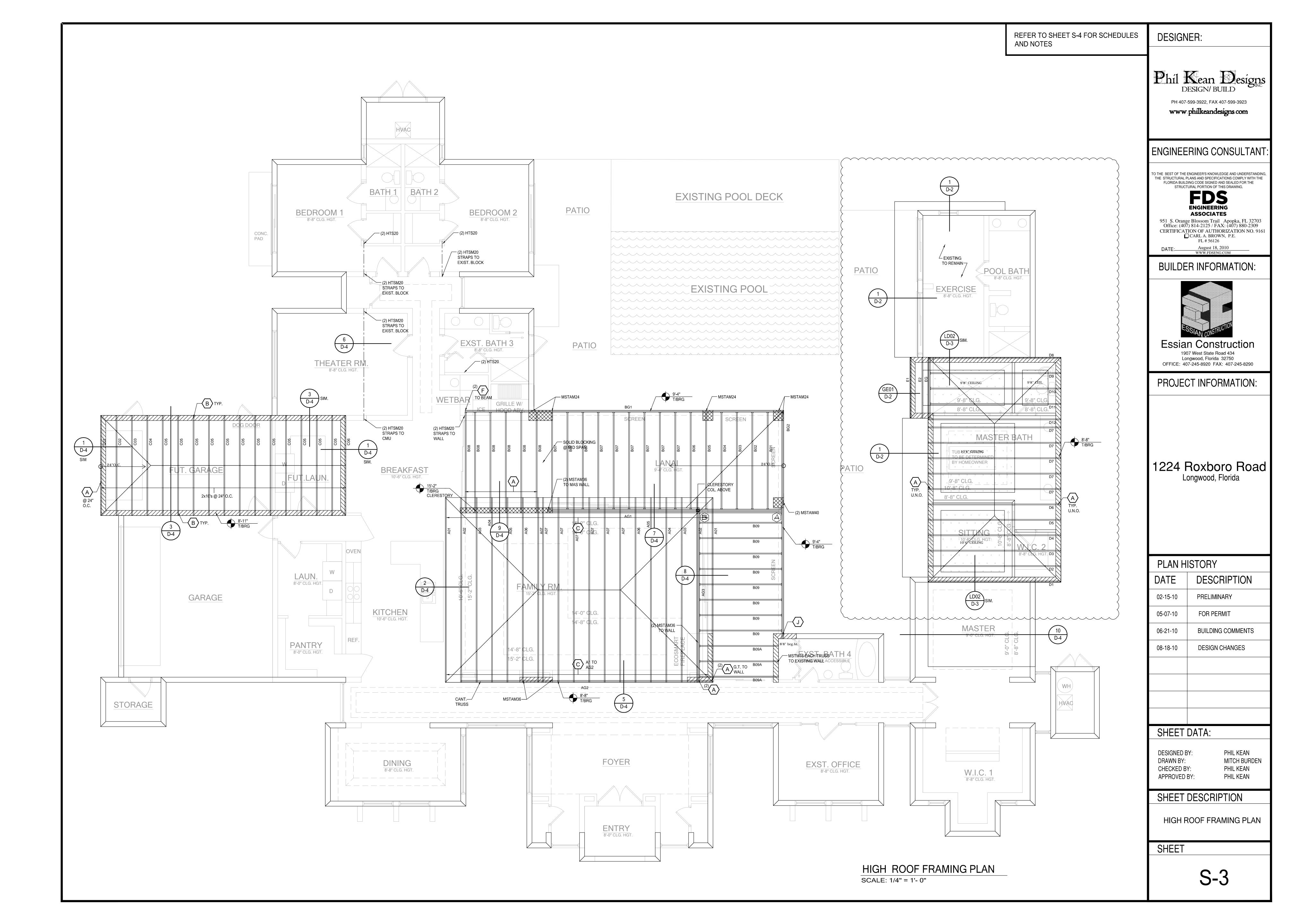
TERMITE SPECIFICATIONS	STRUCTURAL	STF	RUCTURAL DESIGN CRITERIA	DESIGNER:		
SECTION R320 PROTECTION AGAINST TERMITES TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMITICIDES, INCLUDING SOIL APPLIED PESTICIDES, BAITING SYSTEMS, AND PESTICIDES APPLIED TO WOOD, OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED FOR USE A PREVENTIVE TREATMENT TO NEW CONSTRUCTION (SEE SECTION 202, REGISTERED TERMITICIDE). UPON COMPLETION OF THE APPLICATION OF THE TERMITE PROTECTIVE TREATMENT, A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES.	CAST IN PLACE CONCRETE 1. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 2500 PSI, A SLUMP OF 5" PLUS OR MINUS 1", AND HAVE 2 TO 5% AIR ENTRAINMENT, AND A MAXIMUM WATER/CEMENT RATIO OF 0.63 2. ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A-615-04a GRADE 40. 3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185. WWF SHALL BE LAPPED AT LEAST 6" AND CONTAIN AT LEAST ONE CROSS WIRE WITHIN THE 6". 4. HOOKS SHALL BE PROVIDED AT DISCONTINUOUS ENDS OF ALL TOP BARS OF BEAMS. 5. HORIZONTAL FOOTING BARS SHALL BE BENT 1'-0" AROUND CORNERS OR CORNER BARS WITH A 2'-0" LAP PROVIDED.	REFABRICATED WOOD TRUSSES ALL PREFABRICATED WOOD TRUSSES SHALL BE SECURELY FASTENED TO THEIR SUPPORTING WALLS OR BEAMS WITH HURRICANE CLIPS OR ANCHORS. PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF THE "NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENERS" AS RECOMMENDED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION. TRUSS MEMBERS AND CONNECTIONS SHALL BE PROPORTIONED (WITH A MAXIMUM ALLOWABLE STRESS INCREASE FOR LOAD DURATION OF 25%) TO WITHSTAND THE LIVE LOADS GIVEN IN THE NOTES AND TOTAL DEAD LOAD. BRIDGING FOR PRE-ENGINEERED TRUSSES SHALL BE AS REQUIRED BY THE TRUSS MANUFACTURER UNLESS NOTED ON THE PLANS.	NFPA BUILI SPEC BUILI NATIO WOO APA I LIVE LOADS: ROOF RESII BALC STAIF	DENTIAL FLOOR, UNLESS OTHERWISE INDICATED 50 PSF 60 PSF- Under 100 sq. ft. 40 PSF	Phíl DI	ean Designs ESIGN/ BUILD 599-3922, FAX 407-599-3923 philkeandesigns.com
TREATMENT IS IN ACCORDANCE WITH RULES AND LAWS ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES." NOTE: 1) METHOD OF TREATMENT SHALL BE APPROVED BY THE GOVERNING JURISDICTION "LIQUID BORATE OR BOR-A-COR" PRODUCT METHODS MUST BE DETERMINED AT PERMIT STAGE AND PRODUCT APPROVAL DATA MUST BE ON FILE WITH THE BUILDING DEPARTMENT. 2) PRESSURE TREATED LUMBER THAT HAS BEEN CUT OR DRILLED THAT EXPOSES UNTREATED PORTIONS OF WOOD ARE REQUIRED TO BE FIELD TREATED TO PREVENT INSECT INFESTATION.	 6. MINIMUM LAP SPLICES ON ALL REINFORCING BAR SPLICES SHALL BE 40 BAR DIAMETERS TYP. 7. CONCRETE COVER MIN. 3" WHEN EXPOSED TO EARTH OR 1 1/2" TO FORM. REINFORCING STEEL ALL REINFORCING STEEL SHALL BE NEW DEFORMED BARS FREE FROM RUST, SCALE & OIL & SHALL MEET ASTM A-615-04a REINFORCING FOR FOOTING SHALL BE SUPPORTED ON PRE-CAST CONCRETE PADS, TOP REINFORCING SHALL BE POSITIVELY SUPPORTED BY TEMPORARY STRINGERS. DOWELS FOR COLUMNS & FILLED CELLS SHALL BE SECURED IN PLACE BY USING ADDITIONAL CROSS-REINFORCING TIED TO FOOTING REINFORCING. SPLICES IN REINFORCING WHERE PERMITTED SHALL BE THE FOLLOWING MINIMUM, UNLESS OTHERWISE INDICATED ON THE DRAWINGS: 	TRUSS ELEVATIONS AND SECTIONS ARE FOR GENERAL CONFIGURATION OF TRUSSES ONLY. WEB MEMBERS ARE NOT SHOWN, BUT SHALL BE DESIGNED BY THE TRUSS MANUFACTURER IN ACCORDANCE WITH THE FOLLOWING DESIGN LOADS: DESIGN SPECIFICATIONS FOR LIGHT WEIGHT METAL PLATE CONNECTED WOOD TRUSSES PER THE TRUSS PLATE INSTITUTE TPI LATEST EDITION. PRE-ENGINEERED WOOD TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH SPECIFIED LOADS AND GOVERNING CODES. SUBMITTALS SHALL INCLUDE TRUSS FRAMING PLANS AND DETAILS SHOWING MEMBER SIZES, BRACING, ANCHORAGE, CONNECTIONS, TRUSS LOCATIONS, AND PERMANENT BRACING AND/OR BRIDGING AS REQUIRED FOR ERECTION AND FOR THE PERMANENT STRUCTURE. EACH SUBMITTAL SHALL BE SIGNED AND SEALED BY A FLORIDA REGISTERED STRUCTURAL ENGINEER.	CONCRETE ALL CONCRETE STRENGTH CONCRETE CONCRETE STRENGTH CONCRETE CONCRETE STRENGTH	T PARTITIONS (DEAD LOAD), U.N.O. CONCRETE UNLESS OTHERWISE INDICATED PEA GRAVEL CRETE FOR MASONRY CELLS ONLY (DO NOT USE FOR 3000 PSI CRETE COLUMNS OR TIE BEAMS) DED WIRE FABRIC SHALL CONFORM TO REINFORCING BARS STIRRUPS AND TIES PROPYLENE FIBERS FOR SLABS ON GRADE SON HIGH STRENGTH EPOXY-TIE ANCHORING ADHESIVE USED IN THE DESIGN OF THIS PRODUCT. IF CONTRACTORS ITO USE A DIFFERENT EPOXY, THEY MUST FIRST CONTACT ENGINEER OF RECORD FOR WRITTEN APPROVAL.	TO THE BEST OF THE E THE STRUCTURAL PI FLORIDA BUILDI STRUCTU 951 S. Orange Office: (407)	ENGINEER'S KNOWLEDGE AND UNDERSTANDING, LANS AND SPECIFICATIONS COMPLY WITH THE DING CODE SIGNED AND SEALED FOR THE URAL PORTION OF THIS DRAWING. ENGINEERING ASSOCIATES Be Blossom Trail Apopka, FL 32703 9 814-2125 / FAX: (407) 880-2309 ION OF AUTHORIZATION NO. 9161 CARL A. BROWN, P.E. FL # 56126
OPTIONAL BORATE APPLIED TO ALL FRAME MEMBERS WITHIN 24" A.F.F. NOTE:	TEMPERATURE REINFORCING: 20 DIA. OR 1'-0" MIN. WELDED WIRE MESH: 8" LAP	SUBMIT 3 COPIES FOR REVIEW AND APPROVAL PRIOR TO FABRICATION. THE TRUSS MANUFACTURER SHALL DETERMINE ALL SPANS WORKING POINTS, BEARING POINTS, AND SIMILAR CONDITIONS. TRUSS SHOP DRAWINGS SHALL SHOW ALL TRUSSES, ALL BRACING MEMBERS, AND ALL TRUSS TO TRUSS HANGERS.	MASONRY TYPE UNITS: CONC CONT	I C90-03, STANDARD WEIGHT UNITS, fm=1500 PSI MORTAR "S" 1800 PSI CRETE GROUT 3000 PSI TINUOUS MASONRY INSPECTION IS REQUIRED DURING STRUCTION	BUILDE	August 18, 2010 WWW.FDSENG.COM R INFORMATION:
WHERE PROJECT IS TO BE LOCATED IN KNOWN RADON GAS PREVALENT AREAS, APPENDIX "F" OF THE 2007 FLORIDA RESIDENTIAL BUILDING CODE IS TO BE IMPLEMENTED. CONCRETE STRENGTH IN THESE AREAS ARE TO BE A MINIMUM OF 3000 P.S.I. THEREFORE, ANY AND ALL NOTES ON THESE PLANS THAT INDICATE 2500 P.S.I. SHALL BE REPLACED WITH 3000 P.S.I. FOR THE CONCRETE STRENGTH.	CONFORMING TO ASTM C90-03, WITH A MINIMUM NET COMPRESSIVE STRENGTH OF 1900 PSI (f'm = 1500 PSI) MORTAR SHALL BE TYPE "S", CONFORMING TO ASTM C270-04. COARSE GROUT SHALL CONFORM TO ASTM C476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 3000 PSI SLUMP 8" TO 11". VERTICAL REINFORCEMENT SHALL BE AS NOTED ON THE DRAWINGS WITH	PLIFT CONNECTORS UPLIFT CONNECTORS SUCH AS HURRICANE CLIPS, TRUSS ANCHORS AND ANCHOR BOLTS ARE ONLY REQUIRED ON MEMBERS IN WALLS THAT ARE EXPOSED TO UPLIFT FORCES. INTERIOR LOAD BEARING WALLS ARE NOT ALWAYS EXPOSED TO UPLIFT FORCES. THE MEMBERS OF THESE WALLS WOULD NOT NEED TO HAVE CONNECTORS APPLIED. PLEASE CONSULT THE TRUSS ENGINEER FOR THE LOCATION OF THESE WALLS.	STRUCTURAL ALL S STEEL: AND R A36 C WOOD FRAMING: BEAN NO. 2 ROOF SHEA	STRUCTURAL AND MISCELLANEOUS STEEL A36 36,000 PSI, U.N.O SHOP FIELD WELDS: E70XX ELECTRODES ALL BOLTS CAST IN CONCRETE: ASTM DR ASTM A-307 MS, RAFTERS, JOIST, PLATES, ETC. U.N.O. 2 SOUTHERN YELLOW PINE (19% M.C.) F DECK: PLYWOOD C-C/C-D, EXTERIOR, or OSB FLOOR ATHING: T&G A-C GROUP 1 APA RATED (48/24) WALL ATHING: PLYWOOD C-C/C-D, EXTERIOR OR OSB VERSA LAM M Fb = 2900 PSI (2.0E)	Fssiar	n Construction
A.B. Anchor Bolt Abv. Above A/C Air-Conditioner Adj. Adjustable A.F.F. Above Finished Floor A.H.U. Air Handler Unit ALT. Alternate B.C. Base Cabinet B.F. Bifold Door Bk Sh Book Shelf Bm. Beam BOT. Bottom B.P. Bypass door FIR. Sys. Floor System F.Pl. Fireplace F.O.M. Face Of Masonry F.O.M. Face Of Masonry F.D. Fireplace F.O.M. Face Of Masonry F.D. Fireplace F.O.M. Face Of Masonry F.D. Fireplace Fef. Refrigerator F.D. Req'd. Required FR. Red. Radius F.P. Fireplace F.D. Fireplace Fef. Refrigerator F.D. Req'd. Required FR. Red. Radius F.P. Red'd. Required FR. Red. Radius F.P. Red'd. Required FR. Refrigerator F.D. Req'd. Required FR. Refrigerator F.O.M. Face Of Masonry Feg. Footing Fr. Footing Fam. Room FR. Red'd. Required FR. Refrigerator F.O.M. Face Of Masonry FReq'd. Required FR. Red'und Face Of Masonry FReq'd. Required FR. Refrigerator Footing Fire Fireplace Footing Foo	 VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM AND AT A MAXIMUM SPACING OF 192 BAR DIAMETERS. REINFORCEMENT SHALL BE PLACED IN THE CENTER OF THE MASONRY CELL TYPICAL UNLESS OTHERWISE NOTED. REINFORCING STEEL SHALL BE LAPPED A MINIMUM OF 40 BAR DIAMETERS, UNLESS OTHERWISE NOTED ON THE DRAWINGS. GROUT STOPS SHALL BE PROVIDED BELOW BOND BEAM. PLASTIC SCREEN, METAL LATH STRIP OR CAVITY CAPS MAY BE USED TO PREVENT THE FLOW OF GROUT INTO CELLS BELOW. THE USE OF FELT PAPER AS A STOP IS PROHIBITED. WOOD CONSTRUCTION WOOD CONSTRUCTION SHALL CONFORM TO THE NDS "NATIONAL DESIGN 	MISSED "J" BOLTS FOR WOOD BEARING WALLS MAY BE SUBSTITUTED WITH 1/2" DIA. EPOXY ANCHORS WITH 7" EMBEDMENT. SIMPSON "SET" EPOXY ADHESIVE BINDER FOLLOWING ALL MANUFACTURER'S RECOMMENDATIONS OR SIMPSON 1/2" TITEN HD BOLTS WITH MINIMUM 7" EMBEDMENT. SEE PLAN FOR EMBEDMENT DEPTH AT FLOOR STEPS. FOR MISSED VERT. DOWELS, DRILL A 3/4" DIAMETER HOLE 6" DEEP AT THE LOCATION OF THE OMITTED REBAR AND INSTALL A 32" LONG #5 BAR INTO THE EPOXY FILLED HOLE. USE A TWO PART EMBEDMENT EPOXY (SIMPSON "SET", EPOXY) MIXED PER THE MANUFACTURER'S INSTRUCTIONS. ASSURE THAT ALL DUST AND DEBRIS FROM DRILLING ARE REMOVED FROM THE HOLE BY BRUSHING AND USING COMPRESSED AIR PRIOR TO APPLYING THE EPOXY. ALLOW THE EPOXY TO CURE TO THE MANUFACTURER'S SPECIFICATIONS, THEN FILL THE CELL IN THE NORMAL WAY DURING BOND BEAM POUR.	DESITOP CHOI WOOD ROOF TRUSSES: BOTT (NON SEE FOR MAXI DEAI	GN LOADS: GN LOADS: CHORD LIVE LOAD: TOP CHORD LIVE LOAD: TOP TOP TOP TOP TOP TOP TOP TO	190 Lor OFFICE: 407	77 West State Road 434 ngwood, Florida 32750 7-245-8920 FAX: 407-245-8290 CT INFORMATION:
Brg. Bearing Cir. Circle Clg. Ceiling CJ Control Joint Col. Column Comp. A/C Compressor CT. Ceramic Tile D Dryer Dec. Decorative D Ded. Dedicated Outlet Dia. Diameter Int. Interior K/Wall Kneewall K.S. Knee Space K/Wall Kneewall K.S. Knee Space Thik'n. Thicken T.O.B. Top of Block T.O.M. Top of Masonry T.O.P. Top of Plate Trans. Transom Window Typ. Typical U.N.O. Unless Noted Otherwise Vert. Vertical	 SPECIFICATION FOR WOOD CONSTRUCTION", 2005 EDITION. ALL EXTERIOR WOOD STUD WALLS, BEARING WALLS, SHEAR WALLS AND MISC. STRUCTURAL WOOD FRAMING MEMBERS, (I.E. BLOCKING OR GABLE END BRACING) SHALL BE EITHER SOUTHERN PINE, OR S.P.F. NUMBER 2 GRADE OR BETTER SHALL BE USED REGARDLESS OF SPECIES. ANY WOOD FRAME INTERIOR BEARING WALL STUDS THAT HAVE HOLES IN THE CENTER OF THE STUD UP TO 1" DIA. SHALL HAVE STUD PROTECTION SHIELDS. ALL HOLES OVER 1" IN DIA. FOR PLUMBING LINES, ETC. SHALL BE REPAIRED WITH SIMPSON HSS2 STUD SHOES, TYP., U.N.O. MANY OF THE NEW PRESSURE TREATED WOODS USE CHEMICALS THAT ARE CORROSIVE TO STEEL. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THE TYPE OF WOOD TREATMENT AND TO SELECT APPROPRIATE 	FOR MORTAR JOINTS LESS THAN 1/4", PROVIDE (1) #5 VERT. IN CONC. FILLED CELL EACH SIDE OF THE JOINT (BAR DOES NOT HAVE TO BE CONT. TO FOOTING). MISSED LINTEL STRAPS FOR MASONRY CONSTRUCTION MAY BE SUBSTITUTED WITH (1) SIMPSON MTSM16 TWIST STRAP W/ (4) ½" X 2½" TITENS TO MASONRY AND (7)-10d NAILS TO TRUSS FOR UPLIFTS LESS THAN 860 LBS (USE (2) MTSM16 FOR UPLIFTS LESS THAN 1720#). NO MORE THAN 10 STRAPS MAY BE SUBSTITUTED OR NO MORE THAN 3 IN A ROW. IF GIRDER TRUSS CONNECTIONS ARE MISSED, CONTACT THE ENGINEER OF RECORD FOR SUBSTITUTION.	TRUSSES: DEAD LIVE TOTA SOIL BEARING VALUE: SOIL CONI CONI VERI	D LOAD: 15 PSF LOAD: 40 PSF		Roxboro Road ongwood, Florida
Disp. Disposal Min Minimum V.L. Versalam Dist. Distance Mir. Microlam VTR Vent through Roof D.S. Drawer Stack Mir. Mirror W Washer D.V. Dryer Vent Mono Monolithic W/ With D.W. Dishwasher N.T.S. Not to Scale Ea. Each Opn'g. Opening W.A. Wedge Anchor E.W. Each Way Opt. Optional Wd Wood Elec. Electrical Pc. Piece WP Water Proof	CONNECTORS THAT RESIST CORROSION. FOR EXAMPLE, ACQ-C, ACQ-D, CBA-A OR CA-B REQUIRE HOT-DIPPED GALVANIZED OR STAINLESS STEEL FASTENERS. DOT SODIUM BORATE (SBX) DOES NOT. STRUCTURAL STEEL ALL STRUCTURAL & MISCELLANEOUS STEEL A36 36,00PSI, U.N.O. TUBE STEEL: ASTM A500, GRADE B, Fy = 46.000 PSI	NOTICE TO BUILDER AND ALL SUBCONTRACTORS It is the intent of the Engineer listed in the titleblock of these documents that these documents be accurate, providing Licensed Professionals clear information. Every attempt has been made to prevent error. The Builder and all subcontractors are required to review all the information contained in these documents, prior to the commencement of any work. The Engineer are not responsible for any plan errors, omissions, or misinterpretations undetected and not reported to the Engineer prior to	MEAN ROOF 15 FT. HIGH TYPICAL FOR SINGLE STORY HOMES MEAN ROOF 30 FT. HIGH TYPICAL FOR 2 STORY HOMES	WIND SPEED LOADS BASED ON FBC, CHAPTER 16 AND ASCE 7-05 WIND SPEED 120 MPH (3 SEC. GUSTS) WIND IMPORTANCE FACTOR (Iw) = 1.0 CATEGORY II WIND EXPOSURE "B" (FBC CHAPTER 16) INTERNAL PRESSURE COEFFICIENT = +/- 0.18 (ENCLOSED) DESIGN WIND PRESSURE:	PLAN HI DATE	ISTORY DESCRIPTION PRELIMINARY
Elev. Elevation Ped. Pedestal Ext. Exterior P.L. Parallam Exp. Expansion PLF Pounds per linear foot F.B.C. Florida Bldg. Code Fin. Flr. Finished Floor Plt. Ht. Plate Height F.G. Fixed Glass PSF Pounds per square foot Flr. Floor P.T. Pressure Treated Fdn. Foundation Pwd. Powder Room	PIPE STEEL: ASTM A53, TYPE E OR S, Fy = 35,000 PSI SHOP AND FIELD WELDS: E70XX ELECTRODES STRUCTURAL BOLTS: ASTM A325, BEARING TYPE CONNECTIONS STRUCTURAL BOLTS: ASTM A307 STRUCTURAL BOLTS: ASTM A307 FOR SECONDARY CONNECTIONS WHERE INDICATED. ALL BOLTS CAST IN CONCRETE: ASTM A36 OR ASTM A-307 STEEL REINFORCEMENT SHOPDRAWINGS TO BE PROVIDED TO ENGINEER OF RECORD BEFORE FABRICATION	construction. All construction MUST be in accordance to the information found in these documents. Any questions regarding the information found in these plans should be directed to our Quality Assurance Manager at 407-814-2125 immediately. No backcharges will be considered for reimbursement by the Engineer without advanced notification and approval by the Engineer. Payments will be made in accordance to the terms of the agreement.	THIS DRAWING AND DESIGN IS VAI FOR 12 MONTHS AFTER THE DATE IS SIGNED AND SEALED OR WHILE CURRENT CODE IS VALID BUILDING TYPE: TYPE V CONSTRUCTION TYPE: TYPE 5B	LID (COMPONENT AND CLADDING) WORST CASE (10 SF - END ZONE) # END ZONE IS ONLY WITHIN 5'-0"	05-07-10 06-21-10 08-18-10	FOR PERMIT BUILDING COMMENTS DESIGN CHANGES
DATE DESCRIPTION			SHEET NO. S-0 S-1 S-2	DRAWING SHEET INDEX: STRUCTURAL NOTES FOUNDATION PLAN FIRST FLOOR PLAN		
			S-3 S-4 L-1 D-1 D-2	ROOF FRAMING PLAN SCHEDULES LINTEL PLAN DETAILS DETAILS	DESIGNED BY DRAWN BY: CHECKED BY APPROVED E	Y: PHIL KEAN MITCH BURDEN Y: PHIL KEAN
			D-3 D-4	DETAILS DETAILS		RUCTURAL NOTES
					SHEET	S-0







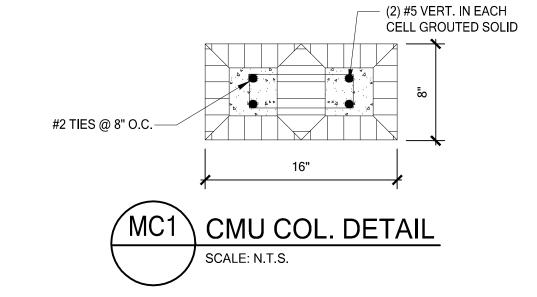
COLUMN SCHEDULE GENERAL FOUNDATION NOTES: MARK COLUMN SIZE (BASE) CONN. & FASTENER 1 PROVIDE MIN. 6 MIL. APPROVED VAPOR BARRIER. ALL JOINTS TO BE LAPPED MIN. 6" AND SEALED. C1 (3) 2 x 4 #2 SPF 2 4" 2500 PSI CONC. SLAB W/ 6X6 10/10 W W M, OR FIBERMESH (4) 16d TOENAILS /FIBERMIX ADDED TO THE CONCRETE. IN ACCORDANCE W/ MANUF'S INSTRUCTIONS AND NER-284 FOR FIBERMESH OR LTT20B W/ 1/2" WEDGE C2 (3) 2 x 4 #2 SPF NER-414 FOR FIBERMIX, OVER 6 MIL VISQUEEN VAPOR BARRIER & ANCHOR* & (10) 10d NAILS TREATED FOR TERMITES. 3 INDICATES FILLED CELL W/3000 PSI CONC. FROM FTR. TO BEAM W/ (1) #5 REBAR TYPICAL ABOVE SLAB. HOOKED FTG. DOWELS 5" EMBEDMENT W/ 25" EXT. ABOVE SLAB. □ INDICATES FILLED CELL W/3000 PSI CONC. W/(1) #5 REBAR TYPICAL BELOW SLAB. HOOKED FTG. DOWELS 5" EMBEDMENT W/ EXT. TO TOP COURSE BELOW SLAB. 4 CONSULT W/ MANUF. SPECIFICATIONS PRIOR TO POURING OR RECESSING DOOR SILLS OR SLIDING GLASS DOOR SILLS. 5 EXTERIOR SLABS SHALL SLOPE MIN. 1/12" PER FOOT AWAY FROM HOUSE U.N.O. ON PLAN. 6 CONTROL JOINTS (IF SHOWN) ARE NOT REQUIRED BY CODE BUT ARE SUGGESTED (ESPECIALLY WHEN USING FIBER REINF. CONCRETE OR IN EXTERIOR CONDITIONS). CONTROL JOINTS TO BE 1/8" SAW CUT A DEPTH OF 1/4" OF THE THICKNESS OF THE SLAB. FILL CUT W/APPROVED JOINT MATERIAL OR USE ALTERNATE APPROVED METHOD. PENDING SITE CONDITIONS, G.C. TO DETERMINE REQUIRED COURSES FOR STEMWALL FOUNDATION. SEE STEMWALL CHART ON D-1 FOR REQUIRED REINFORCEMENT AND FOOTING SIZES PENDING DEPTH OF STEMWALL. 8 NO WOOD STAKES PERMITTED IN FOUNDATION. 9 PENDING SITE CONDITIONS, FOUNDATION MAY HAVE TO BE STEPPED DOWN. SEE FM18 ON SHEET D-1 FOR ADDITIONAL INFORMATION. G.C. TO DETERMINE STEP LOCATIONS IF REQUIRED. FOOTING SCHEDULE GRAVITY SIZE DEPTH REINFORCING CAP. [lbs] 2'-6" X 2'-6" 1'-0" 3 #5 E.W. BOT. 11000 1'-0" 3'-0" X 3'-0" 4 #5 E.W. BOT. 15600 3'-6" X 3'-6" 1'-0" 4 #5 E.W. BOT. 21500 4'-0" X 4'-0" | 1'-0" | 5 #5 E.W. BOT. F4.0 28000 4'-6" X 4'-6" | 1'-4" | 5 #5 E.W. BOT. 34500 6 #5 E.W. BOT. 42500 5'-0" X 5'-0" F6.0 6'-0" X 6'-0" 1'-4" 7 #5 E.W. BOT.

FOUNDATION SCHEDULES

- INDICATES TWO-STORY FOOTING

- INDICATES CONCRETE FOOTING

LEGEND



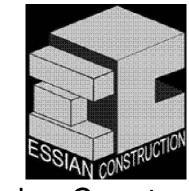
C2	(3) 2 x 4 #2 SPF ANCHOR* & (10) 10d NAILS				1290		
C3	(3) 2 x 4 #2 SYP			6d TOENAILS	0		
C4	(3) 2 x 4 #2		20B W/ 1/2" W HOR* & (10)	1500			
C5	4 x 4 P.T.#2 SYP POST	2		44 W/ 5/8" AT 2) 16d NAILS		2200	
C6	6 x 6 P.T. #2 SYP POST			66 W/ 5/8" AT 2) 16d NAILS	R**	2300	
C7	8 x 8 P.T. #2 SYP POST			88 W/ (2) - 5/8 3) - 16d NAILS		2320	
C8	3.5" x 3.5" F Fb=2900 (WOLMANI	ZED IF EXT.)	& (14 WO0	5-SDS2.5 W/ I) 1/4"X2 1/2" DD SCREWS	SDS	5645	
C9	3.5" x 5.25" Fb=2900 (W IF EXT.)	& (14	HDU5-SDS2.5 W/ 5/8" ATR** & (14) 1/4"X2 1/2" SDS 5645 WOOD SCREWS				
C10	3.5" x 7" P.L. 2.0E Fb=2900 (WOLMANIZED IF EXT.)		& (20	IDU8-SDS2.5 W/ 7/8" ATR*** (20) 1/4"X2 1/2" SDS VOOD SCREWS			
C11	5.25" x 5.25" P.L. 2.0E Fb=2900 (WOLMANIZED IF EXT.)		HDU & (20	8-SDS2.5 W/ 0) 1/4"X2 1/2" DD SCREWS		6970	
C12	7" x 7" P.L. 2 Fb=2900 (WOLMANI	ZED IF EXT.)	HDU & (20	HDU8-SDS2.5 W/ 7/8" ATR*** 3 (20) 1/4"X2 1/2" SDS WOOD SCREWS 697			
** MIN	IIMUM 6 INCH	CH EMBEDMENT I EMBEDMENT I I EMBEDMENT	FOR	: 1/2" WEDGE 5/8" ATR	ANCHOR		
		BEAM S	CH	EDULE			
MARK	S BE/	AM SIZE			NNECTIONS		
BM1		(2) - 2 x 8 #2 SYP W/ 7/16" OSB FLITCH PLATE		CONNECTION: PROVIDE (2) SIMPSON LSTA18 OR (2) SIMPSON HTS20 TO WOOD POST OR (2) SIMPSON HETA16 TO CMU COL U.N.O. ON ROOF PLAN.			
BM2	\ ' '	(2) - 2 x 10 #2 SYP W/ 7/16" OSB FLITCH PLATE		CONNECTION: PROVIDE (2) SIMPSON LSTA24 OR (2) SIMPSON HTS20 TO WOOD POST OR (2) SIMPSON HETA16 TO CMU COL. U.N.O. ON ROOF PLAN			
ВМЗ		(2) - 2 x 12 #2 SYP W/ 7/16" OSB FLITCH PLATE		CONNECTION: PROVIDE (2) SIMPSON LSTA24 OR (2) SIMPSON HTS20 TO WOOD POST OR (2) SIMPSON HETA16 TO CMU COL. U.N.O. ON ROOF PLAN			
ВМ4		(2) - 1 3/4" x 9 1/4" LVL 2.0E Fb=2950		CONNECTION: PROVIDE (2) SIMPSON LSTA24 OR (2) SIMPSON HTS20 TO WOOD POST OR (2) SIMPSON HETA16 TO CMU COL. U.N.O. ON ROOF PLAN			
BM5		(2) - 1 3/4" x 11 7/8" LVL 2.0E Fb=2950		CONNECTION: PROVIDE (2) SIMPSON LSTA24 OR (2) SIMPSON HTS20 TO WOOD POST OR (2) SIMPSON HETA16 TO CMU COL. U.N.O. ON ROOF PLAN			
BM6	/ ' '	(2) - 1 3/4" x 16" LVL 2.0E Fb=2950		CONNECTION: PROVIDE (2) SIMPSON LSTA24 OR (2) SIMPSON HTS20 TO WOOD POST OR (2) SIMPSON HETA16 TO CMU COL. U.N.O. ON ROOF PLAN			
* VERIFY WITH PLAN CORRECT LENGTH OF BEAMS REQUIRED ** SEE PLAN FOR TOP OF BEAM INDICATIONS NOTE: NAIL MULTIPLE PLY BEAM TOGETHER w/ (2) 1d NAILS @ 8" O.C. TYP. U.N.O.							
	120 M	IPH OPEN ENCLOS		G PRES			
A	+25.9 / -28.1			B	+25.9 / -34	.7	
C	+24.7	+24.7 / -26.9		D	+24.7 / -32	.4	
E	+23.2	+23.2 / -25.4		F	+23.2 / -29	9.3	
G	SEE	SEE COVER		H	SEE COVE	ER .	
WALL TYPE							
SYM. TYPE							
NEW MASONRY WALL							
	2 x BEARING WALL - SEE BEARING WALL SCHEDULE						
	NON BRG. INTERIOR WALL						
FLOOR PLAN SCHEDULES							

		CONNECTOR SCHEDULE				
UPLIFT(Lb)	MARK	SIMPSON CONNECTOR W/ FASTENERS	TYPE	SPF	SYP	
0	A	HETA16 W/ (9) - 10d x 1 1/2" OR HETA20 W/ (9) - 10d x 1 1/2"	FRAME TO MASONRY	1585	1615	
1290	B	H2.5A W/ (10) - 8d NAILS FRAME TO 535 FRAME		535	600	
0	(C)	H10 W/ (16) - 8d x 1 1/2" FRAME TO FRAME FRAME		990		
1500	D	D MTS12 W/ (14) - 10d x 1 1/2" FRAME TO FRAME		860	1000	
2200	Œ	MGT W/ (22) - 10d NAILS & 5/8" FRAME TO 3140		3140	3140	
2300	F	HTS20 W/ (24) - 10d x 1 1/2"	FRAME TO FRAME	1245	1450	
2320	G	HGT-2 W/ (16) - 10d W/ 5" EMBED (2) - 3/4" DIA. EPOXY ANCHORS. (HGT-3 FOR 3PLY)	FRAME TO MASONRY	6280	6280	
5645	H	SIMPSON FGTR W/ (18)-SDS 1/4" x 3" WOOD SCREWS AND (2) 1/2" x 5" TITAN HD ANCHOR BOLTS	FRAME TO MASONRY	3600	5000	
5645		(2) HETA16 OR (2) HETA20 1 PLY W/ (10) - 10d x 1 1/2" OR 2 PLY W/ (12) - 16d	FRAME TO MASONRY	2035 2500	2035 2500	
6970	⟨J⟩	(2) LGT-2 W/ (32)-16d SINKERS (14) 1/4" x 2 1/4" TITENS	FRAME TO MASONRY	3700	4300	
6970		(2) LGT-3 W/ (24) SDS 1/4" x 2-1/4" & (8) - 3/8" x 5" TITENS HD's	FRAME TO MASONRY	5310	7370	
6970	⟨K⟩	SIMPSON HU410 OR HUC410 (MAX) W/ (18)-16d & (10)-10d NAILS	BEAM TO BEAM		G#2090 U#1570	
	(L)	SIMPSON HU410 OR HUC410 W/ (18) TITEN 1/4" x 2 3/4" & (10)-10d NAILS	BEAM TO MAS		G#5085 U#1810	

ROOF FRAMING NOTES

TILE ROOFING SYSTEM (SEE ARCH.) OVER MIN. 1/2" CDX (4 PLY) PLYWOOD SHINGLE ROOFING SYSTEM (SEE ARCH.) OVER MIN. 7/16" OSB ON PRE-ENGINEERED WOOD TRUSSES AT 2'-0" O.C. MAX. SEE PLAN FOR TYP. **ROOF SLOPE**

ROOF FRAMING SCHEDULES



DESIGNER:

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DESIGN/ BUILD

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ENGINEERING CONSULTANT

TO THE BEST OF THE ENGINEER'S KNOWLEDGE AND UNDERSTANDING, THE STRUCTURAL PLANS AND SPECIFICATIONS COMPLY WITH THE

FLORIDA BUILDING CODE SIGNED AND SEALED FOR THE

STRUCTURAL PORTION OF THIS DRAWING.

ENGINEERING

ASSOCIATES

951 S. Orange Blossom Trail Apopka, FL 32703 Office: (407) 814-2125 / FAX: (407) 880-2309

CERTIFICATION OF AUTHORIZATION NO. 9161

CARL A. BROWN, P.E.

August 18, 2010
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FL # 56126

BUILDER INFORMATION:

Essian Construction 1907 West State Road 434 Longwood, Florida 32750 OFFICE: 407-245-8920 FAX: 407-245-8290

PROJECT INFORMATION:

1224 Roxboro Road Longwood, Florida

PLAN HISTORY		
DATE	DESCRIPTION	
02-15-10	PRELIMINARY	
05-07-10	FOR PERMIT	
06-21-10	BUILDING COMMENTS	
08-18-10	DESIGN CHANGES	

SHEET DATA:

PHIL KEAN MITCH BURDEN CHECKED BY: APPROVED BY: PHIL KEAN PHIL KEAN

SHEET DESCRIPTION

SCHEDULES

SHEET

